

# APPENDIX G

**PARSONS**

# **Drainage and Stormwater Management Report**

**Adelaide Street North Municipal Class  
Environmental Assessment Study**

**Final Report**

**Submitted to:**

**City of London**

**July 2020**



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# 1. Introduction

## 1.1 PROJECT DESCRIPTION

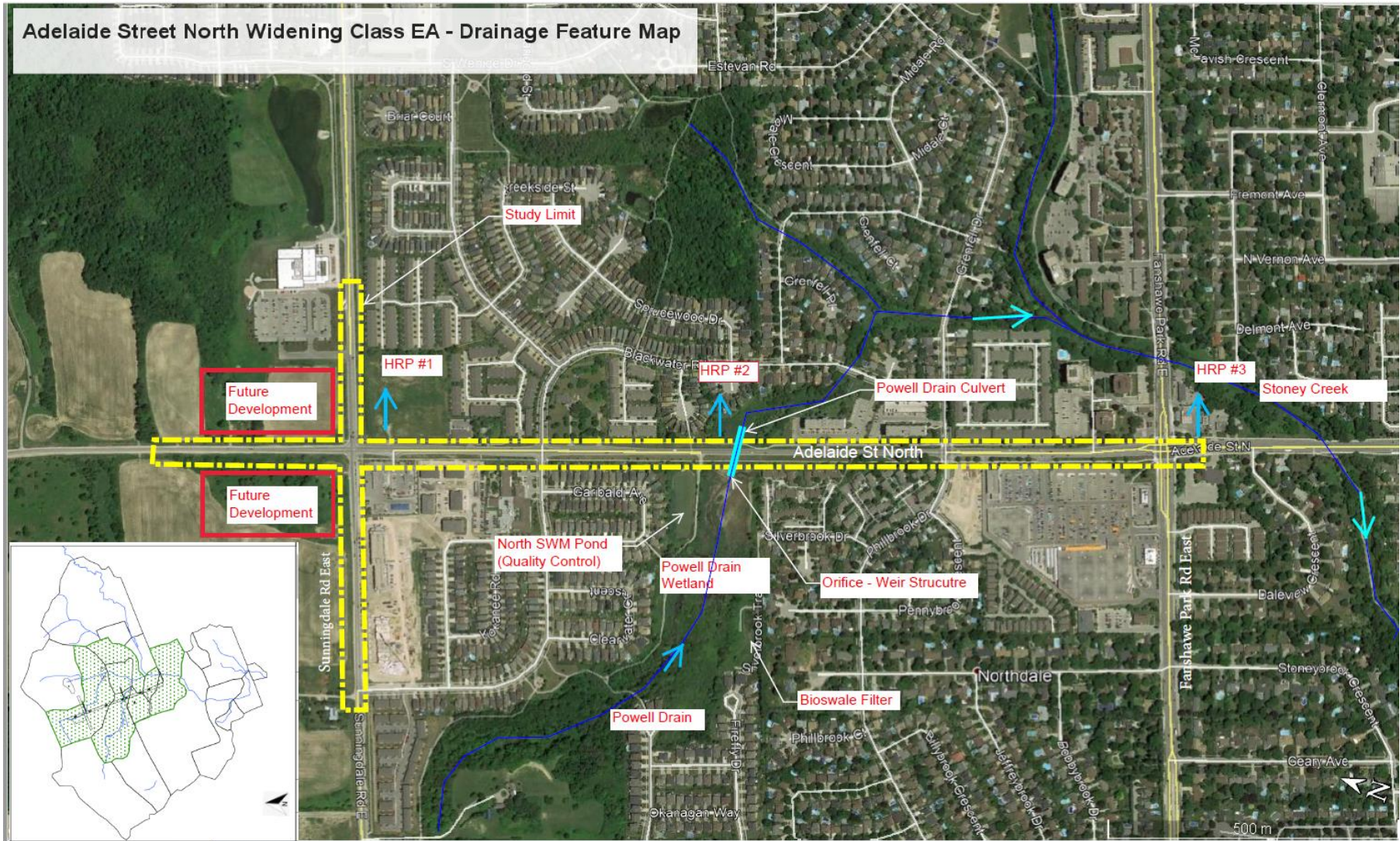
Parsons was retained by the City of London to undertake an Environmental Assessment (EA) Study for the proposed widening of Adelaide Street North from Fanshawe Park Road East to Sunningdale Road East under jurisdiction of the City of London.

As part of the EA Study, this Drainage and Stormwater Management (SWM) Report documents the existing drainage patterns and floodplain conditions and assesses the hydraulic performance of the culvert crossing in the study limit. This report also addresses the SWM strategy to reduce the runoff impacts resulting from the proposed road widening.

The study area for this EA consists of Adelaide Street North from Fanshawe Park Road East to 350m north of Sunningdale Road East, and Sunningdale Road East from Blackwater Road west of Adelaide Street North to Stoney Creek Community Centre Entrance east of Adelaide Street North. **Figure 1-1** shows the study limit of this project. The existing Adelaide Street North roadway consists of a two-lane paved road and the proposed improvements include widening the road to a four-lane cross-section.

As shown in **Figure 1-1**, the Powell Drain discharges the north-west segment tributary areas of the Stoney Creek watershed along the creek and crosses Adelaide Street North through a culvert. The Powell Drain wetland is located upstream of the culvert and an orifice-weir structure is installed immediately upstream of the culvert inlet headwall. The orifice-weir structure is designed and installed to provide a low flow regime in the creek and enhance the ecology of the wetland.

It should be noted that the study area falls in the jurisdiction of the Upper Thames River Conservation Authority (UTRCA).



**Figure 1-1: Study Area**

## 1.2 SCOPE AND OBJECTIVES

This report illustrates how the proposed works within the study area will impact the drainage system and identifies potential mitigation measures needed to meet SWM criteria outlined by the City. The Powell Drain crosses Adelaide Street North at 700 m north of Fanshawe Park Road East. A hydrology study has been undertaken for the north-western segment of the Stoney Creek watershed and the hydraulic performance of the existing culvert crossing at Adelaide Street North was assessed.

The objectives of this study are as follows:

- Undertake a hydrologic and hydraulic analysis of the proposed widening of Adelaide Street North;
- Develop a drainage and stormwater management plan for the proposed Adelaide Street North configuration that minimizes impact on the existing watercourse and drainage system;
- Manage post-development runoff from road drainage within the right of way (ROW);
- Provide safe drainage conveyance to reduce potential flood risks, and prevent damage to private and public properties;
- Provide quality control to post-development runoff prior to discharge to the outlets;
- Provide mitigation measures where necessary; and
- Undertake an investigation of reported road overtopping issues at Adelaide Street North culvert crossing.

The scope of work of the subject tasks includes:

- Review of background information such as the development studies in the watershed and Stoney Creek Subwatershed Study along with servicing information and existing flooding complaints;
- Discussions about the status of the existing Powell Drain wetland at the west of the water crossing of Adelaide Street North;
- Hydrologic and hydraulic assessment of existing drainage features along the Powell Drain particularly for the culvert crossing of Adelaide Street to determine the drainage issues;
- Design the proposed drainage features along the proposed road to confirm hydrologic, hydraulic and SWM criteria are met and identify any issues which may arise; and
- Provide possible solutions to overcome the issues identified through the assessment/design.

## 2 Background Data

### 2.1 BACKGROUND DATA

This Drainage and SWM Report builds upon the findings and recommendations of the following reports:

- Stoney Creek Subwatershed Study (Paragon Engineering Limited, 1996).
- Functional Stormwater Management Report, Ardshell Uplands Development Phase 1 (Development Engineering Limited, May 2002).
- Uplands North Subdivision Functional SWM Servicing Report (AECOM, May 2011).
- Powell Drain Remediation Design (Delcan, 2014)
- Functional Stormwater Management Design Report of Stoney Creek SWMF No. 2 (MTE, June 2017)

The Stoney Creek Subwatershed Study provides a detailed hydrologic study of the Stoney Creek Subwatershed, including hydrologic characteristics, design storms and modeling details. The hydrologic characteristics and design storms outlined in this study have been updated to reflect the latest developments which have an impact on the runoff for the current study. The detailed design of the SWMF located upstream of the Powell Drain culvert crossing of Adelaide Street North is reported in the Functional Stormwater Management Report of Ardshell Uplands Development Phase 1. The SWMF includes orifice and weir structures to provide adequate quantity control and low flow to enhance the ecology of the Powell Drain Wetland. The Uplands North Subdivision Functional SWM Servicing Report presents a functional SWM servicing strategy and SWM facility design for the Uplands North subdivision, which is located north of Sunningdale Road. This subdivision is a part of the Stony Creek watershed. In 2014, Delcan provided a remediation design study for the Powell Drain upstream of the Powell Drain Wetland. The current IDF parameters are used for proposed conditions following the City of London’s Design Specifications and Requirements Manual, 2019.

### 2.2 SITE VISIT

Parsons conducted two site visits, separately on June 27<sup>th</sup>, 2018 and October 16<sup>th</sup>, 2018. In both site visits, it was observed that the orifice-weir structure immediately upstream of the culvert crossing at Adelaide Street North is clogged with vegetation and debris. This clogging will have an impact on the discharge coefficient of the structure and reduce the discharge performance, which will eventually increase the water level in the wetland, particularly at storm events. The pictures of this structure from site visits are shown in **Appendix A**.



## 3 Drainage Design Criteria

### 3.1 RAINFALL DATA

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The 3-hour AES Chicago distribution was selected for application in the Stoney Creek watershed, according to the Stoney Creek Subwatershed Study (Paragon Engineering Limited 1996). The same, AES – Chicago Storm distribution parameters are imported in the model as rainfall data, including 2-yr, 5-yr, 25-yr, 100-yr and 250-yr as regulatory storm. The IDF parameters obtained from the City’s Design Specifications and Requirements Manual, 2019, have been considered for the existing and proposed conditions. Design storm input data can be found in **Table B.3a** and **Table B.3b** in **Appendix B**.

### 3.2 ALLOWABLE FLOW RATE

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The post development flow rates shall approximate the existing flow rates for all storm events from 2-year to the 250-year (Environmental Planning Policy Manual for the Upper Thames River Conservation Authority, 2006). This criterion provides quantity control for any developments proposed in the UTRCA jurisdiction.

### 3.3 HYDRAULIC DESIGN CRITERIA

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The following criteria obtained from the Design Specifications and Requirements Manual by the City of London are utilized in the assessment of the existing and proposed drainage design:

- 2-year design storm for minor flows based on the City of London Rainfall Intensity Curve
- Runoff coefficients (C value):
  - 0.5 for single family/semi detached
  - 0.7 for commercial/medium density
  - 0.9 for road right of way
- Time of concentration ( $T_c$ ) at upstream end:
  - 19.0 minutes for single family/semi-detached
  - 10.5 minutes for commercial and road right of way
- Any new culvert or culverts that are being redesigned, replaced or impacted by road works or road widening must be designed to meet the hydraulic requirements established by Ministry of Transportation Ontario (MTO).
- Minimum 50-year storm event must be conveyed by the culvert.
- According to MTO HDDS, 2008, WC-2 and WC-7, the head water depth over culvert rise ratio for the culverts with a rise less than 3 m shall be less than 1.5 and the freeboard shall be over 0.3 m

## 4 Hydrologic Analysis

### 4.1 EXISTING CONDITIONS

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The study area lies within the Stoney Creek Subwatershed regulated by the Upper Thames River Conservation Authority (UTRCA). The existing subwatershed boundaries, water crossing and the existing drainage patterns with delineated road drainage sub-areas for Adelaide Street North in the study area are presented in Existing Drainage Area Map (**Figure B-1**).

The existing drainage patterns of Adelaide Street North from Fanshawe Park Road East to Sunningdale Road East mainly cross high-density residential areas. The Stoney Creek Subwatershed is being crossed by Adelaide Street North within the study limit at approximately 700m north of Fanshawe Park Road. As shown in **Figure B-1**, three hydrologic reference points (HRPs) have been identified for the overland drainage patterns and the existing outlets within the study limit:

- HRP1 represents major overland flow as well as minor system from the most north boundary of the study limit to Sunningdale Road East which are being conveyed towards southerly to the Creek.
- HRP2 represents the road drainage to the water crossing at the Adelaide Street North where the culvert is located.
- HRP3 is at the south study limit close to Fanshawe Park Road East.

#### 4.1.1 Methodology

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There was no hydrologic model available from UTRCA for this study. A new model was therefore developed to represent the hydrologic conditions for the study area using Visual OTTHYMO (VO) version 5. Two sets of hydrologic modeling have been developed in this study as the Adelaide Street North sub-areas and Stoney Creek Watershed.

In the case of Adelaide Street North road drainage, the overland drainage areas within the ROW were delineated based on the location of high points and low points and HRP locations along the road profile. The hydrologic parameters are defined based on the input parameters for the larger sub-watershed where they are extracted from. Due to the urban nature of the watershed, all these areas are defined as STANDHYDs in the hydrologic model. The imperviousness is adjusted based on the distribution of pervious and impervious areas within the road ROW.

For the Stoney Creek subwatershed study, the modeling parameters of each catchment area that transverses the Adelaide Street North have been extracted from the Stoney Creek Subwatershed Study (Paragon Engineering Limited, 1996). The catchment information of the Stoney Creek Subwatershed Study, labeled as ID 152, ID 142, ID 142.1, ID 108, ID 105 are used to cover the study area. As noted in the Stoney Creek Subwatershed Study (1996), these sub-catchments used to be rural areas, while they are currently developed to urban and residential areas as shown in the **Figure 1-1** and discussed in Uplands North Subdivision Functional SWM Servicing Report (AECOM, May 2011). The VO5 modeling parameters are modified to represent this development.

**Tables B.1 to B.2** in **Appendix B** show the original and modified input parameters used for each sub-catchment.

### 4.1.2 Adelaide Street North Sub-Areas

As described in **Section 4.1**, existing peak flows from the road catchments are determined at each HRP using VO5. **Table 4-1** summarizes the existing peak flows at all HRPs considering the current IDF parameters as per the City of London’s Design Specifications and Requirements Manual, 2019. **Visual OTTHYMO Modelling Schematic-1 Road Drainage - Existing Condition** in **Appendix B** illustrates the existing condition of the road catchments. Detailed hydrologic modeling output files are included in **Appendix B**.

**Table 4-1: Hydrologic Analysis - Existing Condition Peak Flows at HRPs (m<sup>3</sup>/s)**

Return Period (3hr Chicago)	HRP #	HRP # 1	HRP # 2	HRP # 3
	VO ID	10	20	30
	A (ha)	5.42	17.73	1.58
	2-yr	0.544	2.620	0.278
	5-yr	0.556	2.677	0.284
	10-yr	0.686	3.304	0.346
	25-yr	0.903	4.099	0.444
	50-yr	1.038	4.684	0.506
	100-yr	1.179	5.295	0.583
	250-yr	1.414	6.657	0.687

### 4.1.3 Stoney Creek Watershed

Powell Drain is part of the Stoney Creek sub-watershed which discharges the north sub-watersheds and runs between Sunningdale Road East and Adelaide Street North. A VO5 modeling is provided to determine the hydrologic analysis of the drainage areas upstream of the culvert crossing at Adelaide Street North, as discussed in **Section 4.4.1**. Upstream drainage areas are delineated for the water crossing (Refer to **Figure B.1**). The watercourse transverses from northwest to southeast through sub-catchments of IDs 143 and 142 to Powell Drain Creek and it crosses both Sunningdale Road and Adelaide Street through the culverts.

The Uplands North Subdivision Functional SWM Servicing Report (AECOM, May 2011) assessed the SWM servicing strategy for the Uplands North Subdivision. The SWMFs at this subdivision are expected to provide adequate quantity and quality controls for agricultural and residential areas discharging to Powell Drain. A 900 mm CSP culvert and a 150 mm tile drainpipe downstream of these SWMFs crosses the Sunningdale Road and discharges the major storm flows to Powell Drain. **Table B.4** in **Appendix B** shows the storage – discharge curve for Uplands North Subdivision SWMFs and is used in the hydrologic modeling.

The Powell Drain discharges to the Powell Drain wetland. The outflow of which is controlled by an orifice-weir structure. This structure is installed immediately upstream of the culvert crossing at Adelaide Street North and provides a low flow

discharge to the downstream receivers and allows fish passage from Powell Drain and wetland along the creek. The stage – discharge curve for the orifice-weir structure is adopted from Functional Stormwater Management Report Ardshell Uplands Development (Development Engineering Limited, May 2002) and presented in **Table B.5** in **Appendix B**.

Two scenarios as uncontrolled and controlled conditions are considered in the VO5 hydrologic analysis by applying the current IDF curves. The uncontrolled condition does not include any storage volumes as a result of the SWMFs and the Powell Drain wetland. This condition is used for calibrating the subwatershed characteristics with the results in Stoney Creek Subwatershed Study (Paragon Engineering Limited, 1996). The uncontrolled peak flows for the subwatersheds are summarized in **Table 4-2**. The controlled condition includes the storage volumes in the model and routes the hydrograph generated by the subwatersheds. This condition presents a real situation for assessing the discharge at the culvert crossing. **Table 4-3** shows the discharge comparison between the VO5 controlled condition results and the previous study at Sunningdale Road culvert crossing. The comparison indicates that the VO5 modeling results agree well with the Uplands North Subdivision Functional SWM Servicing Report (AECOM, May 2011).

**Table 4-4** presents the flow discharging from the orifice-weir structure at Adelaide Street North. The results indicate that the maximum runoff for the 100-year storm event does not exceed 3.5 m<sup>3</sup>/s and the regulatory flow is less than 4.5 m<sup>3</sup>/s. It should be noted that this analysis is performed with the assumption of no clogging for the orifice-weir structure. VO5 modeling output files are presented in **Appendix B**.

**Table 4-2: Uncontrolled Upstream Catchment Peak Flows**

Uncontrolled Upstream Peak Flows (m <sup>3</sup> /s)		
Return Period	Catchment: ID 142, ID 143 (With Total Area of 182.25 ha)	
	Discharge Based on 1996 -IDF	Discharge Based on 2019 -IDF
2-yr	16.03	16.62
5-yr	23.40	17.00
10-yr	-	21.16
25-yr	35.82	26.72
50-yr	-	30.72
100-yr	45.31	34.90
Regulatory 250-yr	51.52	41.56

**Table 4-3: Comparison of Controlled Upstream Catchment Peak Flows with Previous Study at Sunningdale Road**

Return Period	Flow from Previous Study* (m <sup>3</sup> /s)		Flow from VO5 Model (m <sup>3</sup> /s)
	Uplands North Sub (116 ha)		ID 143 (127.59 ha)
	Discharge Based on 1996 -IDF		Discharge Based on 1996 -IDF
2-yr	0.16		0.18
5-yr	0.20		0.26
25-yr	0.32		0.35
100-yr	0.35		0.37
Regulatory 250-yr	0.38		0.39

\* Uplands North Subdivision Functional SWM Servicing Report (AECOM, May 2011)

**Table 4-4: Controlled Peak Flows Discharging from Orifice-Weir Structure at Adelaide Street**

Return Period	Controlled Flow Rate Based on 2019 -IDF (m <sup>3</sup> /s)	Head Water Elevation* (m)
2-yr	0.62	250.81
5-yr	0.64	250.82
10-yr	0.78	250.88
25-yr	1.26	251.06
50-yr	1.70	251.21
100-yr	3.21	251.65
Regulatory 250-yr	4.06	251.96

\*- Head Water Elevation are based on the HECRAS modelling results discussed in **Section 5**

## 4.2 PROPOSED CONDITIONS

### 4.2.1 Adelaide Street North Sub-Areas

Visual OTTHYMO Modelling Schematic-2 – Road Drainage - Proposed Condition in **Appendix B** shows the proposed condition scenario used for the hydrologic modeling for the Adelaide Street North sub-area. The existing subwatershed boundaries, water crossing and the proposed drainage patterns with delineated road drainage sub-areas for Adelaide Street North in the study area are presented as Drainage Area Map Proposed Condition in **Figure B.2** in **Appendix B**.

The peak flow rates generated from the proposed road catchments are determined at each HRP. **Table 4-5** summarizes the proposed peak flows at all HRPs for different storm events. The increase in the peak flows occurs for all storm events at the HRPs. This rise (max 19.7 %) is due to the increase in the imperviousness of the proposed road catchment areas. Detailed output results for this condition are furnished in **Appendix B**.

**Table 4-5: Hydrologic Analysis - Proposed Condition Peak Flows at HRPs (m<sup>3</sup>/s)**

HRP #	HRP #1		HRP #2		HRP #3	
VO ID	10		20		30	
A (ha)	5.42		17.73		1.58	
Return Period (3hr-Chicago)	Flow (m <sup>3</sup> /s)	Increase %	Flow (m <sup>3</sup> /s)	Increase %	Flow (m <sup>3</sup> /s)	Increase %
2-yr	0.583	7.2%	2.682	2.4%	0.317	14.0%
5-yr	0.595	7.0%	2.739	2.3%	0.323	13.7%
10-yr	0.731	6.6%	3.393	2.7%	0.414	19.7%
25-yr	0.955	5.8%	4.186	2.1%	0.505	13.7%
50-yr	1.095	5.5%	4.780	2.0%	0.574	13.4%
100-yr	1.242	5.3%	5.400	2.0%	0.645	10.6%
250-yr	1.493	5.6%	6.774	1.8%	0.756	10.0%

## 5 Hydraulic Analysis

### 5.1 METHODOLOGY

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Since there is no existing hydraulic model available for the Powell Drain from the previous studies, Parsons developed a hydraulic model using HEC-RAS software based on the following information:

- Culvert crossing structure information obtained from Adelaide Street North Reconstruction As-Built Drawings;
- The available topographical surveying information; and
- The results of hydrologic analysis

### 5.2 HYDRAULIC ANALYSIS

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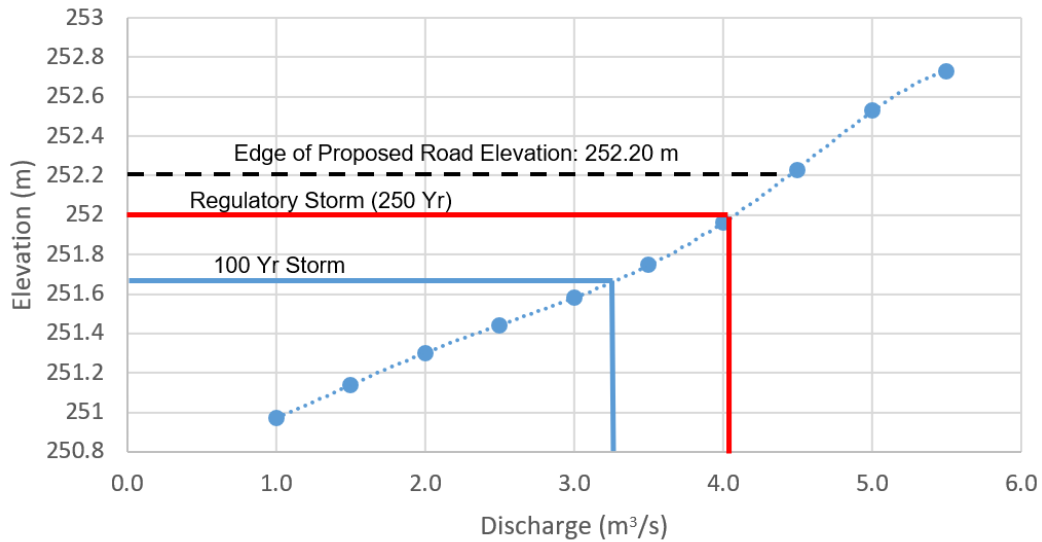
#### 5.2.1 Culvert Crossing at Adelaide Street North

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The Powell Drain Culvert structure at Adelaide Street North consists of two segments. The first segment is an 1800 mm CSP pipe which starts at the culvert inlet, immediately downstream of the orifice-weir structure, with a length of 22.5 m. The second segment has the same length and is an arch 1830 mm x 1140 mm CSP. The two segments are connected to each other through a 3000 mm Maintenance Hole. The proposed road curb elevation at the culvert crossing is 252.2 m.

Hydraulic analysis has been performed to assess the capacity of the existing crossing culvert. Different flow rates up to 5.5 m<sup>3</sup>/s are analysed to determine the variation of headwater elevation and the full capacity of the culvert. The results show that the culvert under Adelaide Street North can convey the flows up to 4.5 m<sup>3</sup>/s without any road overtopping. In other words, the culvert does not obstruct the flow up to 4.5 m<sup>3</sup>/s and the orifice-weir structure will be the hydraulic control structure for the Powell Drain Creek. **Figure 5-1** shows the variation of headwater level at different flow rates at the culvert crossing. As shown, in **Figure 5-1** and **Appendix C**, the freeboard for the 100-yr storm is approximately 0.5m and the headwater/rise is 0.77 which meets the MTO requirement.

As noted in the Environmental Impact Study (EIS), A barrier to fish passage was noted in the downstream reach in the form of a perched steel grade control structure approximately 40 m downstream of the culvert outlet, east of Adelaide Street North. Field investigations observed that that water flowing over the sheet pile wall fell approximately 1 m before hitting gabion baskets in the channel below. Future channel or culvert rehabilitation projects could improve fish habitat in this section of drain by replacing the gabion baskets with round stone to prevent potential entrapment of aquatic animals. Additional future improvement projects should consider removing the sheet pile grade control wall and designing the channel to gradually raise the streambed such that fish may pass (i.e., via rocky ramp).



**Figure 5-1: Variation of Headwater Level at Different Flow Rates**

### 5.3 FLOODPLAIN MAPPING

Figure 5-2 shows the variation of available storage within the Powell Drain Wetland area at different contour levels. The storages used for 100-year and 250-year storm events are obtained from the hydrologic analysis results. The corresponding water levels at these storm events are considered to generate the floodplain mapping with the adjusted topography information. Again, it should be noted that the control hydraulic point for the Powell Drain Creek is the orifice-weir structure for the flows up to 250-year storm events.

The floodplain mapping is shown in Figure C.1 in Appendix C, which includes the extent of the flood level at different storm events, contour lines and existing topographical information, the adjacent developed area and roads.

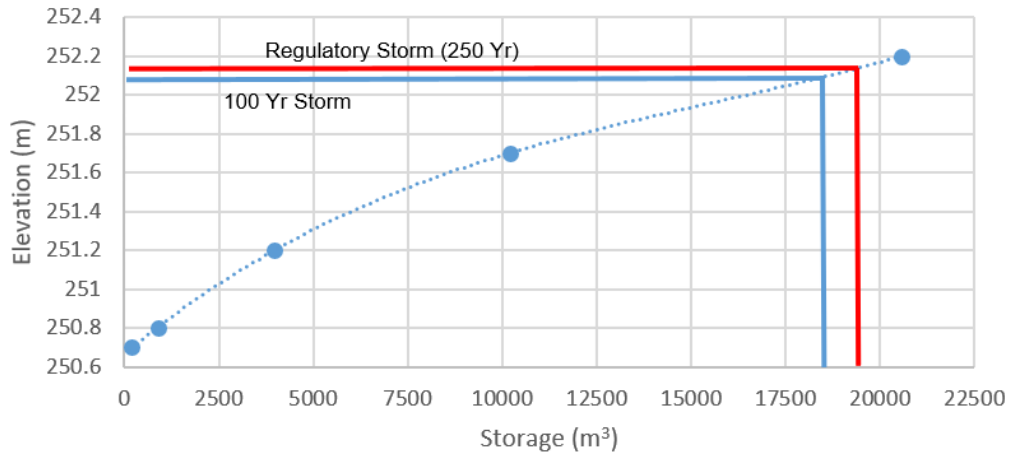


Figure 5-2: Variation of Storage Volume at Different Contour Elevations

## 5.4 STORM SEWER ANALYSIS

As part of this report, an evaluation of the existing minor storm flow collection system in the study area has been completed. This evaluation included an examination of existing as-built information (drainage area plans, design sheets and plan/profile drawings), SIMMS sheets and previous drainage reports (specifically, the Sunningdale Road Improvements ESR) to ascertain which drainage areas have already been examined and/or accounted for in the design of the existing system as well as to establish the criteria used in the original design.

This information was used to delineate proposed drainage areas for the entire study area (Figure 1-1) and establish design criteria appropriate for the recommended road improvement alternative. For the areas with existing storm sewers, the areas and criteria are compared to the original designs and an evaluation was completed for the capacity of the existing sewer outlets to accommodate the recommended road construction alternative using both the original and the current criteria. As part of the evaluation, the Time of Concentration ( $T_c$ ) was calculated for each outlet. In areas where there is no existing storm sewer system, a proposed system to drain minor storm flows was conceptually designed. Detail information is presented in Appendix D.

### 5.4.1 Existing Condition

An examination of existing information shows that within the study area, Adelaide Street North is currently serviced by storm sewers from Sunningdale Road in the north to Fanshawe Park Road to the south. The lands adjacent to this portion of Adelaide Street North have either been fully developed or their development plans are in process with the proposed drainage plans. Currently, there are five (5) outlets available in the study area to service minor storm drainage which will be discharging to the three (3) HRP, as discussed in Section 4.1:



- Outlet 1** - An existing 1200mm diameter storm sewer that services Adelaide Street North from Philbrook Drive to Fanshawe Park Road (discharges to HRP3);
- Outlet 2** - An existing 1050mm diameter storm sewer that services Adelaide Street North from Philbrook Drive to Powell Drain. This outlet drains directly to Powell Drain and services the Northbrook Subdivision on the west side of Adelaide Street North (discharges to HRP2);
- Outlet 3** - An existing 525mm diameter storm sewer that services Adelaide Street North from Sunningdale Road East to Powell Drain. This sewer outlets directly to the culvert crossing Adelaide Street North for Powell Drain (discharges to HRP2);
- Outlet 4** - An existing 1200mm diameter storm sewer that services different areas which discharge to SWM Pond 1N (discharges to HRP1) as:
  - Sunningdale Road East from Blackwater Road to 150m east of Adelaide Street North;
  - A part of Adelaide Street North from 580m north of Sunningdale Road East to Sunningdale Road intersection; and
  - Future developments on both sides of Adelaide Street north of Sunningdale including the Applewood Estates Subdivision which discharges to future Stoney Creek 2 SWM pond. The Stoney Creek 2 SWMF drainage area map was not available during the study. This subdivision is located on the north east quadrant of the Sunningdale Road East and Adelaide Street North intersection. As noted in the Functional Stormwater Management Design Report of Stoney Creek SWMF No. 2 by MTE June 2017, the major and minor drainage system flow will be directed by roadside ditches towards the existing 900mm CSP culvert crossing Sunningdale Rd.
- Outlet 5** - The existing roadside ditch on the south side of Sunningdale East and east of Adelaide Street North discharges the 2.05 ha drainage area on the northwest corner of the intersection of Sunningdale Road East and Adelaide Street North. Although the City's documentation shows that this area is included in the design of Outlet 4, the Sunningdale Road Improvements ESR and the drainage plan for Stoney Creek Subdivision on the southeast corner of the intersection indicate that this area was included in the design of SWM Pond 1W. Additionally, during the construction of the Stoney Creek Subdivision, this ditch was regraded and sized to accommodate this drainage area and direct flows to SWM Pond 1W (discharges to HRP1).

Currently, the existing storm sewers discharge to Outlets 1, 2 and 3, and roadside ditches convey the road drainage to Outlets 4 and 5. The transition from a rural drainage system to urban drainage at Outlets 4 and 5 will be discussed in **Section 5.4.2**.

Hydraulic capacity of the existing drainage system at outlets 1, 2 and 3 have been assessed as follows:

- **Outlet 1:** Existing documentation for the sewers on Adelaide Street North that are connected to this outlet is limited to some plan/profile drawings and a drainage area plan and design sheet for the sewers on Fanshawe Park Road. Since the flows on Fanshawe Park Road govern the design of the existing 1200mm sewer outlet, it was not possible to obtain the original  $T_c$  used to design the sewers on Adelaide Street North. Thus, this evaluation was limited to the original runoff coefficient ( $C=0.85$ ) and the proposed criteria. Using the current criteria, the flow is 9% over the existing sewers full capacity. If the original runoff coefficient is used, this reduces the flow to approximately 3% over the capacity of the existing sewers. This indicates that the existing storm sewers on this portion of Adelaide Street North may need to be replaced with larger pipes. However, considering that the City based criteria for establishing the runoff coefficient is relatively conservative, preliminary calculations indicate that a runoff coefficient of 0.83 and lower for the lands associated with the existing sewers system would result enough discharge to be conveyed through the storm pipes. It is recommended to calculate a composite runoff coefficient during detailed design stage to better assess the existing sewer system capacity at this portion.

- **Outlet 2:** The City has provided the existing drainage plans and design sheets for all the existing storm sewers on this portion of Adelaide Street North. The existing information shows that an initial  $T_c$  of 13.50 minutes and a runoff coefficient of 0.85 was used to design the storm sewer system, which do not meet the current City standards. By applying the recommended criteria, the existing sewers still have adequate capacity to accommodate the design storm. It should be noted that the storm drainage for the Northbrook Subdivision is the governing flow and outlets to these sewers approximately halfway to the outlet at Powell Drain.
- **Outlet 3:** Based on the existing drainage plans and design sheets for all the existing storm sewers discharging to Outlet 3, an initial  $T_c$  of 13.50 minutes and an overall runoff coefficient of 0.8 were considered in the original design, which do not comply with the current City standards. Based on the original  $T_c$  and the proposed C value, the generated discharge from the catchments is 8% over the full sewer capacity. By applying the design criteria, the generated discharge will be 20% over the existing sewers capacity. Preliminary calculations indicate that a composite runoff coefficient of 0.76 or lower for the lands associated with the existing sewers would result adequate flow for the sewer system to avoid surcharging in this area. It is recommended to calculate a composite runoff coefficient during detailed design stage to assess the existing sewers capacity more accurately at this portion.

### 5.4.2 Proposed Condition

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The road drainage discharging to Outlets 4 and 5 is currently a rural cross section and the minor and major drainage discharges to the roadside ditches. The Sunningdale Road Improvements ESR recommends converting Sunningdale Road to an urban cross section with curb and gutter, while the recommended design alternative for Adelaide Street North is to maintain the existing rural cross section north of Sunningdale. Since future developments are being proposed in this area, it is anticipated that the existing rural drainage pattern will change to urban cross section with a need of new storm sewer construction. A conceptual storm sewer design plan has been completed and shown in **Appendix D**.

#### Outlet 4

The Sunningdale Road Improvements ESR, the drainage area plans and design sheets of the sewers upstream of SWM Pond 1N and the Stoney Creek Subdivision study all have examined future drainage and provided design information for the ultimate storm sewer drainage in this portion of the study area. These studies indicate that all future development areas except the northwest corner area of the intersection of Sunningdale Road and Adelaide Street North would discharge to SWM Pond 1N, through a storm sewer trunk system. As part of the construction of SWM Pond 1N, the storm sewer trunk was constructed extending to the west past the entrance to the Stoney Creek YMCA/Stoney Creek Library facility and ending with a capped 1200mm diameter stub. The excluded area will be discharging to Outlet 5.

As part of the SWM pond design, a preliminary storm sewer design for future work that can provide drainage to the developments north of Sunningdale Road was completed by Development Engineering (London) Ltd. in 2009. A design review indicates that enough service area is provided on the north side of Sunningdale Road East to install the storm sewer pipe; however, no Sunningdale Road drainage consideration is noticed in the design drawings. The rationale for locating the sewer in an easement rather than under the road could be due to the constructability issue of storm sewer system crossing the existing 1200mm concrete trunk watermain on Sunningdale Road.

The recommended solution is to construct a local sewer to provide drainage only for Sunningdale Road and connect at the existing 1200mm stub. This would allow a smaller storm sewer to be constructed under Sunningdale Road. This plan would

also permit the Sunningdale Road sewer construction be separate from the development sewer system without any construction schedule interference until they connect at the existing stub.

**Outlet 5**

As shown in **Appendix D**, Areas Ex 1 and Ex 2 are located on the northwest corner of Adelaide Street North and Sunningdale Road. Currently, drainage is provided for these areas via a culvert crossing through the intersection to a roadside ditch on the southeast corner. As part of the development of the Stoney Creek Subdivision, this ditch was reconstructed such that it would continue to provide drainage for Area Ex 1 to SWM Pond 1W but drainage from Area Ex 2 were to be redirected to Outlet 4 or SWM Pond 1N. If the drainage discharges the flow to the southeast part, it requires the City to maintain the existing culvert diagonally across the intersection and cross the existing 1200mm and 600mm watermains at this intersection. Also, the City needs to maintain an open drainage ditch through an easement along Sunningdale Road East. It is recommended to explore more in the detail design stage to redirect the drainage to SWM Pond 1N and overcome the above-mentioned issues.

## 6 Stormwater Management

### 6.1 STORMWATER MANAGEMENT CRITERIA

The study area falls under the jurisdiction of the Upper Thames River Conservation Authority (UTRCA) and so, the criteria outlined by UTRCA guidelines are considered to provide the stormwater management strategy. **Table 6-1** summarizes the criteria for both quantity and quality control based on the UTRCA manual.

**Table 6-1: Stormwater Management Criteria**

<b>Quantity Control</b>	Quantity control typically ensures that post-development flow rates approximate pre-development rates for all return period increments from the 2-year to the 250-year. Any modifications to pre-development hydrology must be justified on the basis that they enhance the pre-development condition and must consider factors such as flood severity, flood timing and in-stream erosion potential of the receiving <i>watercourse</i> .
<b>Quality Control</b>	Enhanced protection corresponds to the end-of-pipe storage volumes required for the long-term average removal of 80% of suspended solids (80% TSS removal) (Stormwater Management Planning and Design Manual, MOECC, 2003)

### 6.2 PROPOSED STORMWATER MANAGEMENT STRATEGY

As part of the background study, the geotechnical study report prepared by Golder Associates Ltd. (Feb.,2019) was reviewed and the key points regarding the soil characteristics and ground water level are outlined as follows:

- Generally, the first 5 meters in depth is silt, sand and sand-gravel materials which is the indication of a high permeable layer. Cohesive materials with low-permeability characteristics are reported at 2 m below the existing ground elevation at Phillbrook Drive/Grenfell Drive.
- The groundwater level is reported to vary between 1.3 m and 3.4 m below the ground surface along the project limit.

The findings indicate that generally the Project area has adequate geotechnical potential to build shallow SWMFs for quantity control, except at the Phillbrook Drive/Grenfell Drive and Adelaide Street North intersection. The findings are considered in the study of proposed quantity control alternatives.

### 6.2.1 Quantity Control

---

As discussed in **Section 4.1**, the three HRP considered in the analysis show a subtle flow increase for the less frequent storm event (10-yr storm) with a maximum 19.7% compared to existing release rate for HRP #3. However, for infrequent events (e.g. 100-Year), less increase release rates can be observed for the proposed condition. The following discusses different options to match the pre-and post-development flow rates, which will be explored more in the detail design stage:

- Storage pipes and orifice control in the proposed storm sewer trunk system can attenuate the post-development flow rates to the pre-development values. This option is a popular and common practice in storm sewer systems. It can be constructed as part of a sewer trunk system. This option can be explored more in detailed design stage to determine the storage and orifice sizes.
- “Silva Cell” system is an effective Low Impact Development (LID) measure that can be utilized at different locations upstream of HRP outlets to reduce the post-development flow rates. The “Silva Cell” is a modular suspended pavement system that uses soil volumes to support large tree growth and provide powerful on-site stormwater management through absorption, evapotranspiration, and interception. The advantage of the system is that it doesn’t require costly maintenance, however the system structure and construction are costly.
- Bio-Swales and Bio-Retentions are other popular, cost-effective and industry accepted LID measures that could be used to provide water quantity/quality measures. The proposed road cross sections show the road drainage is contained curb to curb and the only space available to construct Bio-Swale is in the boulevards on the sides of the sidewalks. To convey the runoff to the Bio-Swale several lead pipes can be extended from the catch-basins to discharge the flow to the Bio-Swales or the drainage flow can directly discharge from the storm sewers to the Bio-Swales. This option looks to be a feasible quantity control measure for this study as shown in **Appendix E**. The storm sewer profile and road profile indicate that there is a slight elevation difference between the storm sewer outlet and the Bio-Retention/Bio-Swale inlet. At the detailed design stage, it should be confirmed if sufficient boulevard space, elevation difference, ideal soil condition and utility locations would allow installation of this type of LID systems to store, treat and release roadway drainage.
- Perforated pipe system is another option to be considered as water quantity/ quality measures to alleviate roadway drainage. The perforated pipe would run parallel to the storm sewer system, outside of the roadway limit, under the sidewalk and ultimately discharging to the existing municipal system. Soil type, high ground water levels, utility

conflict, winter maintenance and salt particles could be the main prohibiting items in selecting this option. There is also the potential risk of clogging the perforations along the pipe that will cause backwater in the sewer system and may cause serious damage to the sewer network.

### 6.2.2 Quality control

Quality control is also provided throughout the study limit in the form of Oil Grit Separator (OGS). Stormceptor units are proposed at four locations upstream of the outlets to HRP's throughout the study limit.

Modeling for quality control units was undertaken using PCSWMM from the Imbrium website. Modeling output files as well as typical design details including drawings and specifications for all units are included in **Appendix E. Table 6-2** summarizes the quality control units required for the Project based on drainage area characteristics at each outlet. Modeling results show that the 80% TSS removal can be achieved by specific types of unit. It should be noted that Outlet 5 discharges the external drainage area to SWM Pond 1W through the ditch and culvert system as similar as existing conditions. Since the proposed works will not impact the drainage pattern no quality control measures are required for Outlet 5 discharging to HRP #1.

**Table 6-2: Summary of Quality Control Units**

Outlet @ HRP	Drainage Area (ha)	Average Impervious Proposed %	Type Required	%TSS Removal Achieved
Outlet 1 @ HRP #3	1.57	65	OSR 300	86
Outlet 2 @ HRP #2	1.17	64	OSR 300	89
Outlet 3 @ HRP #2	2.65	60	OSR 300	81
Outlet 4 @ HRP #1	3.90	50	OSR 750	88

## 7 Erosion and Sediment Control During Construction

The erosion and sediment control (ESC) practices to be developed during detailed design should follow the Ontario Provincial Standards Specifications (OPSS) for Roads and Public Works and the Erosion and Sediment Control Guidelines for Urban Construction (Greater Golden Horseshoe Area Conservation Authorities, Dec 2006).

Impacts on the surrounding environment related to roadway projects can be mitigated by proper erosion and sediment control measures. To minimize potential erosion and sedimentation during construction activities, the following practice need to be considered to mitigate erosion:

- Minimize disturbed areas;
- Limit soil exposure, i.e., implement construction phasing to limit the duration of soil exposure;
- Preserve existing vegetation where and as much possible;

- Stabilize exposed soil as soon as possible with vegetation, where possible, to reduce the amount of sediment that could be conveyed further downstream to existing watercourses;
- Limit slope grades and lengths;
- Encourage sheet flow to avoid concentrated flow paths;
- Reduce water velocities across the ground; and
- Use and maintain Best Management Practices (BMP) during construction until disturbed areas have stabilized.

## 8 Conclusions and Considerations

The drainage and stormwater management requirements as part of the environmental assessment for the improvement of Adelaide Street North from Fanshawe Park Road East to Sunningdale Road East in the City of London has been completed. The EA for the widening of Adelaide Street North from two-lanes to four-lanes also includes the assessment of the hydraulic capacity of the culvert crossing at Adelaide Street North. The impacts of the proposed works on the road drainage, water-crossing floodplain and the stormwater management strategy have been evaluated in this report.

The key results of the drainage and stormwater management study are as follows:

- The existing storm sewer drainage system along the Adelaide Street North corridor has adequate capacity to convey the proposed runoff due to the proposed works in most of the study area. New CBs and MHs shall be installed at some locations and the existing ones will need to be removed. To ensure that there is no need to upsize the storm sewers at the locations with less capacity, further exploration needs to be considered in the detailed design stage.
- The existing orifice-weir structure located immediately upstream of the Powell Drain Culvert controls the flow in the Powell Drain Creek. The site visits on June 27<sup>th</sup>, and October 16<sup>th</sup>, 2018 indicated that the structure is clogged with debris and vegetation. Clogging in the orifice-weir structure will reduce the discharge capacity of the Powell Drain Creek and will increase the water level elevation in the Powell Drain Wetland. This will increase the risk of road overtopping particularly at low-frequent storm events.
- Three options can be considered to improve the hydraulic performance of the orifice-weir structure as follows:
  - Provide a regular maintenance and cleanout of the orifice-weir structure from vegetation and debris to reduce the risk of clogging and any potential flooding. It is recommended to consider this option as the cost effective, feasible solution to alleviate the flooding due to the low hydraulic performance of the orifice-weir structure. An Operation and Maintenance Manual for the routine maintenance of this structure which will improve the fish migration from downstream to the Powell Drain wetland should be provided in the detail design stage.
  - Relocate the orifice-weir structure from the existing location to a new location approximately 2 m upstream of the culvert entrance. This relocation will decrease the flooding and road overtopping risk in case of clogging occurrence. The gap between the relocated orifice-weir structure and the culvert entrance will provide adequate space for the flow to overtop the structure and convey through the culvert without any adverse impact on the adjacent properties. A geotechnical study will be required in the detailed design stage to assess the creek condition at upstream of the Powell Drain Culvert if the orifice-weir structure relocation is selected.
  - The proposed wildlife culvert on the north side of the culvert crossing at Adelaide Street North can help mitigate the potential flooding at low-frequent storm events if needed. This wildlife culvert is proposed to enhance the animal passage across Adelaide Street North along the Powell Drain. The culvert's inlet is at higher elevation than the orifice-weir structure and can help mitigate the potential flooding at low-frequent storm events if needed.

- The results from HEC-RAS analysis indicate that the existing culvert crossing at Adelaide Street North has enough capacity to convey the controlled flow for different storm events. The existing culvert can convey the controlled flow at less frequent storms including 50-yr 100-yr and Regulatory (250-yr) storm events without any road overtopping.
- The proposed uncontrolled runoff from the storm sewer system at HRPs is larger than the existing condition. Different options are discussed in **Section 6.2.1** to mitigate the increased peak flows and provide quantity control prior discharging to HRPs. In addition to storage pipes and Orifice control option, bio-swale option is recommended as a popular, cost-effective and industry accepted practice to provide water quantity measures at the outlets. The preliminary locations and dimensions for bio-swale are presented in **Appendix E**. Further exploration will be required in detailed design stage to finalize the dimensions and locations of the quantity control measures.
- Four (4) Oil Grit Separator (OGS) units are recommended to be placed upstream of the outlets throughout the study limit to provide enhanced quality control. Stormceptor unit type OSR 300 is sized for the storm sewer system discharging to Outlet 1, Outlet 2 and Outlet 3 and type OSR 750 is sized for Outlet 4. No quality control measures are required for Outlet 5, as it just discharges the external drainage area.



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Drainage Engineer

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Reviewed by:

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Engineering Manager -Water Resources

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## References

The following documents were used in preparation for this report:

1. Functional Stormwater Management Report, Ardshell Uplands Development Phase 1 (Development Engineering, May 2002).
2. Uplands North Subdivision Functional SWM Servicing Report (AECOM, May 2011).
3. Stoney Creek Subwatershed Study (Paragon Engineering Limited, 1996).
4. Powell Drain Remediation Design (Delcan, 2014)
5. Preliminary Geotechnical Assessment prepared by Golder Associates Ltd. (Feb.,2019)
6. Design Specifications and Requirements Manual by the City of London
7. MTO Highway Drainage Design Standards (Jan 2008)
8. Environmental Planning Policy Manual for the Upper Thames River Conservation Authority UTRCA (June 2006)

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# APPENDIX A

## Background Study

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## **APPENDIX A: Background Study**

### Site Visit Photos

- June 27th 2018
- October 16th 2018

**Site Visit on June 27th 2018**

Site Visit Dated June 27<sup>th</sup> 2018



Powell Drain Culvert Headwall



Powell Drain Culvert Orifice – Weir Structure



Powell Drain Culvert Orifice – Weir Structure



Powell Drain Wetland Area



Powell Drain Creek





Powell Drain Culvert Outlet

**Site Visit on October 16th 2018**

Site Visit Dated October 16<sup>th</sup> 2018



Powell Drain Culvert Orifice – Weir Structure



Powell Drain Culvert Outlet



Powell Drain Wetland Area

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# **APPENDIX B**

## **Hydrologic Analysis**

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## APPENDIX B: Hydrologic Analysis

**Figure B.1:** Existing Drainage Area Map

**Figure B.2:** Proposed Drainage Area Map

**Table B.1:** Original VO Input Parameters - Stoney Creek Subwatershed Study 1996

**Table B.2:** Parsons Modified VO Input Parameters - NasHyd to StandHyd

**Table B.3a:** IDF Parameters for 3-hr AES Chicago Storm - Stoney Creek Subwatershed Study 1996

**Table B.3b:** IDF Parameters for AES Storm - Design Specifications & Requirements Manual 2019 (City of London)

**Table B.4:** Outlet Stage – Discharge Relationship for Sunningdale Road Crossing

**Table B.5:** Outlet Stage – Discharge Relationship for Adelaide Street Crossing

**Visual OTTHYMO Modelling Schematic-1 - Existing Condition**

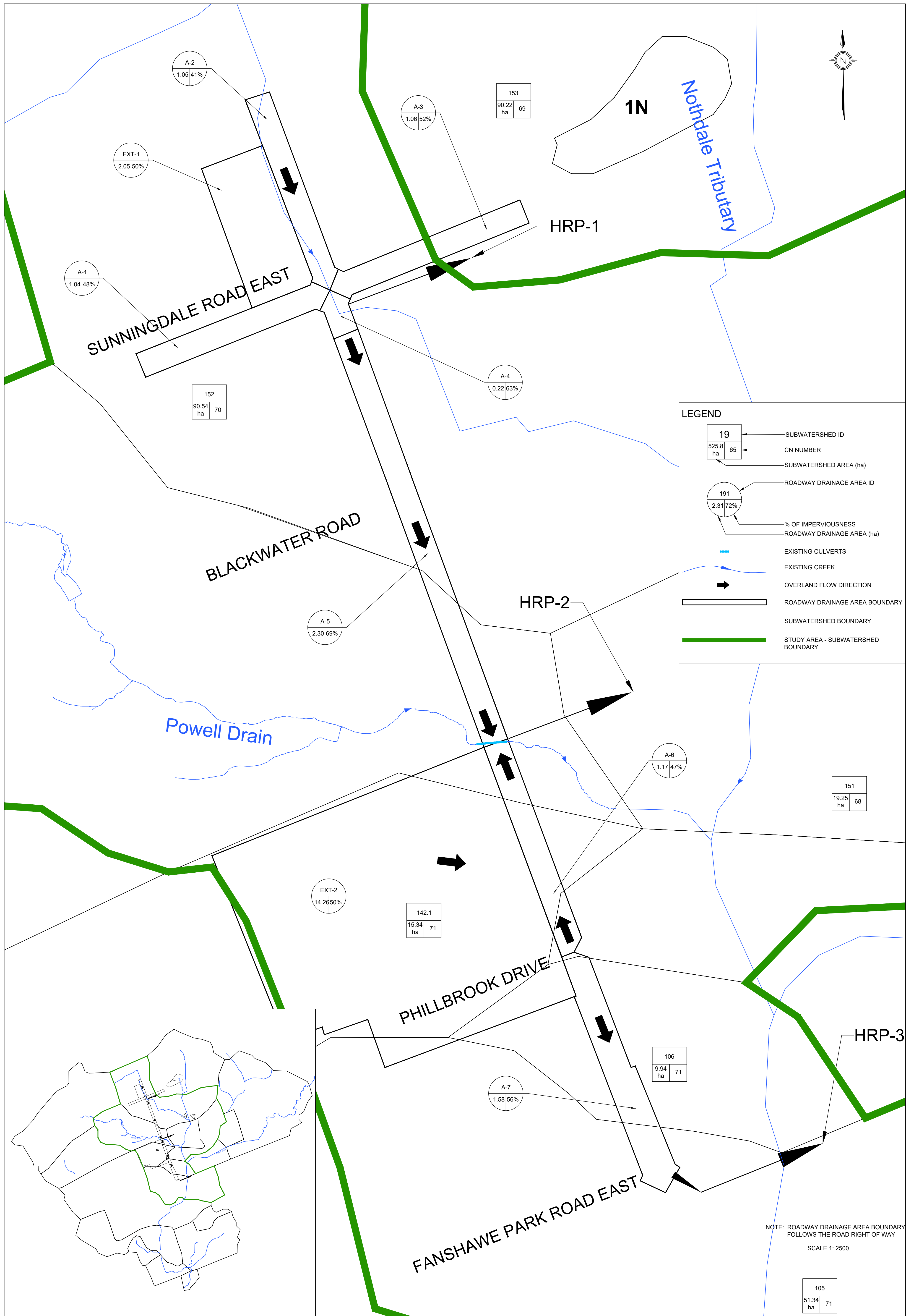
VO Output File – Road Drainage - Existing Condition

**Visual OTTHYMO Modelling Schematic-2 - Proposed Condition**

VO Output File – Road Drainage - Proposed Condition

**Visual OTTHYMO Modelling Schematic-3 - Upstream Drainage**

VO Output File – Upstream Drainage of Adelaide Water Crossing



No.	DATE	REVISIONS	BY
1	May 2019		
2			

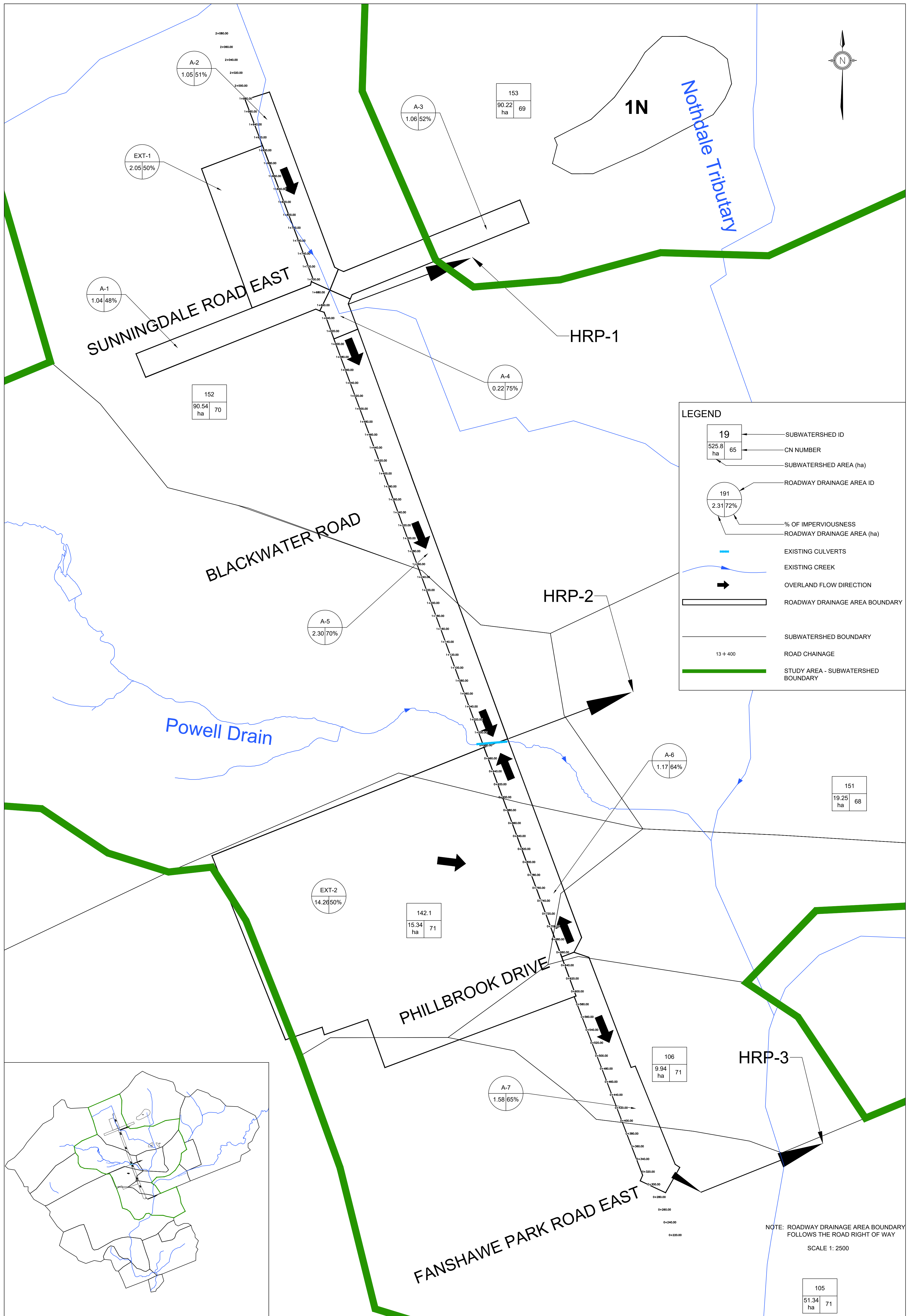


**PARSONS**  
625 COCHRANE DR., SUITE 500, MARKHAM, ONT. L3R 9R9  
TEL: 905-943-0500 FAX: 905-943-0400

DESIGN  
M.H.  
DRAWN  
M.H.  
CHECKED

ADLAIDE STREET IMPROVEMENTS  
FROM SUNNINGDALE Rd TO FANSHAWE PARK Rd  
**DRAINAGE AREA MAP**  
**EXISTING CONDITION**  
(HYDROLOGIC ANALYSIS)

DWG. NAME  
EX DA  
CONT. NO.  
-  
SHEET NO.  
Figure B.1



No.	DATE	REVISIONS	BY
1	May 2019		
2			



**PARSONS**  
 625 COCHRANE DR., SUITE 500, MARKHAM, ONT. L3R 9R9  
 TEL: 905-943-0500 FAX: 905-943-0400

DESIGN  
M.H.  
 DRAWN  
M.H.  
 CHECKED

ADLAIDE STREET IMPROVEMENTS  
 FROM SUNNINGDALE Rd TO FANSHAWE PARK Rd  
**DRAINAGE AREA MAP**  
**PROPOSED CONDITION**  
 (HYDROLOGIC ANALYSIS)

DWG. NAME  
PROP DA  
 CONT. NO.  
-  
 SHEET NO.  
Figure B.2



**Table B.1: Original VO Input Parameters - Stoney Creek Subwatershed Study 1996**

Watershed	Catchment ID	Tributary Name	VO ID	Type	Area (hectares)	CN	IA	Impervious Area Flow Length(m)	TIMP	XIMP	Impervious Area Slope(%)	Impervious Area Mannings	IA(mm)	Pervious Area Slope(%)	Pervious Area Flow Length(m)	Pervious Area Mannings
Stoney Creek	142	Powell Drain	142	Rural Component	54.150	76	16.04									
Stoney Creek	142.1		1421	Urban Component	15.000	71	10.37	258.20	0.3	0.29	1	0.015	2	3.00	40.00	0.20
Stoney Creek	143	Powell Drain	143	Rural Component	119.240	77	15.17									
Stoney Creek	151	Nothdale Tributary	151	Rural Component	18.200	68	11.95									
Stoney Creek	152	Nothdale Tributary	152	Rural Component	90.600	70	21.77									
Stoney Creek	153	Nothdale Tributary	153	Rural Component	91.470	69	22.82									
<b>Total Area</b>					442.810											

**Table B.2: Modified VO Input Parameters - NasHyd to StandHyd**

Watershed	Catchment ID	Tributary Name	VO ID	Type	Area (hectares)	CN	IA per(mmm)	Impervious Area Flow Length(m)	TIMP	XIMP	Impervious Area Slope(%)	Impervious Area Mannings	IA imp(mm)	Pervious Area Slope(%)	Pervious Area Flow Length(m)	Pervious Area Mannings
Stoney Creek	143	Powell Drain	143	StandHyd	127.59	69	5	922.28*	0.61	0.49	1*	0.013*	2	2.00*	40.00*	0.25*
Stoney Creek	142	Powell Drain	142	StandHyd	54.150	76	5	600.83*	0.67	0.64	1*	0.013*	2	2.00*	40.00*	0.25*

S: 80.2105

\* default number from VO, since no available parameters.

**Table B.3a: IDF Parameters for 3-hr AES Chicago Storm - Stoney Creek Subwatershed Study 1996**

Design Storm Input Data					
Return Period(years)	Rainfall Parameters			3-hr Rainfall Amount(mm)	24-hr Rainfall-Snowmet
	a	b	c		
2	724.7	5.000	0.800	33.3	30
5	1330.3	7.938	0.855	45.4	40
25	1496.3	5.250	0.825	60.4	55
100	1499.5	3.298	0.794	71.6	67
250(Regulatory Storm)	1498.1	2.188	0.778	78.5	XX

**Table B.3b: IDF Parameters for AES Storm - Design Specifications & Requirements Manual 2019 (City of London)**

Parameter	25mm*	2yr**	5yr	10yr	25yr	50yr	100yr	250yr
A	538.85	1290.00	1183.74	1574.382	2019.372	2270.665	2619.363	3048.22
B	6.331	8.500	7.641	9.025	9.824	9.984	10.5	10.03
C	0.809	0.860	0.838	0.860	0.875	0.876	0.884	0.888

Table B.4: post-development flow rates and the post-development water ponding elevation by wetland and existing Sunningdale Road Crossing (AECOM, 2011)

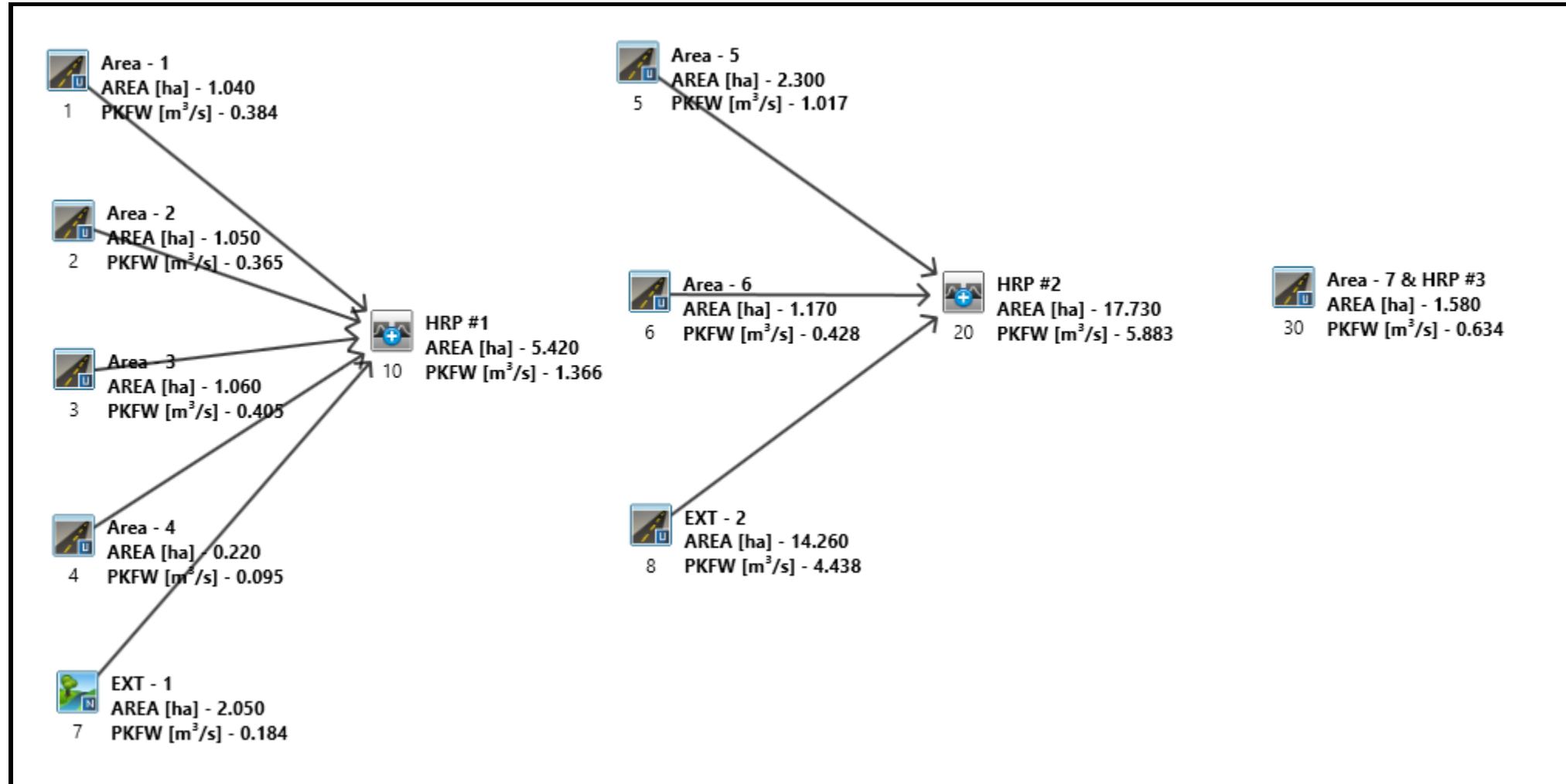
Storm Event (year)	Pre-Development Target Peak Outflow (m <sup>3</sup> /s)	Post-Development Peak Inflow (m <sup>3</sup> /s)	Post-Development Peak Outflow (m <sup>3</sup> /s)	Post-Development Storage (m <sup>3</sup> )	Post-Development Elevation (m)
2	0.18	5.9	0.16	17 840	271.2
5	0.24	8.9	0.20	27 010	271.4
10	0.29	11.1	0.26	32 740	271.6
25	0.34	13.5	0.32	38 870	271.7
50	0.40	18.1	0.34	43 720	271.8
100	0.47	20.6	0.35	48 710	271.9
250	0.67	21.8	0.38	62 160	272.2

\* Based on 3-hour Chicago Storm and SWMHYMO model: UpNPre3.dat and UpNPost8.dat

Table B.5: Stage-storage for quantity storage Powell Drain Wetland Area (Development Engineering, 2002)

Storm Event	Peak Inflow (cms)	Peak Outflow (cms)	Max. Storage (m <sup>3</sup> )	Stage (Elev. in metres)
2-Year	2.22	0.85	14450	252.47
5-Year	3.66	1.83	17990	252.67
10-Year	5.69	2.01	18050	252.67
25-Year	8.30	2.02	18070	252.67
50-Year	10.76	2.39	18110	252.67
100-Year	12.71	2.97	18200	252.67

## Visual OTTHYMO Modelling Schematic-1 – Road Drainage Existing Condition



VO Output - EX.txt

```

-----
V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W V I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000
Developed and Distributed by Civi.ca Infrastructure
Copyright 2007 - 2019 Civi.ca Infrastructure
All rights reserved.

```

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYM0 5.2\VO2\voinput.dat  
Output filename:  
C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3  
Summary filename:  
C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3  
DATE: 11/12/2019 TIME: 11:30:10  
USER:

COMMENTS:

```

-----
***** SIMULATION : 01 AES 2-yr *****
-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

```

```

-----
CALIB STANDHYD ( 0030) Area (ha) = 1.58 Dir. Conn. (%) = 56.00
ID= 1 DT= 5.0 min Total Imp(%) = 56.00

Surface Area (ha) = 0.98 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 0.70
Average Slope (%) = 1.00 2.00
Length (m) = 102.63 40.00
Mannings n = 0.013 0.250

```

```

-----
VO Output - EX.txt
Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 15.00
Storage Coeff. (mi n) = 2.46 (ii) 11.90 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 15.00
Unit Hyd. peak (cms) = 0.30 0.09

PEAK FLOW (cms) = 0.26 0.06
TIME TO PEAK (hrs) = 0.50 0.75
RUNOFF VOLUME (mm) = 33.03 17.03
TOTAL RAINFALL (mm) = 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

```

```

-----
CALIB STANDHYD ( 0007) Area (ha) = 2.05 Curve Number (CN) = 70.0
ID= 1 DT= 5.0 min Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

Unit Hyd. Tpeak (cms) = 0.391

PEAK FLOW (cms) = 0.062 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 6.100
TOTAL RAINFALL (mm) = 34.030
RUNOFF COEFFICIENT = 0.179

```

- (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

```

```

-----
CALIB STANDHYD ( 0001) Area (ha) = 1.04 Dir. Conn. (%) = 48.00
ID= 1 DT= 5.0 min Total Imp(%) = 48.00

```

VO Output - EX.txt

```

-----
Surface Area (ha) = 0.50 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 2.00
Length (m) = 83.27 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 15.00
Storage Coeff. (mi n) = 2.17 (ii) 11.61 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 15.00
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.15 0.04
TIME TO PEAK (hrs) = 0.50 0.75
RUNOFF VOLUME (mm) = 33.03 17.03
TOTAL RAINFALL (mm) = 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

```

```

-----
CALIB STANDHYD ( 0002) Area (ha) = 1.05 Dir. Conn. (%) = 41.00
ID= 1 DT= 5.0 min Total Imp(%) = 41.00

Surface Area (ha) = 0.43 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 2.00
Length (m) = 83.67 40.00
Mannings n = 0.013 0.250

```

```

-----
Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 15.00
Storage Coeff. (mi n) = 2.18 (ii) 11.62 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 15.00
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.13 0.05
TIME TO PEAK (hrs) = 0.50 0.75
RUNOFF VOLUME (mm) = 33.03 17.03
TOTAL RAINFALL (mm) = 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

VO Output - EX.txt

THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

```

```

-----
CALIB STANDHYD ( 0003) Area (ha) = 1.06 Dir. Conn. (%) = 52.00
ID= 1 DT= 5.0 min Total Imp(%) = 52.00

Surface Area (ha) = 0.55 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 2.00
Length (m) = 84.06 40.00
Mannings n = 0.013 0.250

```

```

-----
Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 15.00
Storage Coeff. (mi n) = 2.18 (ii) 11.62 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 15.00
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.17 0.04
TIME TO PEAK (hrs) = 0.50 0.75
RUNOFF VOLUME (mm) = 33.03 17.03
TOTAL RAINFALL (mm) = 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

```

```

-----
CALIB STANDHYD ( 0004) Area (ha) = 0.22 Dir. Conn. (%) = 63.00
ID= 1 DT= 5.0 min Total Imp(%) = 63.00

```

VO Output - EX.txt  
 IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.14 0.08  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 38.30 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 114.34 48.33  
 over (mi n) = 5.00 15.00  
 Storage Coeff. (mi n) = 1.36 (ii) 10.80 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 15.00  
 Unit t Hyd. peak (cms) = 0.33 0.09  
 \*TOTALS\*  
 PEAK FLOW (cms) = 0.04 0.05 0.045 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.71  
 RUNOFF VOLUME (mm) = 33.03 17.03 27.07  
 TOTAL RAINFALL (mm) = 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3  
 AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0001): 1.04 0.163 0.50 24.70  
 + ID2= 2 ( 0002): 1.05 0.144 0.50 23.58  
 ID = 3 ( 0010): 2.09 0.307 0.50 24.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1  
 AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0010): 2.09 0.307 0.50 24.14  
 + ID2= 2 ( 0003): 1.06 0.178 0.50 25.34  
 ID = 1 ( 0010): 3.15 0.485 0.50 24.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3  
 AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0010): 3.15 0.485 0.50 24.54  
 + ID2= 2 ( 0004): 0.22 0.045 0.50 27.07  
 ID = 3 ( 0010): 3.37 0.530 0.50 24.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1  
 AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0010): 3.37 0.530 0.50 24.71  
 + ID2= 2 ( 0007): 2.05 0.062 0.83 6.10  
 ID = 1 ( 0010): 5.42 0.544 0.50 17.67

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VO Output - EX.txt  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 88.32 40.00  
 Length (m) = 0.013 0.250  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 114.34 48.33  
 over (mi n) = 5.00 15.00  
 Storage Coeff. (mi n) = 2.25 (ii) 11.69 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 15.00  
 Unit t Hyd. peak (cms) = 0.30 0.09  
 \*TOTALS\*  
 PEAK FLOW (cms) = 0.17 0.05 0.179 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.50  
 RUNOFF VOLUME (mm) = 33.03 17.03 24.54  
 TOTAL RAINFALL (mm) = 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.72

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3  
 Comments: 2yr 1hr AES  
 Ptotal = 34.03 mm  
 TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
 hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
 0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08  
 0.17 4.08 0.50 114.34 0.83 20.42  
 0.25 12.25 0.58 61.25 0.92 12.25  
 0.33 32.67 0.67 49.00 1.00 4.08

CALIB STANDHYD ( 0008 )  
 ID= 1 DT= 5.0 mi n  
 Area (ha) = 14.26  
 Total Imp(%) = 50.00 Dir. Conn. (%) = 50.00

Surface Area (ha) = 7.13 7.13  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 308.33 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 114.34 48.33  
 over (mi n) = 5.00 15.00  
 Storage Coeff. (mi n) = 4.76 (ii) 14.20 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 15.00  
 Unit t Hyd. peak (cms) = 0.22 0.08  
 \*TOTALS\*  
 PEAK FLOW (cms) = 1.80 0.54 1.944 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.75 0.50  
 RUNOFF VOLUME (mm) = 33.03 17.03 25.03  
 TOTAL RAINFALL (mm) = 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.74

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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VO Output - EX.txt  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3  
 Comments: 2yr 1hr AES  
 Ptotal = 34.03 mm  
 TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
 hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
 0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08  
 0.17 4.08 0.50 114.34 0.83 20.42  
 0.25 12.25 0.58 61.25 0.92 12.25  
 0.33 32.67 0.67 49.00 1.00 4.08

CALIB STANDHYD ( 0005 )  
 ID= 1 DT= 5.0 mi n  
 Area (ha) = 2.30  
 Total Imp(%) = 69.00 Dir. Conn. (%) = 69.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 1.59 0.71  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 123.83 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 114.34 48.33  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 2.75 (ii) 7.61 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit t Hyd. peak (cms) = 0.28 0.13  
 \*TOTALS\*  
 PEAK FLOW (cms) = 0.46 0.07 0.497 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.67 0.50  
 RUNOFF VOLUME (mm) = 33.03 17.03 28.07  
 TOTAL RAINFALL (mm) = 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3  
 Comments: 2yr 1hr AES  
 Ptotal = 34.03 mm  
 TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
 hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
 0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08  
 0.17 4.08 0.50 114.34 0.83 20.42  
 0.25 12.25 0.58 61.25 0.92 12.25  
 0.33 32.67 0.67 49.00 1.00 4.08

CALIB STANDHYD ( 0006 )  
 ID= 1 DT= 5.0 mi n  
 Area (ha) = 1.17  
 Total Imp(%) = 47.00 Dir. Conn. (%) = 47.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.55 0.62

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VO Output - EX.txt

ADD HYD ( 0020 )  
 1 + 2 = 3  
 AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0005): 2.30 0.497 0.50 28.07  
 + ID2= 2 ( 0006): 1.17 0.179 0.50 24.54  
 ID = 3 ( 0020): 3.47 0.676 0.50 26.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020 )  
 3 + 2 = 1  
 AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0020): 3.47 0.676 0.50 26.88  
 + ID2= 2 ( 0008): 14.26 1.944 0.50 25.03  
 ID = 1 ( 0020): 17.73 2.620 0.50 25.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L L (v 5. 2. 2003)  
 V V I SS U U A A L  
 V V I SS U U A A A A L  
 V V I SS U U A A L  
 V V I SSSS UUUU A A LLLLL  
 000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y M M 0 0  
 0 0 T T H H Y Y M M 0 0  
 000 T T H H Y Y M M 000  
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTHMMO 5.2\VO2\voi n.dat  
 Output filename:  
 C:\Users\p001279d\AppData\Local\Ci vi ca\WHS\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\dfaf7  
 3f-e898-4198-a0a5-23ec6db716d7\scse  
 Summary filename:  
 C:\Users\p001279d\AppData\Local\Ci vi ca\WHS\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\dfaf7  
 3f-e898-4198-a0a5-23ec6db716d7\scse

DATE: 11/12/2019 TIME: 11:30:12

USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 02 AES 5-yr \*\*\*\*\*

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\c4dc5ad3  
 Comments: 2yr 1hr AES  
 Ptotal = 34.03 mm

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VO Output - EX.txt  
 ata\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\51069ce4

Ptotal = 34.64 mm  
 Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB STANDHYD ( 0030)  
 ID= 1 DT= 5.0 min

Area (ha) = 1.58  
 Total Imp(%) = 56.00 Dir. Conn. (%) = 56.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.88	0.70
Dep. Storage (mm) = 1.00	2.00
Average Slope (%) = 1.00	2.00
Length (m) = 102.63	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.39 over (mi n) = 5.00  
 Storage Coeff. (mi n) = 2.44 (ii) 11.78 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00  
 Unit t Hyd. peak (cms) = 0.30 0.09

PEAK FLOW (cms) = 0.27 0.06  
 TIME TO PEAK (hrs) = 0.50 0.75  
 RUNOFF VOLUME (mm) = 33.64 17.50  
 TOTAL RAINFALL (mm) = 34.64 34.64  
 RUNOFF COEFFICIENT = 0.97 0.51

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.64 mm  
 Comments: 5yr 1hr AES

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\51069ce4

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB NASHYD ( 0007)  
 ID= 1 DT= 5.0 min

Area (ha) = 2.05  
 Total Imp(%) = 5.00 Curve Number (CN) = 70.0  
 U.H. Tp(hrs) = 0.20 # of Linear Res. (N) = 3.00

Unit t Hyd. Tpeak (cms) = 0.391

PEAK FLOW (cms) = 0.064 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 6.331  
 TOTAL RAINFALL (mm) = 34.640

VO Output - EX.txt  
 RUNOFF COEFFICIENT = 0.183  
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.64 mm  
 Comments: 5yr 1hr AES

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\51069ce4

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB STANDHYD ( 0001)  
 ID= 1 DT= 5.0 min

Area (ha) = 1.04  
 Total Imp(%) = 48.00 Dir. Conn. (%) = 48.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.50	0.54
Dep. Storage (mm) = 1.00	2.00
Average Slope (%) = 1.00	2.00
Length (m) = 83.27	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.39 over (mi n) = 5.00  
 Storage Coeff. (mi n) = 2.15 (ii) 11.49 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00  
 Unit t Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.15 0.05  
 TIME TO PEAK (hrs) = 0.50 0.75  
 RUNOFF VOLUME (mm) = 33.64 17.50  
 TOTAL RAINFALL (mm) = 34.64 34.64  
 RUNOFF COEFFICIENT = 0.97 0.51

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.64 mm  
 Comments: 5yr 1hr AES

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\51069ce4

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB STANDHYD ( 0002)  
 ID= 1 DT= 5.0 min

Area (ha) = 1.05  
 Total Imp(%) = 41.00 Dir. Conn. (%) = 41.00

VO Output - EX.txt

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.43	0.62
Dep. Storage (mm) = 1.00	2.00
Average Slope (%) = 1.00	2.00
Length (m) = 83.67	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.39 over (mi n) = 5.00  
 Storage Coeff. (mi n) = 2.16 (ii) 11.49 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00  
 Unit t Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.13 0.05  
 TIME TO PEAK (hrs) = 0.50 0.75  
 RUNOFF VOLUME (mm) = 33.64 17.50  
 TOTAL RAINFALL (mm) = 34.64 34.64  
 RUNOFF COEFFICIENT = 0.97 0.51

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.64 mm  
 Comments: 5yr 1hr AES

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\51069ce4

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB STANDHYD ( 0003)  
 ID= 1 DT= 5.0 min

Area (ha) = 1.06  
 Total Imp(%) = 52.00 Dir. Conn. (%) = 52.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.55	0.51
Dep. Storage (mm) = 1.00	2.00
Average Slope (%) = 1.00	2.00
Length (m) = 84.06	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.39 over (mi n) = 5.00  
 Storage Coeff. (mi n) = 2.17 (ii) 11.50 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00  
 Unit t Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.17 0.04  
 TIME TO PEAK (hrs) = 0.50 0.75  
 RUNOFF VOLUME (mm) = 33.64 17.50  
 TOTAL RAINFALL (mm) = 34.64 34.64  
 RUNOFF COEFFICIENT = 0.97 0.51

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

VO Output - EX.txt  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.64 mm  
 Comments: 5yr 1hr AES

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\51069ce4

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB STANDHYD ( 0004)  
 ID= 1 DT= 5.0 min

Area (ha) = 0.22  
 Total Imp(%) = 63.00 Dir. Conn. (%) = 63.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.14	0.08
Dep. Storage (mm) = 1.00	2.00
Average Slope (%) = 1.00	2.00
Length (m) = 38.30	40.00
Mannings n = 0.013	0.250

Max. Eff. Inten. (mm/hr) = 116.39 over (mi n) = 5.00  
 Storage Coeff. (mi n) = 1.35 (ii) 10.69 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00  
 Unit t Hyd. peak (cms) = 0.33 0.09

PEAK FLOW (cms) = 0.04 0.01  
 TIME TO PEAK (hrs) = 0.50 0.75  
 RUNOFF VOLUME (mm) = 33.64 17.50  
 TOTAL RAINFALL (mm) = 34.64 34.64  
 RUNOFF COEFFICIENT = 0.97 0.51

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
 1 + 2 = 5

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 ( 0001):	1.04	0.166	0.50 25.24
+ ID2 = 2 ( 0002):	1.05	0.147	0.50 24.11
ID = 3 ( 0010):	2.09	0.313	0.50 24.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
 3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 3 ( 0010):	2.09	0.313	0.50 24.68
+ ID2 = 2 ( 0003):	1.06	0.181	0.50 25.89
ID = 1 ( 0010):	3.15	0.495	0.50 25.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
ID1= 1 ( 0010): 3.15 0.495 0.50 25.08
+ ID2= 2 ( 0004): 0.22 0.046 0.50 27.64
ID = 3 ( 0010): 3.37 0.541 0.50 25.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)
3 + 2 = 1
AREA OPEAK TPEAK R.V.
ID1= 3 ( 0010): 3.37 0.541 0.50 25.25
+ ID2= 2 ( 0007): 2.05 0.064 0.83 6.33
ID = 1 ( 0010): 5.42 0.556 0.50 18.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM
File name: C:\Users\p001279d\AppData
Total = 34.64 mm
Comments: 5yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN
0.08 0.00 0.42 62.35 0.75 33.25
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB STANDHYD ( 0005)
Area (ha) = 2.30
Total Imp(%) = 69.00
Dir. Conn. (%) = 69.00
Surface Area (ha) = 1.59
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 123.83
Mannings n = 0.013
Max. Eff. Inten. (mm/hr) = 116.39
Storage Coeff. (mi n) = 2.73 (ii)
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.29

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.
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READ STORM
File name: C:\Users\p001279d\AppData
Total = 34.64 mm
Comments: 5yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN
0.08 0.00 0.42 62.35 0.75 33.25
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB STANDHYD ( 0006)
Area (ha) = 1.17
Total Imp(%) = 47.00
Dir. Conn. (%) = 47.00
Surface Area (ha) = 0.55
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 88.32
Mannings n = 0.013
Max. Eff. Inten. (mm/hr) = 116.39
Storage Coeff. (mi n) = 2.23 (ii)
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.30

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM
File name: C:\Users\p001279d\AppData
Total = 34.64 mm
Comments: 5yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN
0.08 0.00 0.42 62.35 0.75 33.25
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB STANDHYD ( 0008)
Area (ha) = 14.26
Total Imp(%) = 50.00
Dir. Conn. (%) = 50.00
Surface Area (ha) = 7.13

Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 308.33
Mannings n = 0.013
Max. Eff. Inten. (mm/hr) = 116.39
Storage Coeff. (mi n) = 4.72 (ii)
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.22
PEAK FLOW (cms) = 1.84
TIME TO PEAK (hrs) = 0.50
RUNOFF VOLUME (mm) = 33.64
TOTAL RAINFALL (mm) = 34.64
RUNOFF COEFFICIENT = 0.97

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
ID1= 1 ( 0005): 2.30 0.507 0.50 28.64
+ ID2= 2 ( 0006): 1.17 0.183 0.50 25.08
ID = 3 ( 0020): 3.47 0.690 0.50 27.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)
3 + 2 = 1
AREA OPEAK TPEAK R.V.
ID1= 3 ( 0020): 3.47 0.690 0.50 27.44
+ ID2= 2 ( 0008): 14.26 1.987 0.50 25.57
ID = 1 ( 0020): 17.73 2.677 0.50 25.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input file name: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1n.dat
Output file name:
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C:\Users\p001279d\AppData\Local\Clvi ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\VF376c7f-04f9-471d-8a85-8c363b28df4\scse
Summary File name:
C:\Users\p001279d\AppData\Local\Clvi ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\VF376c7f-04f9-471d-8a85-8c363b28df4\scse
DATE: 11/12/2019
USER:

COMMENTS:
\*\*\*\*\* SIMULATION 03 AES 10-YR \*\*\*\*\*

READ STORM
File name: C:\Users\p001279d\AppData
Total = 41.26 mm
Comments: 10yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN
0.08 0.00 0.42 74.27 0.75 39.61
0.17 4.95 0.50 138.63 0.83 24.76
0.25 14.85 0.58 74.27 0.92 14.85
0.33 39.61 0.67 59.41 1.00 4.95

CALIB STANDHYD ( 0030)
Area (ha) = 1.58
Total Imp(%) = 56.00
Dir. Conn. (%) = 56.00
Surface Area (ha) = 0.88
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 102.63
Mannings n = 0.013
Max. Eff. Inten. (mm/hr) = 138.63
Storage Coeff. (mi n) = 2.28 (ii)
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.30

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM
File name: C:\Users\p001279d\AppData
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VO Output - EX.txt  
 ata\Local\Temp\63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 10yr 1hr AES

Ptotal = 41.26 mm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
 NASHYD ( 0007) Area (ha) = 2.05 Curve Number (CN) = 70.0  
 ID= 1 DT= 5.0 min Ia (mm) = 5.00 # of Li near Res. (N) = 3.00  
 U. H. Tp (hrs) = 0.20

Unit Hyd. Qpeak (cms) = 0.391  
 PEAK FLOW (cms) = 0.091 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 9.043  
 TOTAL RAINFALL (mm) = 41.260  
 RUNOFF COEFFICIENT = 0.219

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
 STANDHYD ( 0001) Area (ha) = 1.04 Dir. Conn. (%) = 48.00  
 ID= 1 DT= 5.0 min Total Imp (%) = 48.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.50 0.54  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 83.27 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (mi n) = 2.01 (ii) 10.38 (ii)  
 Unit t. Tpeak (min) = 5.00 15.00  
 Unit t. Hyd. peak (cms) = 0.31 0.09  
 PEAK FLOW (cms) = 0.18 0.06 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 0.203 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 31.20  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.76

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - EX.txt

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.55 0.51  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 84.06 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (mi n) = 2.02 (ii) 10.39 (ii)  
 Unit t. Tpeak (min) = 5.00 15.00  
 Unit t. Hyd. peak (cms) = 0.31 0.09  
 PEAK FLOW (cms) = 0.20 0.06 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 0.221 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 31.89  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
 STANDHYD ( 0004) Area (ha) = 0.22 Dir. Conn. (%) = 63.00  
 ID= 1 DT= 5.0 min Total Imp (%) = 63.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.14 0.08  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 38.30 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 10.00  
 Storage Coeff. (mi n) = 1.26 (ii) 9.63 (ii)  
 Unit t. Tpeak (min) = 5.00 10.00  
 Unit t. Hyd. peak (cms) = 0.33 0.11  
 PEAK FLOW (cms) = 0.05 0.01 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.67 0.058 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 33.79  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

VO Output - EX.txt  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
 STANDHYD ( 0002) Area (ha) = 1.05 Dir. Conn. (%) = 41.00  
 ID= 1 DT= 5.0 min Total Imp (%) = 41.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.43 0.62  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 83.67 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (mi n) = 2.01 (ii) 10.39 (ii)  
 Unit t. Tpeak (min) = 5.00 15.00  
 Unit t. Hyd. peak (cms) = 0.31 0.09  
 PEAK FLOW (cms) = 0.16 0.07 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 0.180 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 29.98  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.73

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
 STANDHYD ( 0003) Area (ha) = 1.06 Dir. Conn. (%) = 52.00  
 ID= 1 DT= 5.0 min Total Imp (%) = 52.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.55 0.51  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 84.06 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (mi n) = 2.02 (ii) 10.39 (ii)  
 Unit t. Tpeak (min) = 5.00 15.00  
 Unit t. Hyd. peak (cms) = 0.31 0.09  
 PEAK FLOW (cms) = 0.20 0.06 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 0.221 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 31.89  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.77

VO Output - EX.txt  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
 1 + 2 = 3  
 AREA OPEAK TPEAK R. V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0001): 1.04 0.203 0.50 31.20  
 + ID2= 2 ( 0002): 1.05 0.180 0.50 29.98  
 ID = 3 ( 0010): 2.09 0.384 0.50 30.58

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
 3 + 2 = 1  
 AREA OPEAK TPEAK R. V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0010): 2.09 0.384 0.50 30.58  
 + ID2= 2 ( 0003): 1.06 0.221 0.50 31.89  
 ID = 1 ( 0010): 3.15 0.605 0.50 31.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
 1 + 2 = 3  
 AREA OPEAK TPEAK R. V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0010): 3.15 0.605 0.50 31.02  
 + ID2= 2 ( 0004): 0.22 0.058 0.50 33.79  
 ID = 3 ( 0010): 3.37 0.663 0.50 31.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
 3 + 2 = 1  
 AREA OPEAK TPEAK R. V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0010): 3.37 0.663 0.50 31.20  
 + ID2= 2 ( 0007): 2.05 0.091 0.83 9.04  
 ID = 1 ( 0010): 5.42 0.686 0.50 22.82

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
 STANDHYD ( 0005) Area (ha) = 2.30 Dir. Conn. (%) = 69.00  
 ID= 1 DT= 5.0 min Total Imp (%) = 69.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.55 0.51  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 84.06 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (mi n) = 2.02 (ii) 10.39 (ii)  
 Unit t. Tpeak (min) = 5.00 15.00  
 Unit t. Hyd. peak (cms) = 0.31 0.09  
 PEAK FLOW (cms) = 0.20 0.06 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 0.221 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 31.89  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

VO Output - EX.txt

Surface Area (ha) = 1.59 0.71  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 123.83 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 2.55 (ii) 7.05 (ii)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.29 0.14

PEAK FLOW (cms) = 0.57 0.10 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.67 0.50  
 RUNOFF VOLUME (mm) = 40.26 22.84 34.86  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\  
 d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 Total = 41.26 mm Comments: 10yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	74.27	0.75	39.61
0.17	4.95	0.50	138.63	0.83	24.76
0.25	14.85	0.58	74.27	0.92	14.85
0.33	39.61	0.67	59.41	1.00	4.95

CALIB STANDHYD ( 0006) Area (ha) = 1.17  
 ID= 1 DT= 5.0 min Total Imp(%) = 47.00 Dir. Conn. (%) = 47.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.55 0.62  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 88.32 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (min) = 2.08 (ii) 10.45 (ii)  
 Unit Hyd. Tpeak (min) = 5.00 15.00  
 Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.20 0.07 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 0.224 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 31.02  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.75

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
 Page 21

VO Output - EX.txt

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\  
 d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\F0961233  
 Total = 41.26 mm Comments: 10yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	74.27	0.75	39.61
0.17	4.95	0.50	138.63	0.83	24.76
0.25	14.85	0.58	74.27	0.92	14.85
0.33	39.61	0.67	59.41	1.00	4.95

CALIB STANDHYD ( 0008) Area (ha) = 14.26  
 ID= 1 DT= 5.0 min Total Imp(%) = 50.00 Dir. Conn. (%) = 50.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 7.13 7.13  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 308.33 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (min) = 4.41 (ii) 12.78 (ii)  
 Unit Hyd. Tpeak (min) = 5.00 15.00  
 Unit Hyd. peak (cms) = 0.23 0.08

PEAK FLOW (cms) = 2.24 0.76 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 2.461 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 31.55  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.76

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3	2.30	0.619	0.50	34.86
ID1= 1 ( 0005):	1.17	0.224	0.50	31.02
+ ID2= 2 ( 0006):	1.17	0.224	0.50	31.02
ID = 3 ( 0020):	3.47	0.843	0.50	33.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
3 + 2 = 1	3.47	0.843	0.50	33.56
ID1= 3 ( 0020):	14.26	2.461	0.50	31.55
+ ID2= 2 ( 0008):	14.26	2.461	0.50	31.55
ID = 1 ( 0020):	17.73	3.304	0.50	31.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
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VO Output - EX.txt

FINISH

```

V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000
  
```

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input file name: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1n.dat

Output file name:  
 C:\Users\p001279d\AppData\Local\CI\ci\ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\50f180-67-f766-4a63-9227-05e1234bb68e\scse  
 Summary file name:  
 C:\Users\p001279d\AppData\Local\CI\ci\ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\50f180-67-f766-4a63-9227-05e1234bb68e\scse

DATE: 11/12/2019 TIME: 11:30:11

USER:

COMMENTS:

\*\* SIMULATION : 04 AES 25-yr \*\*

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\  
 d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
 Total = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

CALIB STANDHYD ( 0030) Area (ha) = 1.58  
 ID= 1 DT= 5.0 min Total Imp(%) = 56.00 Dir. Conn. (%) = 56.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 1.00 2.00  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 102.63 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 2.12 (ii) 9.67 (ii)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.31 0.11

PEAK FLOW (cms) = 0.39 0.11 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.67 0.444 (iii)  
 RUNOFF VOLUME (mm) = 48.17 29.51 39.96  
 TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60 0.81

VO Output - EX.txt

Surface Area (ha) = 0.88 0.70  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 102.63 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (min) = 5.00 10.00  
 Storage Coeff. (min) = 2.12 (ii) 9.67 (ii)  
 Unit Hyd. Tpeak (min) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.31 0.11

PEAK FLOW (cms) = 0.39 0.11 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.67 0.444 (iii)  
 RUNOFF VOLUME (mm) = 48.17 29.51 39.96  
 TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\  
 d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
 Total = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

CALIB NASHYD ( 0007) Area (ha) = 2.05  
 ID= 1 DT= 5.0 min Ia (mm) = 5.00 # of Li near Res. (N) = 3.00  
 U. H. Tp (hrs) = 0.20

Unit Hyd Opeak (cms) = 0.391

PEAK FLOW (cms) = 0.128 (i)  
 TIME TO PEAK (hrs) = 0.750  
 RUNOFF VOLUME (mm) = 12.725  
 TOTAL RAINFALL (mm) = 49.170  
 RUNOFF COEFFICIENT = 0.259

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\  
 d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
 Total = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

VO Output - EX.txt

CALIB  
STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha) = 1.04  
Total Imp(%) = 48.00 Dir. Conn.(%) = 48.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.50	0.54
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	83.27	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 165.21  
over (min) = 5.00  
Storage Coeff. (mi n) = 1.87 (ii) 9.42 (iii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.32 0.12

PEAK FLOW (cms) = 0.22 0.09  
TIME TO PEAK (hrs) = 0.50 0.67 0.50 0.266 (iii)  
RUNOFF VOLUME (mm) = 48.17 29.51 38.46  
TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.60 0.78

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
Total = 49.17 mm  
Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB  
STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha) = 1.05  
Total Imp(%) = 41.00 Dir. Conn.(%) = 41.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.43	0.62
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	83.67	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 165.21  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 1.88 (ii) 9.43 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.32 0.12

PEAK FLOW (cms) = 0.19 0.10  
TIME TO PEAK (hrs) = 0.50 0.67 0.50 0.242 (iii)  
RUNOFF VOLUME (mm) = 48.17 29.51 37.16  
TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.60 0.76

VO Output - EX.txt

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
Total = 49.17 mm  
Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB  
STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha) = 1.06  
Total Imp(%) = 52.00 Dir. Conn.(%) = 52.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.55	0.51
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	84.06	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 165.21  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 1.88 (ii) 9.43 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.32 0.12

PEAK FLOW (cms) = 0.24 0.08  
TIME TO PEAK (hrs) = 0.50 0.67 0.50 0.287 (iii)  
RUNOFF VOLUME (mm) = 48.17 29.51 39.21  
TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.60 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
Total = 49.17 mm  
Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

VO Output - EX.txt

CALIB  
STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha) = 0.22  
Total Imp(%) = 63.00 Dir. Conn.(%) = 63.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.14	0.08
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	38.30	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 165.21  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 1.18 (ii) 5.84 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.33 0.15

PEAK FLOW (cms) = 0.06 0.02  
TIME TO PEAK (hrs) = 0.50 0.58 0.50 0.072 (iii)  
RUNOFF VOLUME (mm) = 48.17 29.51 41.26  
TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.60 0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

ID	DT	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1	1 ( 0001)	1.04	0.266	0.50	38.46
2	2 ( 0002)	1.05	0.242	0.50	37.16
3	3 ( 0010)	2.09	0.509	0.50	37.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 =

ID	DT	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1	3 ( 0010)	2.09	0.509	0.50	37.81
2	2 ( 0003)	1.06	0.287	0.50	39.21
3	1 ( 0010)	3.15	0.795	0.50	38.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

ID	DT	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1	1 ( 0010)	3.15	0.795	0.50	38.28
2	2 ( 0004)	0.22	0.072	0.50	41.26
3	3 ( 0010)	3.37	0.867	0.50	38.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

ID	DT	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1	3 ( 0010)	3.15	0.795	0.50	38.28
2	2 ( 0004)	0.22	0.072	0.50	41.26
3	1 ( 0010)	3.37	0.867	0.50	38.47

VO Output - EX.txt

ID= 1 ( 0010): 5.42 0.903 0.50 28.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
Total = 49.17 mm  
Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB  
STANDHYD ( 0005)  
ID= 1 DT= 5.0 min

Area (ha) = 2.30  
Total Imp(%) = 69.00 Dir. Conn.(%) = 69.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	1.59	0.71
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	123.83	40.00
Mannings n	0.013	0.250

Max. Eff. Inten. (mm/hr) = 165.21  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 2.38 (ii) 6.57 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.30 0.14

PEAK FLOW (cms) = 0.68 0.13  
TIME TO PEAK (hrs) = 0.50 0.58 0.50 0.75 (iii)  
RUNOFF VOLUME (mm) = 48.17 29.51 42.38  
TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.60 0.86

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
Total = 49.17 mm  
Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

VO Output - EX.txt

CALIB  
STANDHYD ( 0006)  
ID= 1 DT= 5.0 min

Area (ha)= 1.17  
Total Imp(%)= 47.00 Dir. Conn.(%)= 47.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.55 0.62  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 88.32 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 165.21 84.67  
over (mi n)= 5.00 10.00  
Storage Coeff. (mi n)= 1.94 (ii) 9.48 (ii)  
Unit t Hyd. Tpeak (mi n)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.31 0.12

PEAK FLOW (cms)= 0.24 0.10  
TIME TO PEAK (hrs)= 0.50 0.67  
RUNOFF VOLUME (mm)= 48.17 29.51  
TOTAL RAINFALL (mm)= 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.60

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 49.17 mm

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\7e735abf  
Comments: 25yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB  
STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha)= 14.26  
Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 7.13 7.13  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 308.33 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 165.21 84.67  
over (mi n)= 5.00 15.00  
Storage Coeff. (mi n)= 4.11 (ii) 11.65 (ii)  
Unit t Hyd. Tpeak (mi n)= 5.00 15.00  
Unit t Hyd. peak (cms)= 0.24 0.09

PEAK FLOW (cms)= 2.73 1.02  
TIME TO PEAK (hrs)= 0.50 0.75  
RUNOFF VOLUME (mm)= 48.17 29.51  
TOTAL RAINFALL (mm)= 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.60

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

VO Output - EX.txt

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0005):	2.30	0.757	42.38
+ ID2= 2 ( 0006):	1.17	0.294	0.50
ID = 3 ( 0020):	3.47	1.051	0.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0020):	3.47	1.051	0.50
+ ID2= 2 ( 0008):	14.26	3.048	0.50
ID = 1 ( 0020):	17.73	4.099	0.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
V V I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M O O O T M  
0 0 T T H H Y Y M M O O O  
0 0 T T H H Y Y M M O O O  
000 T T T H H Y Y M M O O O

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input file name: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output file name:  
C:\Users\p001279d\AppData\Local\Civica\WH5\b8cccece0-2cc7-4973-8ab8-dd0de50b3a28\5067d5da-B166-4a50-be11-6e96f9944fcf\scce

Summary file name:  
C:\Users\p001279d\AppData\Local\Civica\WH5\b8cccece0-2cc7-4973-8ab8-dd0de50b3a28\5067d5da-B166-4a50-be11-6e96f9944fcf\scce

DATE: 11/12/2019 TIME: 11:30:11

USER:

COMMENTS:

VO Output - EX.txt

\*\* SIMULATION : 05 AES 50-yr

READ STORM  
Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0030)  
ID= 1 DT= 5.0 min

Area (ha)= 1.58  
Total Imp(%)= 56.00 Dir. Conn.(%)= 56.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.88 0.70  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 102.63 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 184.63 99.26  
over (mi n)= 5.00 10.00  
Storage Coeff. (mi n)= 2.03 (ii) 9.11 (ii)  
Unit t Hyd. Tpeak (mi n)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.31 0.12

PEAK FLOW (cms)= 0.43 0.13  
TIME TO PEAK (hrs)= 0.50 0.67  
RUNOFF VOLUME (mm)= 53.95 34.54  
TOTAL RAINFALL (mm)= 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
NASHYD ( 0007)  
ID= 1 DT= 5.0 min

Area (ha)= 2.05 Curve Number (CN)= 70.0  
Ia (mm)= 5.00 # of Linear Res. (N)= 3.00  
U.H. Tp(hrs)= 0.20

VO Output - EX.txt

Unit Hyd Opeak (cms)= 0.391

PEAK FLOW (cms)= 0.158 (i)  
TIME TO PEAK (hrs)= 0.750  
RUNOFF VOLUME (mm)= 15.681  
TOTAL RAINFALL (mm)= 64.950  
RUNOFF COEFFICIENT = 0.285

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha)= 1.04  
Total Imp(%)= 48.00 Dir. Conn.(%)= 48.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.50 0.54  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.27 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 184.63 99.26  
over (mi n)= 5.00 10.00  
Storage Coeff. (mi n)= 1.79 (ii) 8.87 (ii)  
Unit t Hyd. Tpeak (mi n)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.25 0.10  
TIME TO PEAK (hrs)= 0.50 0.67  
RUNOFF VOLUME (mm)= 53.95 34.54  
TOTAL RAINFALL (mm)= 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha) = 1.05  
Total Imp(%) = 41.00 Dir. Conn.(%) = 41.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.43	0.62
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	83.67	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	184.63	99.26
over (min)	5.00	10.00
Storage Coeff. (min)	1.80 (ii)	8.88 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.32	0.12

PEAK FLOW (cms) = 0.27 0.12 \*TOTALS\*  
TIME TO PEAK (hrs) = 0.50 0.67 0.279 (iii)  
RUNOFF VOLUME (mm) = 53.95 34.54 42.49  
TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 54.95 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha) = 1.06  
Total Imp(%) = 52.00 Dir. Conn.(%) = 52.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.55	0.51
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	84.06	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	184.63	99.26
over (min)	5.00	10.00
Storage Coeff. (min)	1.80 (ii)	8.88 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.32	0.12

PEAK FLOW (cms) = 0.27 0.10 \*TOTALS\*  
TIME TO PEAK (hrs) = 0.50 0.67 0.327 (iii)  
RUNOFF VOLUME (mm) = 53.95 34.54 44.63  
TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63 0.81

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3 + 2 = 1

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0010):	2.09	0.583	0.50 43.17
+ ID2= 2 ( 0003):	1.06	0.327	0.50 44.63
ID = 1 ( 0010):	3.15	0.910	0.50 43.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0010):	3.15	0.910	0.50 43.66
+ ID2= 2 ( 0004):	0.22	0.081	0.50 46.76
ID = 3 ( 0010):	3.37	0.992	0.50 43.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0010):	3.37	0.92	0.50 43.86
+ ID2= 2 ( 0007):	2.05	0.158	0.5 15.68
ID = 1 ( 0010):	5.42	1.038	0.50 33.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
Total = 54.95 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0005)  
ID= 1 DT= 5.0 min

Area (ha) = 2.30  
Total Imp(%) = 69.00 Dir. Conn.(%) = 69.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	1.55	0.71
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	123.83	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	184.63	99.26
over (min)	5.00	10.00
Storage Coeff. (min)	2.27 (ii)	6.28 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.30	0.15

PEAK FLOW (cms) = 0.77 0.15 \*TOTALS\*  
TIME TO PEAK (hrs) = 0.50 0.58 0.858 (iii)  
RUNOFF VOLUME (mm) = 53.95 34.54 47.93  
TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63 0.87

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

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\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 54.95 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha) = 0.22  
Total Imp(%) = 63.00 Dir. Conn.(%) = 63.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.14	0.08
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	38.30	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	184.63	99.26
over (min)	5.00	10.00
Storage Coeff. (min)	1.12 (ii)	5.59 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.34	0.16

PEAK FLOW (cms) = 0.07 0.02 \*TOTALS\*  
TIME TO PEAK (hrs) = 0.50 0.58 0.50  
RUNOFF VOLUME (mm) = 53.95 34.54 46.76  
TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63 0.85

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0001):	1.04	0.305	0.50 43.85
+ ID2= 2 ( 0002):	1.05	0.50	0.50 42.49
ID = 3 ( 0010):	2.09	0.583	0.50 43.17

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
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(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 54.95 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0006)  
ID= 1 DT= 5.0 min

Area (ha) = 1.17  
Total Imp(%) = 47.00 Dir. Conn.(%) = 47.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	0.55	0.62
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	2.00
Length (m)	88.32	40.00
Mannings n	0.013	0.250
Max. Eff. Inten. (mm/hr)	184.63	99.26
over (min)	5.00	10.00
Storage Coeff. (min)	1.86 (ii)	8.93 (ii)
Unit Hyd. Tpeak (min)	5.00	10.00
Unit Hyd. peak (cms)	0.32	0.12

PEAK FLOW (cms) = 0.27 0.12 \*TOTALS\*  
TIME TO PEAK (hrs) = 0.50 0.67 0.337 (iii)  
RUNOFF VOLUME (mm) = 53.95 34.54 43.66  
TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63 0.79

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 54.95 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\ac308a9d  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

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VO Output - EX.txt

CALIB  
STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha)= 14.26  
Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 7.13 7.13  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 308.33 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 184.63 99.26  
over (min)= 5.00 15.00  
Storage Coeff. (min)= 3.93 (ii) 11.01 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 15.00  
Unit t Hyd. peak (cms)= 0.24 0.09

PEAK FLOW (cms)= 3.09 1.22 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.75 3.489 (iii)  
RUNOFF VOLUME (mm)= 53.95 34.54 44.24  
TOTAL RAINFALL (mm)= 54.95 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0005):	2.30	0.858	0.50	47.93
+ ID2= 2 ( 0006):	1.17	0.337	0.50	43.66
ID = 3 ( 0020):	3.47	1.195	0.50	46.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0020):	3.47	1.195	0.50	46.49
+ ID2= 2 ( 0008):	14.26	3.489	0.50	44.24
ID = 1 ( 0020):	17.73	4.684	0.50	44.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSSS UUUUU A A LLLLL

000 TTTTT TTTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T H H Y Y M M 000

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VO Output - EX.txt  
\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Temp\5263cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
e2-3cad-4473-a127-0eea7526c18c\vsce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Temp\5263cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
e2-3cad-4473-a127-0eea7526c18c\vsce

DATE: 11/12/2019 TIME: 11:30:13

USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 06 AES 100-yr \*\*\*\*\*

READ STORM  
Total = 60.87 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\5263cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB  
STANDHYD ( 0030)  
ID= 1 DT= 5.0 min

Area (ha)= 1.58  
Total Imp(%)= 56.00 Dir. Conn.(%)= 56.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.70 0.70  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 102.63 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.95 (ii) 6.76 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.31 0.14

PEAK FLOW (cms)= 0.48 0.17 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.58 0.50  
RUNOFF VOLUME (mm)= 59.87 39.79 51.03  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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VO Output - EX.txt  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 60.87 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\5263cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB  
NASHYD ( 0007)  
ID= 1 DT= 5.0 min

Area (ha)= 2.05  
Ia (mm)= 5.00  
U. H. Tp(hrs)= 0.20

Curve Number (CN)= 70.0  
# of LI near Res. (N)= 3.00

Unit t Hyd Opeak (cms)= 0.391  
PEAK FLOW (cms)= 0.191 (i)  
TIME TO PEAK (hrs)= 0.750  
RUNOFF VOLUME (mm)= 18.913  
TOTAL RAINFALL (mm)= 60.870  
RUNOFF COEFFICIENT = 0.311

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 60.87 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\5263cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB  
STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha)= 1.04  
Total Imp(%)= 48.00 Dir. Conn.(%)= 48.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.50 0.54  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.27 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.72 (ii) 8.41 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.28 0.12 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.67 0.50  
RUNOFF VOLUME (mm)= 59.87 39.79 49.42  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87

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VO Output - EX.txt  
RUNOFF COEFFICIENT = 0.98 0.65 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 60.87 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\5263cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB  
STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha)= 1.05  
Total Imp(%)= 41.00 Dir. Conn.(%)= 41.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.43 0.62  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.67 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.72 (ii) 8.41 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.24 0.14 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.67 0.50  
RUNOFF VOLUME (mm)= 59.87 39.79 48.02  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.79

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 60.87 mm

Filename: C:\Users\p001279d\AppData\Local\Temp\5263cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

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VO Output - EX.txt

CALIB  
STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha) = 1.06  
Total Imp(%) = 52.00 Dir. Conn.(%) = 52.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 0.55 0.51  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 84.06 40.00  
Mannings n = 0.013 0.250  
Max. Eff. Inten. (mm/hr) = 204.52 114.44  
over (min) = 5.00 10.00  
Storage Coeff. (min) = 1.73 (ii) 8.42 (ii)  
Unit Hyd. Tpeak (min) = 5.00 10.00  
Unit Hyd. peak (cms) = 0.32 0.12

\*TOTALS\*  
PEAK FLOW (cms) = 0.30 0.11 0.369 (iii)  
TIME TO PEAK (hrs) = 0.50 0.67 0.50  
RUNOFF VOLUME (mm) = 59.87 39.79 50.23  
TOTAL RAINFALL (mm) = 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.83

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM

Filename: C:\Users\p001279d\AppData  
ata\Local\Temp\  
d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

Total = 60.87 mm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB  
STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha) = 0.22  
Total Imp(%) = 63.00 Dir. Conn.(%) = 63.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 0.14 0.08  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 38.30 40.00  
Mannings n = 0.013 0.250  
Max. Eff. Inten. (mm/hr) = 204.52 114.44  
over (min) = 5.00 10.00  
Storage Coeff. (min) = 1.08 (ii) 5.36 (ii)  
Unit Hyd. Tpeak (min) = 5.00 10.00  
Unit Hyd. peak (cms) = 0.34 0.16

\*TOTALS\*  
PEAK FLOW (cms) = 0.08 0.02 0.091 (iii)  
TIME TO PEAK (hrs) = 0.50 0.58 0.50  
RUNOFF VOLUME (mm) = 59.87 39.79 52.43  
TOTAL RAINFALL (mm) = 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.86

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VO Output - EX.txt

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 1 ( 0001): 1.04	0.345	0.50	49.42
+ ID2 = 2 ( 0002): 1.05	0.316	0.50	48.02
*****			
ID = 3 ( 0010): 2.09	0.661	0.50	48.72

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 3 ( 0010): 2.09	0.661	0.50	48.72
+ ID2 = 2 ( 0003): 1.06	0.369	0.50	50.23
*****			
ID = 1 ( 0010): 3.15	1.030	0.50	49.23

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 1 ( 0010): 3.15	1.030	0.50	49.23
+ ID2 = 2 ( 0004): 0.22	0.091	0.50	52.43
*****			
ID = 3 ( 0010): 3.37	1.122	0.50	49.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 3 ( 0010): 3.37	1.122	0.50	49.44
+ ID2 = 2 ( 0007): 2.05	0.191	0.75	18.91
*****			
ID = 1 ( 0010): 5.42	1.179	0.50	37.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM

Filename: C:\Users\p001279d\AppData  
ata\Local\Temp\  
d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

Total = 60.87 mm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

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VO Output - EX.txt

CALIB  
STANDHYD ( 0005)  
ID= 1 DT= 5.0 min

Area (ha) = 2.30  
Total Imp(%) = 69.00 Dir. Conn.(%) = 69.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 1.59 0.71  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 123.83 40.00  
Mannings n = 0.013 0.250  
Max. Eff. Inten. (mm/hr) = 204.52 114.44  
over (min) = 5.00 10.00  
Storage Coeff. (min) = 2.18 (ii) 6.03 (ii)  
Unit Hyd. Tpeak (min) = 5.00 10.00  
Unit Hyd. peak (cms) = 0.31 0.15

\*TOTALS\*  
PEAK FLOW (cms) = 0.86 0.18 0.964 (iii)  
TIME TO PEAK (hrs) = 0.50 0.58 0.50  
RUNOFF VOLUME (mm) = 59.87 39.79 53.65  
TOTAL RAINFALL (mm) = 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.88

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM

Filename: C:\Users\p001279d\AppData  
ata\Local\Temp\  
d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

Total = 60.87 mm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB  
STANDHYD ( 0006)  
ID= 1 DT= 5.0 min

Area (ha) = 1.17  
Total Imp(%) = 47.00 Dir. Conn.(%) = 47.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 0.55 0.62  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 88.32 40.00  
Mannings n = 0.013 0.250  
Max. Eff. Inten. (mm/hr) = 204.52 114.44  
over (min) = 5.00 10.00  
Storage Coeff. (min) = 1.78 (ii) 8.42 (ii)  
Unit Hyd. Tpeak (min) = 5.00 10.00  
Unit Hyd. peak (cms) = 0.32 0.12

\*TOTALS\*  
PEAK FLOW (cms) = 0.30 0.14 0.381 (iii)  
TIME TO PEAK (hrs) = 0.50 0.67 0.50  
RUNOFF VOLUME (mm) = 59.87 39.79 49.23  
TOTAL RAINFALL (mm) = 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

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VO Output - EX.txt

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM

Filename: C:\Users\p001279d\AppData  
ata\Local\Temp\  
d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\57b9b19e  
Comments: 100yr 1hr AES

Total = 60.87 mm

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB  
STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha) = 14.26  
Total Imp(%) = 50.00 Dir. Conn.(%) = 50.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 7.13 7.13  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 308.33 40.00  
Mannings n = 0.013 0.250  
Max. Eff. Inten. (mm/hr) = 204.52 114.44  
over (min) = 5.00 15.00  
Storage Coeff. (min) = 3.77 (ii) 10.46 (ii)  
Unit Hyd. Tpeak (min) = 5.00 10.00  
Unit Hyd. peak (cms) = 0.25 0.09

\*TOTALS\*  
PEAK FLOW (cms) = 3.46 1.43 3.950 (iii)  
TIME TO PEAK (hrs) = 0.50 0.75 0.50  
RUNOFF VOLUME (mm) = 59.87 39.79 49.83  
TOTAL RAINFALL (mm) = 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 1 ( 0005): 2.30	0.964	0.50	53.65
+ ID2 = 2 ( 0006): 1.17	0.381	0.50	49.23
*****			
ID = 3 ( 0020): 3.47	1.346	0.50	52.15

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 3 ( 0020): 3.47	1.346	0.50	52.15
+ ID2 = 2 ( 0008): 0.75	0.316	0.50	48.02
*****			
ID = 1 ( 0020): 4.22	1.662	0.50	44.17

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VO Output - EX.txt  
+ ID1= 3 ( 0020): 3.47 1.346 0.50 52.15  
+ ID2= 2 ( 0008): 14.26 3.950 0.50 49.83  
+ ID= 1 ( 0020): 17.73 5.295 0.50 50.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

V V I SSSSS U U A L (v 5.2. 2003)
V V I SS U U A A L
V V I SS U U AAAAA L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y M M 000

```

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voin.dat

Output filename:  
C:\Users\p001279d\AppData\Local\CVI\ci\va\H5\B8CCE0-2CC7-4973-8AB8-DD0E50B3A28\2956fc  
b2-24ef-4092-8d54-465211437673\scce  
Summary filename:  
C:\Users\p001279d\AppData\Local\CVI\ci\va\H5\B8CCE0-2CC7-4973-8AB8-DD0E50B3A28\2956fc  
b2-24ef-4092-8d54-465211437673\scce

DATE: 11/12/2019 TIME: 11:30:10  
USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 07 AES 250yr \*\*\*\*\*

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

CALIB STANDBYD ( 0030)  
ID= 1 DT= 5.0 min  
Area (ha)= 1.58  
Total Imp(%)= 56.00 Dir. Conn. (%)= 56.00

VO Output - EX.txt

CALIB STANDBYD ( 0001)  
ID= 1 DT= 5.0 min  
Area (ha)= 1.04  
Total Imp(%)= 48.00 Dir. Conn. (%)= 48.00

IMPERVIOUS				PERVIOUS (i)			
Surface Area (ha)	=	0.50	0.54				
Dep. Storage (mm)	=	1.00	2.00				
Average Slope (%)	=	1.00	2.00				
Length (m)	=	83.27	40.00				
Mannings n	=	0.013	0.250				

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

CALIB STANDBYD ( 0002)  
ID= 1 DT= 5.0 min  
Area (ha)= 1.05  
Total Imp(%)= 41.00 Dir. Conn. (%)= 41.00

IMPERVIOUS				PERVIOUS (i)			
Surface Area (ha)	=	0.43	0.62				
Dep. Storage (mm)	=	1.00	2.00				
Average Slope (%)	=	1.00	2.00				
Length (m)	=	83.67	40.00				
Mannings n	=	0.013	0.250				

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - EX.txt  
IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.88 0.70  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 102.63 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) over (mi n)	235.37	138.35	5.00	10.00
Storage Coeff. (mi n)	1.63 (ii)	7.82 (ii)	5.00	10.00
Unit t Hyd. Tpeak (mi n)	5.00	10.00	0.32	0.15
Unit t Hyd. peak (cms)	0.32	0.15		

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

CALIB NASHYD ( 0007)  
ID= 1 DT= 5.0 min  
Area (ha)= 2.05 Curve Number (CN)= 70.0  
U. H. Tp(hrs)= 0.20 # of Linear Res. (N)= 3.00

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

VO Output - EX.txt  
RUNOFF COEFFICIENT = 0.99 0.69 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

CALIB STANDBYD ( 0003)  
ID= 1 DT= 5.0 min  
Area (ha)= 1.06  
Total Imp(%)= 52.00 Dir. Conn. (%)= 52.00

IMPERVIOUS				PERVIOUS (i)			
Surface Area (ha)	=	0.55	0.51				
Dep. Storage (mm)	=	1.00	2.00				
Average Slope (%)	=	1.00	2.00				
Length (m)	=	84.06	40.00				
Mannings n	=	0.013	0.250				

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM											
Total = 70.05 mm						Filename: C:\Users\p001279d\AppData\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000					
Comments: 250yr 1hr AES											
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41				
0.17	8.41	0.50	235.37	0.83	42.03						
0.25	25.22	0.58	126.09	0.92	25.22						
0.33	67.25	0.67	100.87	1.00	8.41						



VO Output - EX.txt

CALIB STANDHYD ( 0004) ID= 1 DT= 5.0 min			
Area Total	(ha) = 0.22	Dir. Conn. (%) = 63.00	
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	0.14	0.08	
Dep. Storage (mm)	1.00	2.00	
Average Slope (%)	1.00	2.00	
Length (m)	38.30	40.00	
Mannings n	0.013	0.250	
Max. Eff. Inten. (mm/hr) over (min)	235.37	138.35	
Storage Coeff. (min)	1.02 (ii)	5.07 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.34	0.16	
*TOTALS*			
PEAK FLOW (cms)	0.09	0.03	0.107 (iii)
TIME TO PEAK (hrs)	0.50	0.58	0.50
RUNOFF VOLUME (mm)	69.05	48.10	61.29
TOTAL RAINFALL (mm)	70.05	70.05	70.05
RUNOFF COEFFICIENT	0.99	0.69	0.87

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010) 1 + 2 = 3			
AREA OPEAK TPEAK R.V.	(ha) (cms) (hrs) (mm)		
ID1= 1 ( 0001):	1.04 0.408 0.50 58.15		
+ ID2= 2 ( 0002):	1.05 0.377 0.50 56.69		
ID = 3 ( 0010):	2.09 0.785 0.50 57.42		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1			
AREA OPEAK TPEAK R.V.	(ha) (cms) (hrs) (mm)		
ID1= 3 ( 0010):	2.09 0.785 0.50 57.42		
+ ID2= 2 ( 0003):	1.06 0.444 0.50 58.99		
ID = 1 ( 0010):	3.15 1.230 0.50 57.95		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 1 + 2 = 3			
AREA OPEAK TPEAK R.V.	(ha) (cms) (hrs) (mm)		
ID1= 1 ( 0010):	3.15 1.230 0.50 57.95		
+ ID2= 2 ( 0004):	0.22 0.107 0.50 61.29		
ID = 3 ( 0010):	3.37 1.336 0.50 58.16		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)

VO Output - EX.txt

3 + 2 = 1			
AREA OPEAK TPEAK R.V.	(ha) (cms) (hrs) (mm)		
ID1= 3 ( 0010):	3.37 1.336 0.50 58.16		
+ ID2= 2 ( 0007):	2.05 0.245 0.75 24.29		
ID = 1 ( 0010):	5.42 1.414 0.50 45.35		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM		Filename: C:\Users\p001279d\AppData	
Total = 70.05 mm		ata\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000	
Comments: 250yr 1hr AES			
TIME RAIN	TIME RAIN	TIME RAIN	TIME RAIN
hrs mm/hr	hrs mm/hr	hrs mm/hr	hrs mm/hr
0.08 0.00	0.42 126.09	0.75 67.25	1.08 8.41
0.17 8.41	0.50 235.37	0.83 42.03	
0.25 25.22	0.58 126.09	0.92 25.22	
0.33 67.25	0.67 100.87	1.00 8.41	

CALIB STANDHYD ( 0005) ID= 1 DT= 5.0 min			
Area Total	(ha) = 2.30	Dir. Conn. (%) = 69.00	
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	1.59	0.71	
Dep. Storage (mm)	1.00	2.00	
Average Slope (%)	1.00	2.00	
Length (m)	123.83	40.00	
Mannings n	0.013	0.250	
Max. Eff. Inten. (mm/hr) over (min)	235.37	138.35	
Storage Coeff. (min)	2.06 (ii)	10.00	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.31	0.15	
*TOTALS*			
PEAK FLOW (cms)	0.99	0.23	1.130 (iii)
TIME TO PEAK (hrs)	0.50	0.58	0.50
RUNOFF VOLUME (mm)	69.05	48.10	62.55
TOTAL RAINFALL (mm)	70.05	70.05	70.05
RUNOFF COEFFICIENT	0.99	0.69	0.89

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM		Filename: C:\Users\p001279d\AppData	
Total = 70.05 mm		ata\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000	
Comments: 250yr 1hr AES			
TIME RAIN	TIME RAIN	TIME RAIN	TIME RAIN
hrs mm/hr	hrs mm/hr	hrs mm/hr	hrs mm/hr
0.08 0.00	0.42 126.09	0.75 67.25	1.08 8.41
0.17 8.41	0.50 235.37	0.83 42.03	
0.25 25.22	0.58 126.09	0.92 25.22	
0.33 67.25	0.67 100.87	1.00 8.41	

VO Output - EX.txt

CALIB STANDHYD ( 0006) ID= 1 DT= 5.0 min			
Area Total	(ha) = 1.17	Dir. Conn. (%) = 47.00	
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	0.55	0.62	
Dep. Storage (mm)	1.00	2.00	
Average Slope (%)	1.00	2.00	
Length (m)	88.32	40.00	
Mannings n	0.013	0.250	
Max. Eff. Inten. (mm/hr) over (min)	235.37	138.35	
Storage Coeff. (min)	1.68 (ii)	7.88 (ii)	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.32	0.13	
*TOTALS*			
PEAK FLOW (cms)	0.35	0.17	0.452 (iii)
TIME TO PEAK (hrs)	0.50	0.58	0.50
RUNOFF VOLUME (mm)	69.05	48.10	57.94
TOTAL RAINFALL (mm)	70.05	70.05	70.05
RUNOFF COEFFICIENT	0.99	0.69	0.83

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM		Filename: C:\Users\p001279d\AppData	
Total = 70.05 mm		ata\Local\Temp\d63cf3c6-cf9e-46e4-a65f-5afb365f61f8\d9c70000	
Comments: 250yr 1hr AES			
TIME RAIN	TIME RAIN	TIME RAIN	TIME RAIN
hrs mm/hr	hrs mm/hr	hrs mm/hr	hrs mm/hr
0.08 0.00	0.42 126.09	0.75 67.25	1.08 8.41
0.17 8.41	0.50 235.37	0.83 42.03	
0.25 25.22	0.58 126.09	0.92 25.22	
0.33 67.25	0.67 100.87	1.00 8.41	

CALIB STANDHYD ( 0008) ID= 1 DT= 5.0 min			
Area Total	(ha) = 14.26	Dir. Conn. (%) = 50.00	
IMPERVIOUS PERVIOUS (i)			
Surface Area (ha)	7.13	7.13	
Dep. Storage (mm)	1.00	2.00	
Average Slope (%)	1.00	2.00	
Length (m)	308.33	40.00	
Mannings n	0.013	0.250	
Max. Eff. Inten. (mm/hr) over (min)	235.37	138.35	
Storage Coeff. (min)	3.50 (ii)	10.00	
Unit Hyd. Tpeak (min)	5.00	10.00	
Unit Hyd. peak (cms)	0.26	0.11	
*TOTALS*			
PEAK FLOW (cms)	4.04	1.83	5.075 (iii)
TIME TO PEAK (hrs)	0.50	0.67	0.50
RUNOFF VOLUME (mm)	69.05	48.10	58.58
TOTAL RAINFALL (mm)	70.05	70.05	70.05
RUNOFF COEFFICIENT	0.99	0.69	0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - EX.txt

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020) 1 + 2 = 3			
AREA OPEAK TPEAK R.V.	(ha) (cms) (hrs) (mm)		
ID1= 1 ( 0005):	2.30 1.130 0.50 62.55		
+ ID2= 2 ( 0006):	1.17 0.452 0.50 57.94		
ID = 3 ( 0020):	3.47 1.582 0.50 61.00		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020) 3 + 2 = 1			
AREA OPEAK TPEAK R.V.	(ha) (cms) (hrs) (mm)		
ID1= 3 ( 0020):	3.47 1.582 0.50 61.00		
+ ID2= 2 ( 0008):	14.26 5.075 0.50 58.58		
ID = 1 ( 0020):	17.73 6.657 0.50 59.05		

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
V V I SSSS U U A L (v 5. 2. 2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLLLL
000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 M M T T H H M M 000
```

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual\OTHYMO 5.2\VO2\voi.n.dat  
 Output filename:  
 C:\Users\p001279d\AppData\Local\Ci\ci\ca\VHS\bcccece0-2cc7-4973-8ab8-dd0de50b3a28\6e70f5  
 29-d8b8-40ca-836b-b20ced424a53\scce  
 Summary filename:  
 C:\Users\p001279d\AppData\Local\Ci\ci\ca\VHS\bcccece0-2cc7-4973-8ab8-dd0de50b3a28\6e70f5  
 29-d8b8-40ca-836b-b20ced424a53\scce

DATE: 11/12/2019 TIME: 11:30:07

USER:

COMMENTS:

VO Output - EX.txt

\*\* SIMULATION : 08 Chicago 3hr 2-yr \*\*

CHICAGO STORM
Ptotal = 33.36 mm
IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show peak flow and runoff data at various time intervals.

CALIB STANDHYD ( 0030)
ID= 1 DT= 5.0 min
Area (ha) = 1.58
Total Imp(%) = 56.00
Dir. Conn.(%) = 56.00

Surface Area (ha) = 0.88
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 102.63
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed hyetograph data.

Max. Eff. Inten. (mm/hr) = 83.04
over (min) = 5.00
Storage Coeff. (mi n) = 2.80
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.28

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

STANDHYD ( 0001)
ID= 1 DT= 5.0 min
Area (ha) = 1.04
Total Imp(%) = 48.00
Dir. Conn.(%) = 48.00

Surface Area (ha) = 0.50
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 83.27
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed hyetograph data.

Max. Eff. Inten. (mm/hr) = 83.04
over (min) = 5.00
Storage Coeff. (mi n) = 2.47
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.30

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM
Ptotal = 33.36 mm
IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show peak flow and runoff data.

CALIB STANDHYD ( 0002)
ID= 1 DT= 5.0 min
Area (ha) = 1.05
Total Imp(%) = 41.00
Dir. Conn.(%) = 41.00

Surface Area (ha) = 0.43
Impervious (%) = 0.62

VO Output - EX.txt

CHICAGO STORM
Ptotal = 33.36 mm

IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show peak flow and runoff data.

CALIB STANDHYD ( 0007)
ID= 1 DT= 5.0 min
Area (ha) = 2.05
Total Imp(%) = 5.00
Dir. Conn.(%) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed hyetograph data.

Unit Hyd. Tpeak (cms) = 0.391

PEAK FLOW (cms) = 0.037 (i)
TIME TO PEAK (hrs) = 1.67
RUNOFF VOLUME (mm) = 5.852
TOTAL RAINFALL (mm) = 33.364
RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM
Ptotal = 33.36 mm

IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show peak flow and runoff data.

CALIB

STANDHYD ( 0001)
ID= 1 DT= 5.0 min
Area (ha) = 1.04
Total Imp(%) = 48.00
Dir. Conn.(%) = 48.00

Surface Area (ha) = 0.50
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 83.27
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed hyetograph data.

Max. Eff. Inten. (mm/hr) = 83.04
over (min) = 5.00
Storage Coeff. (mi n) = 2.47
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.30

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM
Ptotal = 33.36 mm
IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show peak flow and runoff data.

CALIB STANDHYD ( 0003)
ID= 1 DT= 5.0 min
Area (ha) = 1.06
Total Imp(%) = 52.00
Dir. Conn.(%) = 52.00

Surface Area (ha) = 0.55
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 84.06
Mannings n = 0.013

VO Output - EX.txt  
 NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

Max. Eff. Inten. (mm/hr) = 83.04  
 over (min) = 5.00  
 Storage Coeff. (mi n) = 2.48 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.29

PEAK FLOW (cms) = 0.13  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 32.36  
 TOTAL RAINFALL (mm) = 0.97  
 RUNOFF COEFFICIENT = 0.49

\*TOTALS\*  
 0.139 (iii)  
 1.17  
 24.75  
 33.36  
 0.74

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 0004 )  
 ID= 1 DT= 5.0 min

Area (ha) = 0.22  
 Total Imp(%) = 63.00 Dir. Conn. (%) = 63.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.14  
 Dep. Storage (mm) = 1.00  
 Average Slope (%) = 1.00  
 Length (m) = 38.30  
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

VO Output - EX.txt  
 + ID2= 2 ( 0007 ): 2.05 0.037 1.17 5.85  
 ID= 1 ( 0010 ): 5.42 0.436 1.00 17.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 0005 )  
 ID= 1 DT= 5.0 min

Area (ha) = 2.30  
 Total Imp(%) = 69.00 Dir. Conn. (%) = 69.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 1.59  
 Dep. Storage (mm) = 1.00  
 Average Slope (%) = 1.00  
 Length (m) = 123.83  
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

Max. Eff. Inten. (mm/hr) = 83.04  
 over (min) = 5.00  
 Storage Coeff. (mi n) = 3.13 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.27

PEAK FLOW (cms) = 0.35  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 32.36  
 TOTAL RAINFALL (mm) = 0.97  
 RUNOFF COEFFICIENT = 0.49

\*TOTALS\*  
 0.373 (iii)  
 1.17  
 27.44  
 33.36  
 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - EX.txt			
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

Max. Eff. Inten. (mm/hr) = 83.04  
 over (min) = 5.00  
 Storage Coeff. (mi n) = 1.55 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.33

PEAK FLOW (cms) = 0.03  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 32.36  
 TOTAL RAINFALL (mm) = 33.36  
 RUNOFF COEFFICIENT = 0.97

\*TOTALS\*  
 0.034 (iii)  
 1.17  
 26.47  
 33.36  
 0.79

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3

AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0001 ): 1.04 0.128 1.00 24.11  
 + ID2= 2 ( 0002 ): 1.05 0.115 1.00 23.00  
 ID= 3 ( 0010 ): 2.09 0.243 1.00 23.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1

AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0010 ): 2.09 0.243 1.00 23.55  
 + ID2= 2 ( 0003 ): 1.06 0.139 1.00 24.75  
 ID= 1 ( 0010 ): 3.15 0.382 1.00 23.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3

AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0010 ): 3.15 0.382 1.00 23.96  
 + ID2= 2 ( 0004 ): 0.22 0.034 1.00 26.47  
 ID= 3 ( 0010 ): 3.37 0.417 1.00 24.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1

AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 3 ( 0010 ): 3.37 0.417 1.00 24.12

VO Output - EX.txt

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 0006 )  
 ID= 1 DT= 5.0 min

Area (ha) = 1.17  
 Total Imp(%) = 47.00 Dir. Conn. (%) = 47.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.55  
 Dep. Storage (mm) = 1.00  
 Average Slope (%) = 1.00  
 Length (m) = 88.32  
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

Max. Eff. Inten. (mm/hr) = 83.04  
 over (min) = 5.00  
 Storage Coeff. (mi n) = 2.55 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.29

PEAK FLOW (cms) = 0.12  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 32.36  
 TOTAL RAINFALL (mm) = 33.36  
 RUNOFF COEFFICIENT = 0.97

\*TOTALS\*  
 0.142 (iii)  
 1.00  
 23.95  
 33.36  
 0.72

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

VO Output - EX.txt  
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.13		
0.83	18.29	1.67	6.50	2.50	3.47		

CALIB STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha)= 14.26  
Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00

Surface Area (ha)= 7.13 IMPERVIOUS PERVIOUS (i)  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 308.33 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29	1.583	6.50	2.33	3.47
0.167	2.99	0.917	83.04	1.667	6.50	2.42	3.13
0.250	3.69	1.000	83.04	1.750	5.29	2.50	3.13
0.333	3.69	1.083	24.13	1.833	5.29	2.58	2.86
0.417	4.88	1.167	24.13	1.917	4.49	2.67	2.86
0.500	4.88	1.250	12.47	2.000	4.49	2.75	2.63
0.583	7.49	1.333	12.47	2.083	3.91	2.83	2.63
0.667	7.49	1.417	8.49	2.167	3.91	2.92	2.44
0.750	18.29	1.500	8.49	2.250	3.47	3.00	2.44

Max. Eff. Inten. (mm/hr)= 83.04 31.18  
over (min)= 5.00 20.00  
Storage Coeff. (min)= 5.41 (ii) 16.66 (ii)  
Unit Hyd. Tpeak (min)= 5.00 20.00  
Unit Hyd. peak (cms)= 0.21 0.06

PEAK FLOW (cms)= 1.44 0.35 \*TOTALS\*  
TIME TO PEAK (hrs)= 1.00 1.25 (iii)  
RUNOFF VOLUME (mm)= 32.36 16.51 24.44  
TOTAL RAINFALL (mm)= 33.36 33.36 33.36  
RUNOFF COEFFICIENT = 0.97 0.49 0.73

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES. CN\* = 90.0 Ia Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

ID= 1 ( 0005):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	2.30	0.373	1.00	27.44
+ ID2= 2 ( 0006):	1.17	0.142	1.00	23.95
-----				
ID= 3 ( 0020):	3.47	0.515	1.00	26.27

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VO Output - EX.txt

TIME	RAIN	TIME	RAIN	TIME	RAIN		
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr		
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 0030)  
ID= 1 DT= 5.0 min

Area (ha)= 1.58  
Total Imp(%)= 56.00 Dir. Conn.(%)= 56.00

Surface Area (ha)= 1.00 IMPERVIOUS PERVIOUS (i)  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 102.63 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61
0.250	4.38	1.000	112.71	1.750	6.69	2.50	3.61
0.333	4.38	1.083	36.59	1.833	6.69	2.58	3.25
0.417	6.09	1.167	36.59	1.917	5.51	2.67	3.25
0.500	6.09	1.250	17.98	2.000	5.51	2.75	2.95
0.583	10.04	1.333	17.98	2.083	4.69	2.83	2.95
0.667	10.04	1.417	11.61	2.167	4.69	2.92	2.70
0.750	27.27	1.500	11.61	2.250	4.08	3.00	2.70

Max. Eff. Inten. (mm/hr)= 112.71 64.47  
over (min)= 5.00 15.00  
Storage Coeff. (min)= 2.47 (ii) 10.89 (ii)  
Unit Hyd. Tpeak (min)= 5.00 15.00  
Unit Hyd. peak (cms)= 0.29 0.09

PEAK FLOW (cms)= 0.27 0.07 \*TOTALS\*  
TIME TO PEAK (hrs)= 1.00 1.17 (iii)  
RUNOFF VOLUME (mm)= 44.35 26.25 36.38  
TOTAL RAINFALL (mm)= 45.35 45.35 45.35  
RUNOFF COEFFICIENT = 0.98 0.58 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES. CN\* = 90.0 Ia Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
ID= 1 DT= 5.0 min

Area (ha)= 1.04  
Total Imp(%)= 48.00 Dir. Conn.(%)= 48.00

Surface Area (ha)= 0.50 IMPERVIOUS PERVIOUS (i)  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.27 40.00  
Mannings n = 0.013 0.250

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

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VO Output - EX.txt  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

ID= 3 ( 0020):	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
	3.47	0.515	1.00	26.27
+ ID2= 2 ( 0008):	14.26	1.559	1.00	24.44
-----				
ID= 1 ( 0020):	17.73	2.073	1.00	24.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
VV I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1.n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Civi.ca\VH5\b8cacece0-2cc7-4973-8ab8-dd0de50b3a2873ee1a  
93-Sece-4a4a-9789-4abe4efcdd0f5sce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civi.ca\VH5\b8cacece0-2cc7-4973-8ab8-dd0de50b3a2873ee1a  
93-Sece-4a4a-9789-4abe4efcdd0f5sce

DATE: 11/12/2019

TIME: 11:30:08

USER:

COMMENTS:

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855  
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95

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VO Output - EX.txt

CALIB NASHYD ( 0007)  
ID= 1 DT= 5.0 min

Area (ha)= 2.05  
Ia (mm)= 5.00  
U.H. Tp(hrs)= 0.20

Curve Number (CN) = 70.0  
# of Linear Res. (N) = 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61
0.250	4.38	1.000	112.71	1.750	6.69	2.50	3.61
0.333	4.38	1.083	36.59	1.833	6.69	2.58	3.25
0.417	6.09	1.167	36.59	1.917	5.51	2.67	3.25
0.500	6.09	1.250	17.98	2.000	5.51	2.75	2.95
0.583	10.04	1.333	17.98	2.083	4.69	2.83	2.95
0.667	10.04	1.417	11.61	2.167	4.69	2.92	2.70
0.750	27.27	1.500	11.61	2.250	4.08	3.00	2.70

Unit Hyd. Tpeak (cms)= 0.391

PEAK FLOW (cms)= 0.075 (i)  
TIME TO PEAK (hrs)= 1.167  
RUNOFF VOLUME (mm)= 10.889  
TOTAL RAINFALL (mm)= 45.34  
RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855  
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha)= 1.04  
Total Imp(%)= 48.00 Dir. Conn.(%)= 48.00

Surface Area (ha)= 0.50 IMPERVIOUS PERVIOUS (i)  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.27 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61

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VO Output - EX.txt  
0.250 4.38 1.000 112.71 1.750 6.69 2.50 3.61  
0.333 4.38 1.083 36.59 1.833 6.69 2.58 3.25  
0.417 6.09 1.167 36.59 1.917 5.51 2.67 3.25  
0.500 6.09 1.250 17.98 2.000 5.51 2.75 2.95  
0.583 10.04 1.333 17.98 2.083 4.69 2.83 2.95  
0.667 10.04 1.417 11.61 2.167 4.69 2.92 2.70  
0.750 27.27 1.500 11.61 2.250 4.08 3.00 2.70

Max. Eff. Inten. (mm/hr)= 112.71 64.47  
over (mi n) = 5.00 15.00  
Storage Coeff. (mi n) = 2.18 (ii) 10.59 (ii)  
Unit t Hyd. Tpeak (mi n) = 5.00 15.00  
Unit t Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.16 0.06 \*TOTALS\*  
TIME TO PEAK (hrs) = 1.00 1.17 0.199 (iii)  
RUNOFF VOLUME (mm) = 44.35 26.25 34.93  
TOTAL RAINFALL (mm) = 45.35 45.35 45.35  
RUNOFF COEFFICIENT = 0.98 0.58 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha) = 1.05  
Total Imp(%) = 41.00 Dir. Conn. (%) = 41.00

Surface Area (ha) = 0.43 IMPERVIOUS 0.62  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 83.67 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61
0.250	6.09	1.000	112.71	1.750	6.69	2.50	3.61
0.333	4.38	1.083	36.59	1.833	6.69	2.58	3.25
0.417	6.09	1.167	36.59	1.917	5.51	2.67	3.25
0.500	6.09	1.250	17.98	2.000	5.51	2.75	2.95
0.583	10.04	1.333	17.98	2.083	4.69	2.83	2.95

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VO Output - EX.txt  
0.667 10.04 1.417 11.61 2.167 4.69 2.92 2.70  
0.750 27.27 1.500 11.61 2.250 4.08 3.00 2.70

Max. Eff. Inten. (mm/hr)= 112.71 64.47  
over (mi n) = 5.00 15.00  
Storage Coeff. (mi n) = 2.19 (ii) 10.60 (ii)  
Unit t Hyd. Tpeak (mi n) = 5.00 15.00  
Unit t Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.13 0.07 \*TOTALS\*  
TIME TO PEAK (hrs) = 1.00 1.17 0.168 (iii)  
RUNOFF VOLUME (mm) = 44.35 26.25 33.67  
TOTAL RAINFALL (mm) = 45.35 45.35 45.35  
RUNOFF COEFFICIENT = 0.98 0.58 0.74

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha) = 1.06  
Total Imp(%) = 52.00 Dir. Conn. (%) = 52.00

Surface Area (ha) = 0.55 IMPERVIOUS 0.51  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 84.06 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61
0.250	4.38	1.000	112.71	1.750	6.69	2.50	3.61
0.333	4.38	1.083	36.59	1.833	6.69	2.58	3.25
0.417	6.09	1.167	36.59	1.917	5.51	2.67	3.25
0.500	6.09	1.250	17.98	2.000	5.51	2.75	2.95
0.583	10.04	1.333	17.98	2.083	4.69	2.83	2.95
0.667	10.04	1.417	11.61	2.167	4.69	2.92	2.70
0.750	27.27	1.500	11.61	2.250	4.08	3.00	2.70

Max. Eff. Inten. (mm/hr)= 112.71 64.47  
over (mi n) = 5.00 15.00

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VO Output - EX.txt  
Storage Coeff. (mi n) = 2.19 (ii) 10.61 (ii)  
Unit t Hyd. Tpeak (mi n) = 5.00 15.00  
Unit t Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.17 0.05 \*TOTALS\*  
TIME TO PEAK (hrs) = 1.00 1.17 0.199 (iii)  
RUNOFF VOLUME (mm) = 44.35 26.25 35.65  
TOTAL RAINFALL (mm) = 45.35 45.35 45.35  
RUNOFF COEFFICIENT = 0.98 0.58 0.79

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha) = 0.22  
Total Imp(%) = 63.00 Dir. Conn. (%) = 63.00

Surface Area (ha) = 0.14 IMPERVIOUS 0.08  
Dep. Storage (mm) = 1.00 2.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 38.30 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61
0.250	4.38	1.000	112.71	1.750	6.69	2.50	3.61
0.333	4.38	1.083	36.59	1.833	6.69	2.58	3.25
0.417	6.09	1.167	36.59	1.917	5.51	2.67	3.25
0.500	6.09	1.250	17.98	2.000	5.51	2.75	2.95
0.583	10.04	1.333	17.98	2.083	4.69	2.83	2.95
0.667	10.04	1.417	11.61	2.167	4.69	2.92	2.70
0.750	27.27	1.500	11.61	2.250	4.08	3.00	2.70

Max. Eff. Inten. (mm/hr)= 112.71 64.47  
over (mi n) = 5.00 10.00  
Storage Coeff. (mi n) = 1.37 (ii) 9.78 (ii)  
Unit t Hyd. Tpeak (mi n) = 5.00 10.00  
Unit t Hyd. peak (cms) = 0.33 0.11

PEAK FLOW (cms) = 0.04 0.01 \*TOTALS\*  
TIME TO PEAK (hrs) = 1.00 1.17 0.051 (iii)

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VO Output - EX.txt  
TIME TO PEAK (hrs) = 1.00 1.08 1.00  
RUNOFF VOLUME (mm) = 44.35 26.25 37.62  
TOTAL RAINFALL (mm) = 45.35 45.35 45.35  
RUNOFF COEFFICIENT = 0.98 0.58 0.83

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 ( 0001):	1.04	0.195	1.00 34.92
+ ID2= 2 ( 0002):	1.05	0.168	1.00 33.67
ID = 3 ( 0010):	2.09	0.353	1.00 34.29

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0010):	2.09	0.353	1.00 34.29
+ ID2= 2 ( 0003):	1.06	0.199	1.00 35.65
ID = 1 ( 0010):	3.15	0.552	1.00 34.75

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0010):	3.15	0.552	1.00 34.75
+ ID2= 2 ( 0004):	0.22	0.051	1.00 37.62
ID = 3 ( 0010):	3.37	0.603	1.00 34.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0010):	3.37	0.603	1.00 34.94
+ ID2= 2 ( 0007):	2.05	0.075	1.17 10.89
ID = 1 ( 0010):	5.42	0.644	1.00 25.84

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

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VO Output - EX.txt	
0.17	3.43
0.33	4.38
0.50	6.09
0.67	10.04
0.83	27.27

CALIB STANHYD ( 0005)  
ID= 1 DT= 5.0 min

Area (ha) = 2.30  
Total Imp(%) = 69.00 Dir. Conn. (%) = 69.00

IMPERVIOUS PERVIOUS (i)	
Surface Area (ha)	0.71
Dep. Storage (mm)	1.00
Average Slope (%)	1.00
Length (m)	123.83
Mannings n	0.13

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	3.43	0.833	27.27	1.583	8.50
0.167	3.43	0.917	112.71	1.667	8.50
0.250	4.38	1.000	112.71	1.750	6.69
0.333	3.43	1.083	36.59	1.833	6.69
0.417	6.09	1.167	36.59	1.917	5.51
0.500	6.09	1.250	17.98	2.000	5.51
0.583	10.04	1.333	17.98	2.083	4.69
0.667	10.04	1.417	11.61	2.167	4.69
0.750	27.27	1.500	11.61	2.250	4.08

--- TOTALS ---	
Max. Eff. Inten. (mm/hr) over (min)	112.71
Storage Coeff. (mi n)	2.77 (ii)
Unit Hyd. Tpeak (mi n)	5.00
Unit Hyd. peak (cms)	0.28
PEAK FLOW (cms)	0.49
TIME TO PEAK (hrs)	1.00
RUNOFF VOLUME (mm)	44.35
TOTAL RAINFALL (mm)	45.35
RUNOFF COEFFICIENT	0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM ID= 1 DT= 5.0 min  
Total = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	3.43	0.833	27.27	1.583	8.50
0.167	3.43	0.917	112.71	1.667	8.50
0.250	4.38	1.000	112.71	1.750	6.69
0.333	3.43	1.083	36.59	1.833	6.69
0.417	6.09	1.167	36.59	1.917	5.51
0.500	6.09	1.250	17.98	2.000	5.51
0.583	10.04	1.333	17.98	2.083	4.69
0.667	10.04	1.417	11.61	2.167	4.69
0.750	27.27	1.500	11.61	2.250	4.08

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VO Output - EX.txt

CALIB STANHYD ( 0006)  
ID= 1 DT= 5.0 min

Area (ha) = 1.17  
Total Imp(%) = 47.00 Dir. Conn. (%) = 47.00

IMPERVIOUS PERVIOUS (i)	
Surface Area (ha)	0.55
Dep. Storage (mm)	1.00
Average Slope (%)	1.00
Length (m)	88.32
Mannings n	0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	3.43	0.833	27.27	1.583	8.50
0.167	3.43	0.917	112.71	1.667	8.50
0.250	4.38	1.000	112.71	1.750	6.69
0.333	4.38	1.083	36.59	1.833	6.69
0.417	6.09	1.167	36.59	1.917	5.51
0.500	6.09	1.250	17.98	2.000	5.51
0.583	10.04	1.333	17.98	2.083	4.69
0.667	10.04	1.417	11.61	2.167	4.69
0.750	27.27	1.500	11.61	2.250	4.08

--- TOTALS ---	
Max. Eff. Inten. (mm/hr) over (min)	112.71
Storage Coeff. (mi n)	2.26 (ii)
Unit Hyd. Tpeak (mi n)	5.00
Unit Hyd. peak (cms)	0.30
PEAK FLOW (cms)	0.17
TIME TO PEAK (hrs)	1.00
RUNOFF VOLUME (mm)	44.35
TOTAL RAINFALL (mm)	45.35
RUNOFF COEFFICIENT	0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM ID= 1 DT= 5.0 min  
Total = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	3.43	0.833	27.27	1.583	8.50
0.167	3.43	0.917	112.71	1.667	8.50
0.250	4.38	1.000	112.71	1.750	6.69
0.333	3.43	1.083	36.59	1.833	6.69
0.417	6.09	1.167	36.59	1.917	5.51
0.500	6.09	1.250	17.98	2.000	5.51
0.583	10.04	1.333	17.98	2.083	4.69
0.667	10.04	1.417	11.61	2.167	4.69
0.750	27.27	1.500	11.61	2.250	4.08

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STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha) = 14.26  
Total Imp(%) = 50.00 Dir. Conn. (%) = 50.00

IMPERVIOUS PERVIOUS (i)	
Surface Area (ha)	7.13
Dep. Storage (mm)	1.00
Average Slope (%)	1.00
Length (m)	308.33
Mannings n	0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	3.43	0.833	27.27	1.583	8.50
0.167	3.43	0.917	112.71	1.667	8.50
0.250	4.38	1.000	112.71	1.750	6.69
0.333	4.38	1.083	36.59	1.833	6.69
0.417	6.09	1.167	36.59	1.917	5.51
0.500	6.09	1.250	17.98	2.000	5.51
0.583	10.04	1.333	17.98	2.083	4.69
0.667	10.04	1.417	11.61	2.167	4.69
0.750	27.27	1.500	11.61	2.250	4.08

--- TOTALS ---	
Max. Eff. Inten. (mm/hr) over (min)	112.71
Storage Coeff. (mi n)	4.79 (ii)
Unit Hyd. Tpeak (mi n)	5.00
Unit Hyd. peak (cms)	0.22
PEAK FLOW (cms)	2.02
TIME TO PEAK (hrs)	1.00
RUNOFF VOLUME (mm)	44.35
TOTAL RAINFALL (mm)	45.35
RUNOFF COEFFICIENT	0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0005):	2.30	0.561	1.00
+ ID2= 2 ( 0006):	1.17	0.205	1.00
ID = 3 ( 0020):	3.47	0.766	1.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0020):	3.47	0.766	1.00
+ ID2= 2 ( 0008):	14.26	2.365	1.00
ID = 1 ( 0020):	17.73	3.131	1.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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VO Output - EX.txt (v 5.2.2003)  
V V I SSSS U U A L  
V V I SS U U A L  
V V I SS U U A A A L  
V V I SS U U A A A L  
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T T H H Y Y M M 0 0 T  
0 0 T T T H H Y Y M M 0 0 T

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output filename: C:\Users\p001279d\AppData\Local\Civi ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\992ff5c3-d7fd-4123-8842-19820aeb601a\scce  
Summary filename: C:\Users\p001279d\AppData\Local\Civi ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\992ff5c3-d7fd-4123-8842-19820aeb601a\scce

DATE: 11/12/2019 TIME: 11:30:08  
USER:

COMMENTS:

CHI CAGO STORM ID= 1 DT= 5.0 min  
Total = 60.39 mm

IDF curve parameters: A=1496.300  
B= 8.250  
C= 0.825

used in: INTENSITY = A / (t + B)^C

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
0.083	3.43	0.833	27.27	1.583	8.50
0.167	3.43	0.917	112.71	1.667	8.50
0.250	4.38	1.000	112.71	1.750	6.69
0.333	6.10	1.083	158.06	1.833	6.94
0.417	6.10	1.167	44.35	2.000	7.51
0.500	8.21	1.250	22.12	2.125	6.49
0.583	12.90	1.333	14.74	2.208	5.73
0.667	12.90	1.417	11.10	2.292	5.14
0.750	33.21	1.500	11.10	2.375	5.14

CALIB STANHYD ( 0030)  
ID= 1 DT= 5.0 min

Area (ha) = 1.58  
Total Imp(%) = 56.00 Dir. Conn. (%) = 56.00

IMPERVIOUS PERVIOUS (i)	
Surface Area (ha)	0.88
Dep. Storage (mm)	1.00
Average Slope (%)	1.00
Length (m)	102.63
Mannings n	0.013

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NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

Max. Eff. Inten. (mm/hr) = 158.06 over (min) = 5.00 Storage Coeff. (mi n) = 2.16 (ii) Unit Hyd. Tpeak (mi n) = 5.00 Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.39 TIME TO PEAK (hrs) = 1.00 RUNOFF VOLUME (mm) = 59.39 TOTAL RAINFALL (mm) = 60.39 RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM Total = 60.39 mm IDF curve parameters: A=1496.300 B= 5.250 C= 0.825 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB STANDHYD ( 0007) Area (ha) = 2.05 Curve Number (CN) = 70.0 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

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Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

Unit Hyd Opeak (cms) = 0.391

PEAK FLOW (cms) = 0.134 (i) TIME TO PEAK (hrs) = 1.167 RUNOFF VOLUME (mm) = 18.646 TOTAL RAINFALL (mm) = 60.393 RUNOFF COEFFICIENT = 0.309

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM Total = 60.39 mm IDF curve parameters: A=1496.300 B= 5.250 C= 0.825 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB STANDHYD ( 0001) Area (ha) = 1.04 Total Imp(%) = 48.00 Dir. Conn.(%) = 48.00

Surface Area (ha) = 0.50 IMPERVIOUS PERVIOUS (i) Dep. Storage (mm) = 1.00 Average Slope (%) = 1.00 Length (m) = 83.27 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

Max. Eff. Inten. (mm/hr) = 158.06 over (mi n) = 5.00 Storage Coeff. (mi n) = 1.91 (ii) Unit Hyd. Tpeak (mi n) = 5.00 Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.22 TIME TO PEAK (hrs) = 1.00 RUNOFF VOLUME (mm) = 59.39

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TOTAL RAINFALL (mm) = 60.39 RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM Total = 60.39 mm IDF curve parameters: A=1496.300 B= 5.250 C= 0.825 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB STANDHYD ( 0002) Area (ha) = 1.05 Total Imp(%) = 41.00 Dir. Conn.(%) = 41.00

Surface Area (ha) = 0.43 IMPERVIOUS PERVIOUS (i) Dep. Storage (mm) = 1.00 Average Slope (%) = 1.00 Length (m) = 83.67 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

Max. Eff. Inten. (mm/hr) = 158.06 over (mi n) = 5.00 Storage Coeff. (mi n) = 1.91 (ii) Unit Hyd. Tpeak (mi n) = 5.00 Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.19 TIME TO PEAK (hrs) = 1.00 RUNOFF VOLUME (mm) = 59.39 TOTAL RAINFALL (mm) = 60.39 RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

TOTAL RAINFALL (mm) = 60.39 RUNOFF COEFFICIENT = 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM Total = 60.39 mm IDF curve parameters: A=1496.300 B= 5.250 C= 0.825 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB STANDHYD ( 0003) Area (ha) = 1.06 Total Imp(%) = 52.00 Dir. Conn.(%) = 52.00

Surface Area (ha) = 0.55 IMPERVIOUS PERVIOUS (i) Dep. Storage (mm) = 1.00 Average Slope (%) = 1.00 Length (m) = 84.06 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED TIME, TRANSFORMED RAIN, HYETOGRAPH TIME, HYETOGRAPH RAIN, TIME, RAIN. Rows show rainfall intensity over time.

Max. Eff. Inten. (mm/hr) = 158.06 over (mi n) = 5.00 Storage Coeff. (mi n) = 1.92 (ii) Unit Hyd. Tpeak (mi n) = 5.00 Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.24 TIME TO PEAK (hrs) = 1.00 RUNOFF VOLUME (mm) = 59.39 TOTAL RAINFALL (mm) = 60.39 RUNOFF COEFFICIENT = 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - EX.txt

CHICAGO STORM  
 Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
 B= 5.250  
 C= 0.825

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	8.94	4.66
0.33	6.10	1.17	44.35	2.00	7.51
0.50	8.21	1.33	22.12	2.17	6.49
0.67	12.90	1.50	14.74	2.33	5.73
0.83	33.21	1.67	11.10	2.50	5.14

CALIB  
 STANDHYD ( 0004)  
 ID= 1 DT= 5.0 min

Area (ha)= 0.22  
 Total Imp(%)= 63.00 Dir. Conn. (%)= 63.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.14  
 Dep. Storage (mm)= 1.00  
 Average Slope (%)= 1.00  
 Length (m)= 38.30  
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73
0.167	4.89	0.917	158.06	1.667	11.10	2.42	5.14
0.250	6.10	1.000	158.06	1.750	8.94	2.50	5.14
0.333	6.10	1.083	44.35	1.833	8.94	2.58	4.66
0.417	8.21	1.167	44.35	1.917	7.51	2.67	4.66
0.500	8.21	1.250	22.12	2.000	7.51	2.75	4.27
0.583	12.90	1.333	22.12	2.083	6.49	2.83	4.27
0.667	12.90	1.417	14.74	2.167	6.49	2.92	3.95
0.750	33.21	1.500	14.74	2.250	5.73	3.00	3.95

Max. Eff. Inten. (mm/hr) = 158.06 over (min) = 5.00  
 Storage Coeff. (mi n) = 1.20 (ii) 5.95 (iii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.33

PEAK FLOW (cms) = 0.06  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 59.39  
 TOTAL RAINFALL (mm) = 60.39  
 RUNOFF COEFFICIENT = 0.98

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN= 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
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VO Output - EX.txt

Length (m)= 123.83  
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73
0.167	4.89	0.917	158.06	1.667	11.10	2.42	5.14
0.250	6.10	1.000	158.06	1.750	8.94	2.50	5.14
0.333	6.10	1.083	44.35	1.833	8.94	2.58	4.66
0.417	8.21	1.167	44.35	1.917	7.51	2.67	4.66
0.500	8.21	1.250	22.12	2.000	7.51	2.75	4.27
0.583	12.90	1.333	22.12	2.083	6.49	2.83	4.27
0.667	12.90	1.417	14.74	2.167	6.49	2.92	3.95
0.750	33.21	1.500	14.74	2.250	5.73	3.00	3.95

Max. Eff. Inten. (mm/hr) = 158.06 over (min) = 5.00  
 Storage Coeff. (mi n) = 2.42 (ii) 6.69 (iii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.30

PEAK FLOW (cms) = 0.69  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 59.39  
 TOTAL RAINFALL (mm) = 60.39  
 RUNOFF COEFFICIENT = 0.98

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN= 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
 Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
 B= 5.250  
 C= 0.825

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	8.94	4.66
0.33	6.10	1.17	44.35	2.00	7.51
0.50	8.21	1.33	22.12	2.17	6.49
0.67	12.90	1.50	14.74	2.33	5.73
0.83	33.21	1.67	11.10	2.50	5.14

CALIB  
 STANDHYD ( 0006)  
 ID= 1 DT= 5.0 min

Area (ha)= 1.17  
 Total Imp(%)= 47.00 Dir. Conn. (%)= 47.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.55  
 Dep. Storage (mm)= 1.00  
 Average Slope (%)= 1.00  
 Length (m)= 88.32  
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

VO Output - EX.txt

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0001):	1.04	0.304	1.00	48.97
+ ID2= 2 ( 0002):	1.05	0.286	1.00	47.57
ID = 3 ( 0010):	2.09	0.590	1.00	48.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
 3 + 2 = 1

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0010):	2.09	0.590	1.00	48.27
+ ID2= 2 ( 0003):	1.06	0.321	1.00	49.78
ID = 1 ( 0010):	3.15	0.912	1.00	48.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
 1 + 2 = 3

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0010):	3.15	0.912	1.00	48.78
+ ID2= 2 ( 0004):	0.22	0.077	1.00	51.97
ID = 3 ( 0010):	3.37	0.988	1.00	48.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
 3 + 2 = 1

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0010):	3.37	0.988	1.00	48.98
+ ID2= 2 ( 0007):	2.05	0.134	1.17	18.65
ID = 1 ( 0010):	5.42	1.069	1.00	37.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHICAGO STORM  
 Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
 B= 5.250  
 C= 0.825

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
 STANDHYD ( 0005)  
 ID= 1 DT= 5.0 min

Area (ha)= 2.30  
 Total Imp(%)= 69.00 Dir. Conn. (%)= 69.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 1.59  
 Dep. Storage (mm)= 1.00  
 Average Slope (%)= 1.00

VO Output - EX.txt

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73
0.167	4.89	0.917	158.06	1.667	11.10	2.42	5.14
0.250	6.10	1.000	158.06	1.750	8.94	2.50	5.14
0.333	6.10	1.083	44.35	1.833	8.94	2.58	4.66
0.417	8.21	1.167	44.35	1.917	7.51	2.67	4.66
0.500	8.21	1.250	22.12	2.000	7.51	2.75	4.27
0.583	12.90	1.333	22.12	2.083	6.49	2.83	4.27
0.667	12.90	1.417	14.74	2.167	6.49	2.92	3.95
0.750	33.21	1.500	14.74	2.250	5.73	3.00	3.95

Max. Eff. Inten. (mm/hr) = 158.06 over (min) = 5.00  
 Storage Coeff. (mi n) = 1.97 (ii) 8.91 (iii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.24  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 59.39  
 TOTAL RAINFALL (mm) = 60.39  
 RUNOFF COEFFICIENT = 0.98

\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN= 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
 Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
 B= 5.250  
 C= 0.825

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
 STANDHYD ( 0008)  
 ID= 1 DT= 5.0 min

Area (ha)= 14.26  
 Total Imp(%)= 50.00 Dir. Conn. (%)= 50.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 7.13  
 Dep. Storage (mm)= 1.00  
 Average Slope (%)= 1.00  
 Length (m)= 308.33  
 Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73



VO Output - EX.txt table with 7 columns and 12 rows of data.

Max. Eff. Inten. (mm/hr) = 158.06
over (min) = 5.00
Storage Coeff. (mi n) = 4.18 (ii)
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.24

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020) table with 5 columns and 4 rows of data.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020) table with 5 columns and 4 rows of data.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5. 2. 2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

VO Output - EX.txt

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1.n.d

Output filename:
C:\Users\p001279d\AppData\Local\civi ca\WHS\b8ccce0-2cc7-4973-8ab8-dd0de50b3a28\243a2d
47-5b71-4da3-ad80-a1428f053ce2\5cse
Summary File name:
C:\Users\p001279d\AppData\Local\civi ca\WHS\b8ccce0-2cc7-4973-8ab8-dd0de50b3a28\243a2d
47-5b71-4da3-ad80-a1428f053ce2\5cse

DATE: 11/12/2019 TIME: 11:30:09
USER:

COMMENTS:

\*\*\* SIMULATION : 11 Chicago 3hr 100-yr \*\*\*

CHI CAGO STORM IDF curve parameters: A=1499.500
Ptotal = 71.76 mm B= 3.298
C= 0.794
used in: INTENSITY = A / ( t + B ) ^ C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rain intensity over time for different durations.

CALIB STANDHYD ( 0030) Area (ha) = 1.58
Total Imp(%) = 56.00 Dir. Conn. (%) = 56.00

Surface Area (ha) = 0.88
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 102.63
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity over time.

VO Output - EX.txt table with 7 columns and 12 rows of data.

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1499.500
Ptotal = 71.76 mm B= 3.298
C= 0.794
used in: INTENSITY = A / ( t + B ) ^ C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rain intensity over time.

CALIB NASHYD ( 0007) Area (ha) = 2.05
Curve Number (CN) = 70.0
ID = 1 DT = 5.0 min Ia (mm) = 5.00
# of Li near Res. (N) = 3.00
U. H. Tp (hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity over time.

Unit Hyd Opeak (cms) = 0.391
PEAK FLOW (cms) = 0.184 (i)
TIME TO PEAK (hrs) = 1.167
RUNOFF VOLUME (mm) = 25.328
TOTAL RAINFALL (mm) = 71.757
RUNOFF COEFFICIENT = 0.353

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - EX.txt

CHI CAGO STORM IDF curve parameters: A=1499.500
Ptotal = 71.76 mm B= 3.298
C= 0.794
used in: INTENSITY = A / ( t + B ) ^ C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rain intensity over time.

CALIB STANDHYD ( 0001) Area (ha) = 1.04
Total Imp(%) = 48.00 Dir. Conn. (%) = 48.00

Surface Area (ha) = 0.50
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 83.27
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity over time.

Max. Eff. Inten. (mm/hr) = 192.16
over (min) = 5.00
Storage Coeff. (mi n) = 1.76 (ii)
Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.32

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1499.500
Ptotal = 71.76 mm B= 3.298
C= 0.794
used in: INTENSITY = A / ( t + B ) ^ C

VO Output - EX.txt

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rainfall intensity over time.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rainfall intensity over time.

CALIB STANDHYD ( 0002) ID= 1 DT= 5.0 min

Area (ha)= 1.05
Total Imp(%)= 41.00 Dir. Conn.(%)= 41.00

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Mannings n. Values in parentheses.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed hyetograph.

Table with rows: Max. Eff. Inten., Storage Coeff., Unit Hyd. Tpeak, Unit Hyd. peak. Values in parentheses.

Table with rows: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT. Values in parentheses.

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm

IDF curve parameters: A=1499.500 B= 3.298 C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

CALIB STANDHYD ( 0003) ID= 1 DT= 5.0 min

Area (ha)= 1.06
Total Imp(%)= 52.00 Dir. Conn.(%)= 52.00

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Mannings n. Values in parentheses.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed hyetograph.

Table with rows: Max. Eff. Inten., Storage Coeff., Unit Hyd. Tpeak, Unit Hyd. peak. Values in parentheses.

Table with rows: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT. Values in parentheses.

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm

IDF curve parameters: A=1499.500 B= 3.298 C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed hyetograph.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rainfall intensity over time.

CALIB STANDHYD ( 0004) ID= 1 DT= 5.0 min

Area (ha)= 0.22
Total Imp(%)= 63.00 Dir. Conn.(%)= 63.00

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Mannings n. Values in parentheses.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed hyetograph.

Table with rows: Max. Eff. Inten., Storage Coeff., Unit Hyd. Tpeak, Unit Hyd. peak. Values in parentheses.

Table with rows: PEAK FLOW, TIME TO PEAK, RUNOFF VOLUME, TOTAL RAINFALL, RUNOFF COEFFICIENT. Values in parentheses.

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010) 1 + 2 = 3

Table with columns: AREA, OPEAK, TPEAK, R.V. Values in parentheses.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1

Table with columns: AREA, OPEAK, TPEAK, R.V. Values in parentheses.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows rainfall intensity over time.

ADD HYD ( 0010) 1 + 2 = 3

Table with columns: AREA, OPEAK, TPEAK, R.V. Values in parentheses.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1

Table with columns: AREA, OPEAK, TPEAK, R.V. Values in parentheses.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm

IDF curve parameters: A=1499.500 B= 3.298 C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed hyetograph.

CALIB STANDHYD ( 0005) ID= 1 DT= 5.0 min

Area (ha)= 2.30
Total Imp(%)= 69.00 Dir. Conn.(%)= 69.00

Table with columns: Surface Area, Dep. Storage, Average Slope, Length, Mannings n. Values in parentheses.

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed hyetograph.

VO Output - EX.txt
0.750 36.28 | 1.500 17.22 | 2.250 7.33 | 3.00 5.22
Max. Eff. Inten. (mm/hr)= 192.16 136.59
over (min)= 5.00 10.00
Storage Coeff. (min)= 2.24 (ii) 6.18 (ii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM
Ptotal = 71.76 mm
IDF curve parameters: A=1499.500
B= 3.298
C= 0.794
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB STANDHYD ( 0006)
ID= 1 DT= 5.0 min
Area (ha)= 1.17
Total Imp(%)= 47.00 Dir. Conn.(%)= 47.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.55 0.62
Dep. Storage (mm)= 1.00 2.00
Average Slope (%)= 1.00 2.00
Length (m)= 88.32 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

Max. Eff. Inten. (mm/hr)= 192.16 136.59
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.83 (ii) 8.06 (ii)

VO Output - EX.txt
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.32 0.13
PEAK FLOW (cms)= 0.29 0.16
TIME TO PEAK (hrs)= 1.00 1.08
RUNOFF VOLUME (mm)= 70.76 49.66
TOTAL RAINFALL (mm)= 71.76 71.76
RUNOFF COEFFICIENT = 0.99 0.69

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM
Ptotal = 71.76 mm
IDF curve parameters: A=1499.500
B= 3.298
C= 0.794
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB STANDHYD ( 0008)
ID= 1 DT= 5.0 min
Area (ha)= 14.26
Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 7.13 7.13
Dep. Storage (mm)= 1.00 2.00
Average Slope (%)= 1.00 2.00
Length (m)= 308.33 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

Max. Eff. Inten. (mm/hr)= 192.16 136.59
over (min)= 5.00 10.00
Storage Coeff. (min)= 3.87 (ii) 10.10 (ii)
Unit Hyd. Tpeak (min)= 5.00 15.00
Unit Hyd. peak (cms)= 0.25 0.10
PEAK FLOW (cms)= 3.57 1.54
TIME TO PEAK (hrs)= 1.00 1.17

VO Output - EX.txt
RUNOFF VOLUME (mm)= 70.76 49.66 60.21
TOTAL RAINFALL (mm)= 71.76 71.76 71.76
RUNOFF COEFFICIENT = 0.99 0.69 0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0005): 2.30 1.017 1.00 64.22
+ ID2= 2 ( 0006): 1.17 0.428 1.00 59.57
ID= 3 ( 0020): 3.47 1.445 1.00 62.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 3 ( 0020): 3.47 1.445 1.00 62.65
+ ID2= 2 ( 0008): 14.26 4.438 1.00 60.21
ID= 1 ( 0020): 17.73 5.883 1.00 60.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)
V V I SSSSS U U A L
V V I SS U UAAAA L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1n.dat
Output filename:
C:\Users\p001279d\AppData\Local\CVI\vi ca\WHS\b8ccecce0-2cc7-4973-8ab8-dd0de50b3a28\4797ce
1b-8481-4e0d-90c9-90f8281ad861\vsce
Summary filename:
C:\Users\p001279d\AppData\Local\CVI\vi ca\WHS\b8ccecce0-2cc7-4973-8ab8-dd0de50b3a28\4797ce
1b-8481-4e0d-90c9-90f8281ad861\vsce
DATE: 11/12/2019 TIME: 11:30:09
USER:

VO Output - EX.txt

COMMENTS:

\*\* SIMULATION : 12 Chicago 3hr 250-yr Regulat \*\*

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB STANDHYD ( 0030)
ID= 1 DT= 5.0 min
Area (ha)= 1.58
Total Imp(%)= 56.00 Dir. Conn.(%)= 56.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 0.88 0.70
Dep. Storage (mm)= 1.00 2.00
Average Slope (%)= 1.00 2.00
Length (m)= 102.63 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

Max. Eff. Inten. (mm/hr)= 214.13 157.21
over (min)= 5.00 10.00
Storage Coeff. (min)= 1.91 (ii) 6.64 (ii)
Unit Hyd. Tpeak (min)= 5.00 10.00
Unit Hyd. peak (cms)= 0.31 0.14
PEAK FLOW (cms)= 0.52 0.22
TIME TO PEAK (hrs)= 1.00 1.08

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

**CHI CAGO STORM**  
 Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
 B = 2.188  
 C = 0.778  
 used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

**CALIB STANDHYD ( 0007 )**  
 ID = 1 DT = 5.0 min

Area (ha) = 2.05  
 Total Imp(%) = 5.00  
 U.H. Tp(hrs) = 0.20

Curve Number (CN) = 70.0  
 # of Linear Res. (N) = 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Unit Hyd. peak (cms) = 0.391  
 PEAK FLOW (cms) = 0.215 (i)  
 TIME TO PEAK (hrs) = 1.167  
 RUNOFF VOLUME (mm) = 29.435  
 TOTAL RAINFALL (mm) = 78.293  
 RUNOFF COEFFICIENT = 0.376

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

**CHI CAGO STORM**  
 Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
 B = 2.188  
 C = 0.778  
 used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

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**CALIB STANDHYD ( 0001 )**  
 ID = 1 DT = 5.0 min

Area (ha) = 1.04  
 Total Imp(%) = 48.00  
 Dir. Conn. (%) = 48.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)	0.50	0.54
Dep. Storage	(mm)	1.00	2.00
Average Slope	(%)	1.00	2.00
Length	(m)	83.27	40.00
Mannings n		0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13  
 over (min) = 5.00  
 Storage Coeff. (mi n) = 1.69 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.30  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 77.29  
 TOTAL RAINFALL (mm) = 78.29  
 RUNOFF COEFFICIENT = 0.99

\*TOTALS\*  
 0.437 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 90.0 Ca = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

**CHI CAGO STORM**  
 Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
 B = 2.188  
 C = 0.778  
 used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

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**STANDHYD ( 0002 )**  
 ID = 1 DT = 5.0 min

Area (ha) = 1.05  
 Total Imp(%) = 41.00  
 Dir. Conn. (%) = 41.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)	0.43	0.62
Dep. Storage	(mm)	1.00	2.00
Average Slope	(%)	1.00	2.00
Length	(m)	83.67	40.00
Mannings n		0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13  
 over (min) = 5.00  
 Storage Coeff. (mi n) = 1.69 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.26  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 77.29  
 TOTAL RAINFALL (mm) = 78.29  
 RUNOFF COEFFICIENT = 0.99

\*TOTALS\*  
 0.416 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 90.0 Ca = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

**CHI CAGO STORM**  
 Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
 B = 2.188  
 C = 0.778  
 used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

**CALIB STANDHYD ( 0003 )**  
 ID = 1 DT = 5.0 min

Area (ha) = 1.06  
 Total Imp(%) = 52.00  
 Dir. Conn. (%) = 52.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)	0.55	0.51

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**CHI CAGO STORM**  
 Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
 B = 2.188  
 C = 0.778  
 used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

**CALIB STANDHYD ( 0004 )**  
 ID = 1 DT = 5.0 min

Area (ha) = 0.22  
 Total Imp(%) = 63.00  
 Dir. Conn. (%) = 63.00

		IMPERVIOUS	PERVIOUS (i)
Surface Area	(ha)	0.14	0.08
Dep. Storage	(mm)	1.00	2.00
Average Slope	(%)	1.00	2.00
Length	(m)	38.30	40.00
Mannings n		0.013	0.250

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VO Output - EX.txt  
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TRANSFORMED	HYETOGRAPH	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57
0.167	7.21	0.917	214.13	1.667	14.57
0.250	8.74	1.000	214.13	1.750	12.13
0.333	8.74	1.083	48.43	1.833	12.13
0.417	11.28	1.167	48.43	1.917	10.45
0.500	11.28	1.250	26.15	2.000	10.45
0.583	16.53	1.333	26.15	2.083	9.22
0.667	16.53	1.417	18.53	2.167	9.22
0.750	37.27	1.500	18.53	2.250	8.28

Max. Eff. Inten. (mm/hr) = 214.13  
over (min) = 5.00  
Storage Coeff. (mi n) = 1.06 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.34  
PEAK FLOW (cms) = 0.08  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 0.99  
RUNOFF COEFFICIENT = 0.99

157.21  
10.00  
5.27 (ii)  
10.00  
0.16  
0.03  
1.08  
69.29  
78.29  
0.71  
0.88

\*TOTALS\*  
0.107 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010 )	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 1	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0001 ): + ID2= 2 ( 0002 ): ID= 3 ( 0010 ): 2.09 0.853 1.00 65.30	1.04	0.437	1.00	66.06
2.09	0.416	1.00	64.54	
2.09	0.853	1.00	65.30	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )	AREA	OPEAK	TPEAK	R. V.
3 + 2 = 1	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0001 ): + ID2= 2 ( 0003 ): ID= 3 ( 0010 ): 3.15 1.312 1.00 65.84	1.04	0.437	1.00	66.06
1.05	0.416	1.00	64.54	
3.15	1.312	1.00	65.84	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )	AREA	OPEAK	TPEAK	R. V.
1 + 2 = 3	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0010 ): + ID2= 2 ( 0004 ): ID= 3 ( 0010 ): 3.37 1.419 1.00 66.07	3.15	1.312	1.00	65.84
0.22	0.107	1.00	69.29	
3.37	1.419	1.00	66.07	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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VO Output - EX.txt  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(i) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Total = 78.29 mm  
IDF curve parameters: A=1498.100 B= 2.188 C= 0.778  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TRANSFORMED	HYETOGRAPH	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	0.833	37.27	1.83	12.13
0.33	8.74	1.17	48.43	2.00	10.45
0.50	11.28	1.33	26.15	2.17	9.22
0.67	16.53	1.50	18.53	2.33	8.28
0.83	37.27	1.67	14.57	2.50	7.53

CALIB STANDHYD ( 0006 ) ID= 1 DT= 5.0 min  
Area (ha) = 2.09 Total Imp(%) = 47.00 Dir. Conn. (%) = 47.00  
Surface Area (ha) = 0.55 IMPERVIOUS PERVIOUS (i)  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 88.32  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TRANSFORMED	HYETOGRAPH	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57
0.167	7.21	0.917	214.13	1.667	14.57
0.250	8.74	1.000	214.13	1.750	12.13
0.333	8.74	1.083	48.43	1.833	12.13
0.417	11.28	1.167	48.43	1.917	10.45
0.500	11.28	1.250	26.15	2.000	10.45
0.583	16.53	1.333	26.15	2.083	9.22
0.667	16.53	1.417	18.53	2.167	9.22
0.750	37.27	1.500	18.53	2.250	8.28

Max. Eff. Inten. (mm/hr) = 214.13  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 1.75 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.32  
PEAK FLOW (cms) = 0.33  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 0.99  
RUNOFF COEFFICIENT = 0.99

157.21  
10.00  
7.64 (ii)  
10.00  
0.18  
1.08  
65.84  
78.29  
0.71  
0.84

\*TOTALS\*  
0.486 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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VO Output - EX.txt

ADD HYD ( 0010 )	AREA	OPEAK	TPEAK	R. V.
3 + 2 = 1	(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0010 ): + ID2= 2 ( 0007 ): ID= 1 ( 0010 ): 5.42 1.560 1.00 52.21	3.37	1.419	1.00	66.07
2.05	0.215	1.17	29.43	
5.42	1.560	1.00	52.21	

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM Total = 78.29 mm  
IDF curve parameters: A=1498.100 B= 2.188 C= 0.778  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13
0.33	8.74	1.17	48.43	2.00	10.45
0.50	11.28	1.33	26.15	2.17	9.22
0.67	16.53	1.50	18.53	2.33	8.28
0.83	37.27	1.67	14.57	2.50	7.53

CALIB STANDHYD ( 0005 )	Area	(ha)	Total Imp(%)	Dir. Conn. (%)
ID= 1 DT= 5.0 min	2.30	2.30	69.00	69.00

Surface Area (ha) = 1.59  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 123.83  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TRANSFORMED	HYETOGRAPH	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57
0.167	7.21	0.917	214.13	1.667	14.57
0.250	8.74	1.000	214.13	1.750	12.13
0.333	8.74	1.083	48.43	1.833	12.13
0.417	11.28	1.167	48.43	1.917	10.45
0.500	11.28	1.250	26.15	2.000	10.45
0.583	16.53	1.333	26.15	2.083	9.22
0.667	16.53	1.417	18.53	2.167	9.22
0.750	37.27	1.500	18.53	2.250	8.28

Max. Eff. Inten. (mm/hr) = 214.13  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 2.14 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.31  
PEAK FLOW (cms) = 0.94  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 0.99  
RUNOFF COEFFICIENT = 0.99

157.21  
10.00  
5.27 (ii)  
10.00  
0.15  
1.00  
70.60  
78.29  
0.90

\*TOTALS\*  
1.145 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
Page 98

VO Output - EX.txt

CHI CAGO STORM Total = 78.29 mm  
IDF curve parameters: A=1498.100 B= 2.188 C= 0.778  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13
0.33	8.74	1.17	48.43	2.00	10.45
0.50	11.28	1.33	26.15	2.17	9.22
0.67	16.53	1.50	18.53	2.33	8.28
0.83	37.27	1.67	14.57	2.50	7.53

CALIB STANDHYD ( 0008 )	Area	(ha)	Total Imp(%)	Dir. Conn. (%)
ID= 1 DT= 5.0 min	14.26	14.26	50.00	50.00

Surface Area (ha) = 7.13  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 308.33  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TRANSFORMED	HYETOGRAPH	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57
0.167	7.21	0.917	214.13	1.667	14.57
0.250	8.74	1.000	214.13	1.750	12.13
0.333	8.74	1.083	48.43	1.833	12.13
0.417	11.28	1.167	48.43	1.917	10.45
0.500	11.28	1.250	26.15	2.000	10.45
0.583	16.53	1.333	26.15	2.083	9.22
0.667	16.53	1.417	18.53	2.167	9.22
0.750	37.27	1.500	18.53	2.250	8.28

Max. Eff. Inten. (mm/hr) = 214.13  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 3.70 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.25  
PEAK FLOW (cms) = 4.00  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 0.99  
RUNOFF COEFFICIENT = 0.99

157.21  
10.00  
9.59 (ii)  
10.00  
0.11  
1.93  
1.08  
66.49  
78.29  
0.85

\*TOTALS\*  
5.630 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020 )  
1 + 2 = 3  
AREA OPEAK TPEAK R. V.  
Page 100

----- VO Output - EX.txt -----

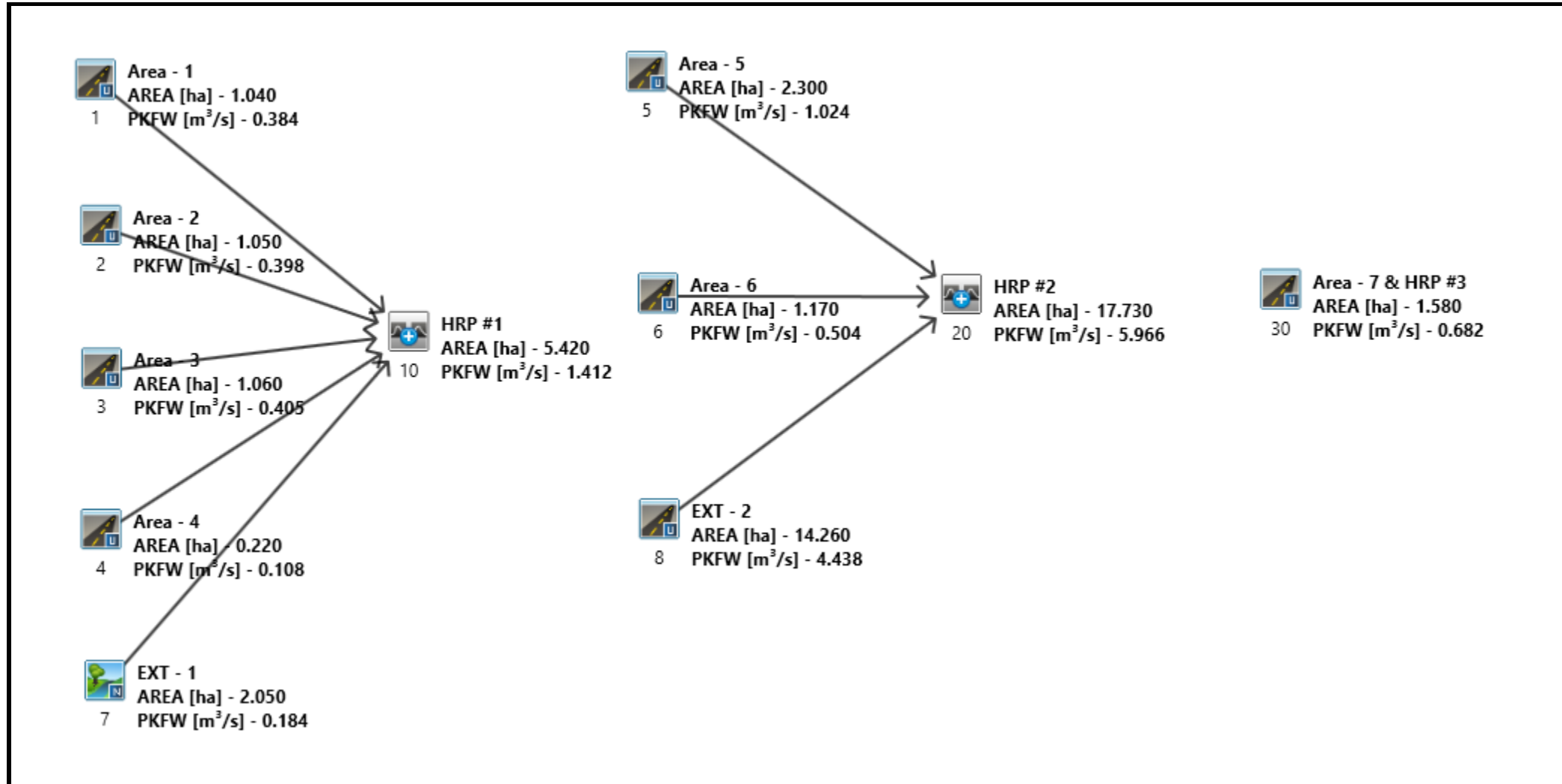
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0005):	2.30	1.145	1.00	70.60
+ ID2= 2 ( 0006):	1.17	0.486	1.00	65.84
-----				
ID = 3 ( 0020):	3.47	1.632	1.00	68.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
3 + 2 = 1				
ID1= 3 ( 0020):	3.47	1.632	1.00	68.99
+ ID2= 2 ( 0008):	14.26	5.530	1.00	66.49
-----				
ID = 1 ( 0020):	17.73	7.261	1.00	66.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

## Visual OTTHYMO Modelling Schematic-2 – Road Drainage Proposed Condition



VO Output - Prop.txt

```

-----
V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W V I SSSSS UUUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M 0 0
O O T T H H Y Y M M 0 0
000 T T H H Y Y M M 000
Developed and Distributed by Civica Infrastructure
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All rights reserved.

```

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\W02\voi.n.dat  
Output filename:  
C:\Users\p001279d\AppData\Local\CVI\ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\bafac7aa-4959-4b46-8cc1-30ffc58da839\svce  
Summary file name:  
C:\Users\p001279d\AppData\Local\CVI\ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\bafac7aa-4959-4b46-8cc1-30ffc58da839\svce  
DATE: 11/12/2019 TIME: 11:34:06  
USER:

COMMENTS:

```

-----
***** SIMULATION : 01 AES 2-yr *****
-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN
hrs  mm/hr  hrs  mm/hr  hrs  mm/hr  hrs  mm/hr
0.08 0.00    0.42 61.25  0.75 32.67  1.08 4.08
0.17 4.08    0.50 114.34 0.83 20.42
0.25 12.25  0.58 61.25  0.92 12.25
0.33 32.67  0.67 49.00  1.00 4.08

```

```

-----
CALIB STANDHYD ( 0030)  Area (ha) = 1.58 Dir. Conn. (%) = 65.00
ID= 1 DT= 5.0 min
Total Imp(%) = 65.00

Surface Area (ha) = 1.03 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 0.55
Average Slope (%) = 1.00 2.00
Length (m) = 102.63 40.00
Mannings n = 0.013 0.250
Page 1

```

```

-----
VO Output - Prop.txt
Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 15.00
Storage Coeff. (mi n) = 2.46 (ii) 11.90 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 15.00
Unit Hyd. peak (cms) = 0.30 0.09

PEAK FLOW (cms) = 0.30 0.05
TIME TO PEAK (hrs) = 0.50 0.75
RUNOFF VOLUME (mm) = 33.03 17.03
TOTAL RAINFALL (mm) = 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50

*TOTALS*
0.317 (iii)

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN
hrs  mm/hr  hrs  mm/hr  hrs  mm/hr  hrs  mm/hr
0.08 0.00    0.42 61.25  0.75 32.67  1.08 4.08
0.17 4.08    0.50 114.34 0.83 20.42
0.25 12.25  0.58 61.25  0.92 12.25
0.33 32.67  0.67 49.00  1.00 4.08

```

```

-----
CALIB STANDHYD ( 0007)  Area (ha) = 2.05 Curve Number (CN) = 70.0
ID= 1 DT= 5.0 min      Ia (mm) = 5.00 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

Unit Hyd. Tpeak (cms) = 0.391

PEAK FLOW (cms) = 0.062 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 6.100
TOTAL RAINFALL (mm) = 34.030
RUNOFF COEFFICIENT = 0.179

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN
hrs  mm/hr  hrs  mm/hr  hrs  mm/hr  hrs  mm/hr
0.08 0.00    0.42 61.25  0.75 32.67  1.08 4.08
0.17 4.08    0.50 114.34 0.83 20.42
0.25 12.25  0.58 61.25  0.92 12.25
0.33 32.67  0.67 49.00  1.00 4.08

```

```

-----
CALIB STANDHYD ( 0001)  Area (ha) = 1.04 Dir. Conn. (%) = 48.00
ID= 1 DT= 5.0 min
Total Imp(%) = 48.00
Page 2

```

```

-----
VO Output - Prop.txt
Surface Area (ha) = 0.50 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 0.54
Average Slope (%) = 1.00 2.00
Length (m) = 83.27 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 15.00
Storage Coeff. (mi n) = 2.17 (ii) 11.61 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 15.00
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.15 0.04 0.163 (iii)
TIME TO PEAK (hrs) = 0.50 0.75 0.50
RUNOFF VOLUME (mm) = 33.03 17.03 24.70
TOTAL RAINFALL (mm) = 34.03 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50 0.73

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN
hrs  mm/hr  hrs  mm/hr  hrs  mm/hr  hrs  mm/hr
0.08 0.00    0.42 61.25  0.75 32.67  1.08 4.08
0.17 4.08    0.50 114.34 0.83 20.42
0.25 12.25  0.58 61.25  0.92 12.25
0.33 32.67  0.67 49.00  1.00 4.08

```

```

-----
CALIB STANDHYD ( 0003)  Area (ha) = 1.06 Dir. Conn. (%) = 52.00
ID= 1 DT= 5.0 min
Total Imp(%) = 52.00

Surface Area (ha) = 0.55 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 0.51
Average Slope (%) = 1.00 2.00
Length (m) = 84.06 40.00
Mannings n = 0.013 0.250

```

```

-----
Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 15.00
Storage Coeff. (mi n) = 2.18 (ii) 15.00 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 15.00
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms) = 0.17 0.04 0.178 (iii)
TIME TO PEAK (hrs) = 0.50 0.75 0.50
RUNOFF VOLUME (mm) = 33.03 17.03 25.34
TOTAL RAINFALL (mm) = 34.03 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50 0.74

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

```

-----
VO Output - Prop.txt
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN
hrs  mm/hr  hrs  mm/hr  hrs  mm/hr  hrs  mm/hr
0.08 0.00    0.42 61.25  0.75 32.67  1.08 4.08
0.17 4.08    0.50 114.34 0.83 20.42
0.25 12.25  0.58 61.25  0.92 12.25
0.33 32.67  0.67 49.00  1.00 4.08

```

```

-----
CALIB STANDHYD ( 0004)  Area (ha) = 0.22 IMPERVIOUS PERVIOUS (i)
ID= 1 DT= 5.0 min      Total Imp(%) = 75.00 Dir. Conn. (%) = 75.00

Surface Area (ha) = 0.17 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 2.00
Length (m) = 38.30 40.00
Mannings n = 0.013 0.250

```

```

-----
Max. Eff. Inten. (mm/hr) = 114.34 48.33
over (mi n) = 5.00 10.00
Storage Coeff. (mi n) = 1.36 (ii) 5.67 (ii)
Unit Hyd. Tpeak (mi n) = 5.00 10.00
Unit Hyd. peak (cms) = 0.33 0.15

PEAK FLOW (cms) = 0.05 0.01 0.055 (iii)
TIME TO PEAK (hrs) = 0.50 0.58 0.50
RUNOFF VOLUME (mm) = 33.03 17.03 29.02
TOTAL RAINFALL (mm) = 34.03 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.50 0.85

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

-----
READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3
Ptotal = 34.03 mm  Comments: 2yr 1hr AES

TIME RAIN   TIME RAIN   TIME RAIN   TIME RAIN
hrs  mm/hr  hrs  mm/hr  hrs  mm/hr  hrs  mm/hr
0.08 0.00    0.42 61.25  0.75 32.67  1.08 4.08
0.17 4.08    0.50 114.34 0.83 20.42
0.25 12.25  0.58 61.25  0.92 12.25
0.33 32.67  0.67 49.00  1.00 4.08

```

```

-----
CALIB STANDHYD ( 0002)  Area (ha) = 1.05 Dir. Conn. (%) = 51.00
ID= 1 DT= 5.0 min
Total Imp(%) = 51.00
Page 4

```



VO Output - Prop.txt  
 IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha)= 0.54 0.51  
 Dep. Storage (mm)= 1.00 2.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 83.67 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr)= 114.34 48.33  
 over (mi n) 5.00 15.00  
 Storage Coeff. (mi n)= 2.18 (ii) 11.62 (iii)  
 Unit t. Tpeak (mi n)= 5.00 15.00  
 Unit t. Hyd. peak (cms)= 0.31 0.09  
 \*TOTALS\*  
 PEAK FLOW (cms)= 0.16 0.04 0.173 (iii)  
 TIME TO PEAK (hrs)= 0.50 0.75  
 RUNOFF VOLUME (mm)= 33.03 17.03 25.18  
 TOTAL RAINFALL (mm)= 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.74

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3  
 ID1= 1 ( 0001 ): AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 1.04 0.163 0.50 24.70  
 + ID2= 2 ( 0002 ): 1.05 0.173 0.50 25.18  
 ID = 3 ( 0010 ): 2.09 0.336 0.50 24.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1  
 ID1= 3 ( 0010 ): AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 2.09 0.336 0.50 24.94  
 + ID2= 2 ( 0003 ): 1.06 0.178 0.50 25.34  
 ID = 1 ( 0010 ): 3.15 0.514 0.50 25.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3  
 ID1= 1 ( 0001 ): AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 3.15 0.514 0.50 25.08  
 + ID2= 2 ( 0004 ): 0.22 0.055 0.50 29.02  
 ID = 3 ( 0010 ): 3.37 0.569 0.50 25.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1  
 ID1= 3 ( 0010 ): AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 3.37 0.569 0.50 25.33  
 + ID2= 2 ( 0007 ): 2.05 0.062 0.83 6.10  
 ID = 1 ( 0010 ): 5.42 0.583 0.50 18.06

Page 5

VO Output - Prop.txt  
 Dep. Storage (mm)= 1.00 2.00  
 Average Slope (%)= 88.32 40.00  
 Length (m)= 0.013 0.250  
 Mannings n =  
 Max. Eff. Inten. (mm/hr)= 114.34 48.33  
 over (mi n) 5.00 15.00  
 Storage Coeff. (mi n)= 2.25 (ii) 11.69 (iii)  
 Unit t. Tpeak (mi n)= 5.00 15.00  
 Unit t. Hyd. peak (cms)= 0.30 0.09  
 \*TOTALS\*  
 PEAK FLOW (cms)= 0.23 0.03 0.235 (iii)  
 TIME TO PEAK (hrs)= 0.50 0.75  
 RUNOFF VOLUME (mm)= 33.03 17.03 27.26  
 TOTAL RAINFALL (mm)= 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3  
 Ptotal = 34.03 mm  
 Comments: 2yr 1hr AES  

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	61.25	0.75	32.67	1.08	4.08
0.17	4.08	0.50	114.34	0.83	20.42		
0.25	12.25	0.58	61.25	0.92	12.25		
0.33	32.67	0.67	49.00	1.00	4.08		

CALIB STANDHYD ( 0005 )  
 ID= 1 DT= 5.0 min  
 Area (ha)= 2.30  
 Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00

Surface Area (ha)= 1.61 0.69  
 Dep. Storage (mm)= 1.00 2.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 123.83 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr)= 114.34 48.33  
 over (mi n) 5.00 10.00  
 Storage Coeff. (mi n)= 2.75 (ii) 7.52 (ii)  
 Unit t. Tpeak (mi n)= 5.00 10.00  
 Unit t. Hyd. peak (cms)= 0.28 0.13  
 \*TOTALS\*  
 PEAK FLOW (cms)= 0.47 0.07 0.503 (iii)  
 TIME TO PEAK (hrs)= 0.50 0.67  
 RUNOFF VOLUME (mm)= 33.03 17.03 28.23  
 TOTAL RAINFALL (mm)= 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.83

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Page 7

VO Output - Prop.txt  
 NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3  
 Ptotal = 34.03 mm  
 Comments: 2yr 1hr AES  

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	61.25	0.75	32.67	1.08	4.08
0.17	4.08	0.50	114.34	0.83	20.42		
0.25	12.25	0.58	61.25	0.92	12.25		
0.33	32.67	0.67	49.00	1.00	4.08		

CALIB STANDHYD ( 0008 )  
 ID= 1 DT= 5.0 min  
 Area (ha)= 14.26  
 Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00

Surface Area (ha)= 7.13 7.13  
 Dep. Storage (mm)= 1.00 2.00  
 Average Slope (%)= 1.00 2.00  
 Length (m)= 308.33 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr)= 114.34 48.33  
 over (mi n) 5.00 15.00  
 Storage Coeff. (mi n)= 4.75 (ii) 14.20 (ii)  
 Unit t. Tpeak (mi n)= 5.00 15.00  
 Unit t. Hyd. peak (cms)= 0.22 0.08  
 \*TOTALS\*  
 PEAK FLOW (cms)= 1.80 0.54 1.944 (iii)  
 TIME TO PEAK (hrs)= 0.50 0.75  
 RUNOFF VOLUME (mm)= 33.03 17.03 25.03  
 TOTAL RAINFALL (mm)= 34.03 34.03 34.03  
 RUNOFF COEFFICIENT = 0.97 0.50 0.74

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3  
 Ptotal = 34.03 mm  
 Comments: 2yr 1hr AES  

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	61.25	0.75	32.67	1.08	4.08
0.17	4.08	0.50	114.34	0.83	20.42		
0.25	12.25	0.58	61.25	0.92	12.25		
0.33	32.67	0.67	49.00	1.00	4.08		

CALIB STANDHYD ( 0006 )  
 ID= 1 DT= 5.0 min  
 Area (ha)= 1.17  
 Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

Surface Area (ha)= 0.75 0.42  
 Page 6

VO Output - Prop.txt  
 ADD HYD ( 0020 )  
 1 + 2 = 3  
 ID1= 1 ( 0005 ): AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 2.30 0.503 0.50 28.23  
 + ID2= 2 ( 0006 ): 1.17 0.235 0.50 27.26  
 ID = 3 ( 0020 ): 3.47 0.737 0.50 27.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020 )  
 3 + 2 = 1  
 ID1= 3 ( 0020 ): AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 3.47 0.737 0.50 27.90  
 + ID2= 2 ( 0008 ): 14.26 1.944 0.50 25.03  
 ID = 1 ( 0020 ): 17.73 2.682 0.50 25.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U A A A A L  
 V V I SS U U A A L  
 V V I SSSS UUUU A A LLLL  
 000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y M M 0 0  
 0 0 T T H H Y Y M M 0 0  
 000 T T H H Y Y M M 000  
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTHMMO 5.2\VO2\vo1.n.dat  
 Output filename:  
 C:\Users\p001279d\AppData\Local\Ci\ci\ca\WHS\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\556b53b8-bc0e-4146-8cb3-444b6af8d943\scse  
 Summary filename:  
 C:\Users\p001279d\AppData\Local\Ci\ci\ca\WHS\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\556b53b8-bc0e-4146-8cb3-444b6af8d943\scse

DATE: 11/12/2019 TIME: 11:34:04

USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 02 AES 5-yr \*\*\*\*\*

READ STORM  
 Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\c4dc5ad3  
 Page 8

VO Output - Prop.txt  
ata\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\51069ce4

Ptotal = 34.64 mm  
Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB  
STANDHYD ( 0030)  
ID= 1 DT= 5.0 min

Area (ha)= 1.58  
Total Imp(%)= 65.00 Dir. Conn. (%)= 65.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 1.03 0.55  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 102.63 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.39 49.73  
over (mi n) = 5.00 15.00  
Storage Coeff. (mi n) = 2.44 (ii) 11.78 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00 15.00  
Unit Hyd. peak (cms) = 0.30 0.09

PEAK FLOW (cms)= 0.31 0.05 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.75 0.323 (iii)  
RUNOFF VOLUME (mm)= 33.64 17.50 27.99  
TOTAL RAINFALL (mm)= 34.64 34.64 34.64  
RUNOFF COEFFICIENT = 0.97 0.51 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 34.64 mm  
Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB  
NASHYD ( 0007)  
ID= 1 DT= 5.0 min

Area (ha)= 2.05 Curve Number (CN)= 70.0  
U.H. Tp(hrs)= 0.20 # of Linear Res. (N)= 3.00

Unit Hyd. Peak (cms) = 0.391  
PEAK FLOW (cms) = 0.064 (i)  
TIME TO PEAK (hrs) = 0.833  
RUNOFF VOLUME (mm) = 6.331  
TOTAL RAINFALL (mm) = 34.640

VO Output - Prop.txt  
RUNOFF COEFFICIENT = 0.183  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 34.64 mm  
Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB  
STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha)= 1.04  
Total Imp(%)= 48.00 Dir. Conn. (%)= 48.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.50 0.54  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.27 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.39 49.73  
over (mi n) = 5.00 15.00  
Storage Coeff. (mi n) = 2.15 (ii) 11.49 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00 15.00  
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms)= 0.15 0.05 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.75 0.166 (iii)  
RUNOFF VOLUME (mm)= 33.64 17.50 25.24  
TOTAL RAINFALL (mm)= 34.64 34.64 34.64  
RUNOFF COEFFICIENT = 0.97 0.51 0.73

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 34.64 mm  
Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB  
STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha)= 1.06  
Total Imp(%)= 52.00 Dir. Conn. (%)= 52.00

VO Output - Prop.txt

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.55 0.51  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 84.06 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.39 49.73  
over (mi n) = 5.00 15.00  
Storage Coeff. (mi n) = 2.17 (ii) 11.50 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00 15.00  
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms)= 0.17 0.04 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.75 0.181 (iii)  
RUNOFF VOLUME (mm)= 33.64 17.50 25.89  
TOTAL RAINFALL (mm)= 34.64 34.64 34.64  
RUNOFF COEFFICIENT = 0.97 0.51 0.75

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 34.64 mm  
Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB  
STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha)= 0.22  
Total Imp(%)= 75.00 Dir. Conn. (%)= 75.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.17 0.05  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 38.30 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.39 49.73  
over (mi n) = 5.00 10.00  
Storage Coeff. (mi n) = 1.35 (ii) 5.63 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00 10.00  
Unit Hyd. peak (cms) = 0.33 0.15

PEAK FLOW (cms)= 0.05 0.01 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.58 0.056 (iii)  
RUNOFF VOLUME (mm)= 33.64 17.50 29.60  
TOTAL RAINFALL (mm)= 34.64 34.64 34.64  
RUNOFF COEFFICIENT = 0.97 0.51 0.85

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

VO Output - Prop.txt  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Ptotal = 34.64 mm  
Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB  
STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha)= 1.05  
Total Imp(%)= 51.00 Dir. Conn. (%)= 51.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 1.00 2.00  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.67 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 116.39 49.73  
over (mi n) = 5.00 15.00  
Storage Coeff. (mi n) = 2.16 (ii) 11.49 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00 15.00  
Unit Hyd. peak (cms) = 0.31 0.09

PEAK FLOW (cms)= 0.16 0.04 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.75 0.177 (iii)  
RUNOFF VOLUME (mm)= 33.64 17.50 25.73  
TOTAL RAINFALL (mm)= 34.64 34.64 34.64  
RUNOFF COEFFICIENT = 0.97 0.51 0.74

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 1 ( 0001):	1.04	0.166	0.50 25.24
+ ID2 = 2 ( 0002):	1.05	0.177	0.50 25.73
ID = 3 ( 0010):	2.09	0.343	0.50 25.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 3 ( 0010):	2.09	0.343	0.50 25.49
+ ID2 = 2 ( 0003):	1.06	0.181	0.50 25.89
ID = 1 ( 0010):	3.15	0.525	0.50 25.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 1 + 2 = 3 AREA OPEAK TPEAK R.V. (mm) ID1= 1 ( 0010): 3.15 0.525 0.50 25.62 + ID2= 2 ( 0004): 0.22 0.056 0.50 29.60 ID = 3 ( 0010): 3.37 0.580 0.50 25.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1 AREA OPEAK TPEAK R.V. (mm) ID1= 3 ( 0010): 3.37 0.580 0.50 25.88 + ID2= 2 ( 0007): 2.05 0.064 0.83 6.33 ID = 1 ( 0010): 5.42 0.595 0.50 18.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData... Total = 34.64 mm Comments: 5yr 1hr AES

CALIB STANDHYD ( 0008) ID= 1 DT= 5.0 min Area Total Imp(%) = 14.26 50.00 Dir. Conn. (%) = 50.00

Surface Area (ha)= 7.13 7.13 Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 308.33 40.00 Mannings n = 0.013 0.250

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY. Page 13

READ STORM File name: C:\Users\p001279d\AppData... Total = 34.64 mm Comments: 5yr 1hr AES

CALIB STANDHYD ( 0006) ID= 1 DT= 5.0 min Area Total Imp(%) = 1.17 64.00 Dir. Conn. (%) = 64.00

Surface Area (ha)= 0.75 0.42 Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 88.32 40.00 Mannings n = 0.013 0.250

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData... Total = 34.64 mm Comments: 5yr 1hr AES

CALIB STANDHYD ( 0005) ID= 1 DT= 5.0 min Area Total Imp(%) = 2.30 70.00 Dir. Conn. (%) = 70.00

Surface Area (ha)= 1.61 0.69

Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 123.83 40.00 Mannings n = 0.013 0.250

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020) 1 + 2 = 3 AREA OPEAK TPEAK R.V. (mm) ID1= 1 ( 0005): 2.30 0.513 0.50 28.80 + ID2= 2 ( 0006): 1.17 0.239 0.50 27.83 ID = 3 ( 0020): 3.47 0.752 0.50 28.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020) 3 + 2 = 1 AREA OPEAK TPEAK R.V. (mm) ID1= 3 ( 0020): 3.47 0.752 0.50 28.47 + ID2= 2 ( 0008): 14.26 1.987 0.50 25.57 ID = 1 ( 0020): 17.73 2.739 0.50 26.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003) V V I SS U U A A L V V I SS U U A A L V V I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM 0 0 T T H H Y Y M M 0 0 0 0 T T H H Y Y M M 000

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input file name: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1n.dat

Output file name:

C:\Users\p001279d\AppData\Local\Clivi\ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\1a282510-d6fc-4d86-a91c-03f2faac763f\scce Summary File name: C:\Users\p001279d\AppData\Local\Clivi\ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\1a282510-d6fc-4d86-a91c-03f2faac763f\scce

DATE: 11/12/2019 TIME: 11:34:04

USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 03 AES 10-YR \*\*\*\*\*

READ STORM File name: C:\Users\p001279d\AppData... Total = 41.26 mm Comments: 10yr 1hr AES

CALIB STANDHYD ( 0030) ID= 1 DT= 5.0 min Area Total Imp(%) = 1.58 65.00 Dir. Conn. (%) = 65.00

Surface Area (ha)= 1.03 0.55 Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 102.63 40.00 Mannings n = 0.013 0.250

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData... Page 16

VO Output - Prop.txt  
ata\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233

Ptotal = 41.26 mm  
Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
NASHYD ( 0007)  
ID= 1 DT= 5.0 min

Area (ha) = 2.05  
Imp (%) = 5.00  
Dir. Conn. (%) = 0.20

Curve Number (CN) = 70.0  
# of Linear Res. (N) = 3.00  
U. H. Tp (hrs) = 0.20

Unit Hyd. Qpeak (cms) = 0.391

PEAK FLOW (cms) = 0.091 (i)  
TIME TO PEAK (hrs) = 0.833  
RUNOFF VOLUME (mm) = 9.043  
TOTAL RAINFALL (mm) = 41.260  
RUNOFF COEFFICIENT = 0.219

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233

Ptotal = 41.26 mm  
Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha) = 1.04  
Total Imp (%) = 48.00  
Dir. Conn. (%) = 48.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.50  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 83.27  
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 138.63  
over (min) = 5.00  
Storage Coeff. (mi n) = 2.01 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.18  
TIME TO PEAK (hrs) = 0.50  
RUNOFF VOLUME (mm) = 40.26  
TOTAL RAINFALL (mm) = 41.26  
RUNOFF COEFFICIENT = 0.98

\*TOTALS\*  
0.06 0.01 0.068 (iii)  
0.50 0.50 0.50  
22.84 31.20 41.26  
0.55 0.77 0.87

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
Page 17

VO Output - Prop.txt

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 0.17	0.05
Dep. Storage (mm) = 1.00	2.00
Average Slope (%) = 1.00	2.00
Length (m) = 38.30	40.00
Mannings n = 0.013	0.250
Max. Eff. Inten. (mm/hr) = 138.63	65.27
over (min) = 5.00	10.00
Storage Coeff. (mi n) = 1.26 (ii)	5.25 (ii)
Unit Hyd. Tpeak (mi n) = 5.00	10.00
Unit Hyd. peak (cms) = 0.33	0.16
PEAK FLOW (cms) = 0.06	0.01
TIME TO PEAK (hrs) = 0.50	0.50
RUNOFF VOLUME (mm) = 40.26	22.84
TOTAL RAINFALL (mm) = 41.26	41.26
RUNOFF COEFFICIENT = 0.98	0.55

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233

Ptotal = 41.26 mm  
Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha) = 1.05  
Total Imp (%) = 51.00  
Dir. Conn. (%) = 51.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.54  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 83.67  
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 138.63  
over (min) = 5.00  
Storage Coeff. (mi n) = 2.01 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.20  
TIME TO PEAK (hrs) = 0.50  
RUNOFF VOLUME (mm) = 40.26  
TOTAL RAINFALL (mm) = 41.26  
RUNOFF COEFFICIENT = 0.98

\*TOTALS\*  
0.219 (ii)  
0.50 0.75 31.72  
41.26 41.26 41.26  
0.55 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
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VO Output - Prop.txt  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233

Ptotal = 41.26 mm  
Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha) = 1.06  
Total Imp (%) = 52.00  
Dir. Conn. (%) = 52.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.55  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 84.06  
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 138.63  
over (min) = 5.00  
Storage Coeff. (mi n) = 2.02 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.20  
TIME TO PEAK (hrs) = 0.50  
RUNOFF VOLUME (mm) = 40.26  
TOTAL RAINFALL (mm) = 41.26  
RUNOFF COEFFICIENT = 0.98

\*TOTALS\*  
0.221 (iii)  
0.50 0.75 31.89  
41.26 41.26 41.26  
0.55 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233

Ptotal = 41.26 mm  
Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha) = 0.22  
Total Imp (%) = 75.00  
Dir. Conn. (%) = 75.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.50  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 83.27  
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 138.63  
over (min) = 5.00  
Storage Coeff. (mi n) = 2.01 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.18  
TIME TO PEAK (hrs) = 0.50  
RUNOFF VOLUME (mm) = 40.26  
TOTAL RAINFALL (mm) = 41.26  
RUNOFF COEFFICIENT = 0.98

\*TOTALS\*  
0.203 (iii)  
0.50 0.50 35.90  
22.84 31.20 41.26  
0.55 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
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THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0001):	1.04	0.203	0.50 31.20
+ ID2= 2 ( 0002):	1.05	0.216	0.50 31.72
ID = 3 ( 0010):	2.09	0.419	0.50 31.46

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 3 ( 0010):	2.09	0.419	0.50 31.46
+ ID2= 2 ( 0003):	1.06	0.221	0.50 31.89
ID = 1 ( 0010):	3.15	0.640	0.50 31.60

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0010):	3.15	0.640	0.50 31.60
+ ID2= 2 ( 0004):	0.22	0.068	0.50 35.90
ID = 3 ( 0010):	3.37	0.708	0.50 31.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 3 ( 0010):	3.37	0.708	0.50 31.88
+ ID2= 2 ( 0007):	2.05	0.091	0.83 9.04
ID = 1 ( 0010):	5.42	0.731	0.50 23.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233

Ptotal = 41.26 mm  
Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB  
STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha) = 14.26  
Total Imp (%) = 50.00  
Dir. Conn. (%) = 50.00

IMPERVIOUS PERVIOUS (i)  
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VO Output - Prop.txt

Surface Area (ha) = 7.13 7.13  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 308.33 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (min) = 5.00 15.00  
 Storage Coeff. (mi n) = 4.41 (ii) 12.78 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 15.00  
 Unit t Hyd. peak (cms) = 0.23 0.08

PEAK FLOW (cms) = 2.24 0.76 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.75 2.461 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 31.55  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.76

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	74.27	0.75	39.61
0.17	4.95	0.50	138.63	0.83	24.76
0.25	14.85	0.58	74.27	0.92	14.85
0.33	39.61	0.67	59.41	1.00	4.95

CALIB STANDHYD ( 0006) Area (ha) = 1.17  
 ID= 1 DT= 5.0 min Total Imp(%) = 64.00 Dir. Conn. (%) = 64.00

Surface Area (ha)	Dep. Storage (mm)	Average Slope (%)	Length (m)	Mannings n
0.75	1.00	1.00	88.32	0.013

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.75 0.42  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 88.32 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 2.08 (ii) 7.00 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit t Hyd. peak (cms) = 0.31 0.14

PEAK FLOW (cms) = 0.28 0.06 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.67 0.306 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 33.99  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
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VO Output - Prop.txt

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\F0961233  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	74.27	0.75	39.61
0.17	4.95	0.50	138.63	0.83	24.76
0.25	14.85	0.58	74.27	0.92	14.85
0.33	39.61	0.67	59.41	1.00	4.95

CALIB STANDHYD ( 0005) Area (ha) = 2.30  
 ID= 1 DT= 5.0 min Total Imp(%) = 70.00 Dir. Conn. (%) = 70.00

Surface Area (ha)	Dep. Storage (mm)	Average Slope (%)	Length (m)	Mannings n
1.61	1.00	1.00	123.83	0.013

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 1.61 0.69  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 123.83 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 138.63 65.27  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 2.55 (ii) 6.96 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit t Hyd. peak (cms) = 0.29 0.14

PEAK FLOW (cms) = 0.58 0.09 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.67 0.626 (iii)  
 RUNOFF VOLUME (mm) = 40.26 22.84 35.03  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.55 0.85

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0005):	2.30	0.626	0.50	35.03
+ ID2= 2 ( 0006):	1.17	0.306	0.50	33.99
=====				
ID = 3 ( 0020):	3.47	0.932	0.50	34.68

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
 3 + 2 = 1

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
+ ID1= 3 ( 0020):	3.47	0.932	0.50	34.68
+ ID2= 2 ( 0008):	14.26	2.461	0.50	31.55
=====				
ID = 1 ( 0020):	17.73	3.393	0.50	32.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
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VO Output - Prop.txt

V V I SSSSS U U A L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U A A A A L  
 V V I SS U U A A A L  
 V V I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y M M 0 0  
 0 0 T T H H Y Y M M 0 0  
 000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input File name: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output file name:  
 C:\Users\p001279d\AppData\Local\Civi.ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\ecd821  
 Of-3816-42ef-a8d4-20d865c6929c\scse  
 Summary File name:  
 C:\Users\p001279d\AppData\Local\Civi.ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\ecd821  
 Of-3816-42ef-a8d4-20d865c6929c\scse

DATE: 11/12/2019 TIME: 11:34:07

USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 04 AES 25-yr \*\*\*\*\*

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

CALIB STANDHYD ( 0030) Area (ha) = 1.58  
 ID= 1 DT= 5.0 min Total Imp(%) = 65.00 Dir. Conn. (%) = 65.00

Surface Area (ha)	Dep. Storage (mm)	Average Slope (%)	Length (m)	Mannings n
1.03	1.00	1.00	102.63	0.013

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 1.03 0.55  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 102.63 40.00  
 Mannings n = 0.013 0.250

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VO Output - Prop.txt

Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 2.12 (ii) 6.63 (ii)  
 Unit t Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit t Hyd. peak (cms) = 0.31 0.14

PEAK FLOW (cms) = 0.45 0.10 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.58 0.505 (iii)  
 RUNOFF VOLUME (mm) = 48.17 29.51 41.64  
 TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60 0.85

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

CALIB NASHYD ( 0007) Area (ha) = 2.05  
 ID= 1 DT= 5.0 min Ia (mm) = 1.00 U.H. Tp (hrs) = 0.20 Curve Number (CN) = 70.0  
 # of Linear Res. (N) = 3.00

Unit Hyd Opeak (cms) = 0.391

PEAK FLOW (cms) = 0.128 (i)  
 TIME TO PEAK (hrs) = 0.750  
 RUNOFF VOLUME (mm) = 12.725  
 TOTAL RAINFALL (mm) = 49.170  
 RUNOFF COEFFICIENT = 0.259

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 ata\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

CALIB STANDHYD ( 0001) Area (ha) = 1.04  
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VO Output - Prop.txt  
 ID= 1 DT= 5.0 min | Total Imp(%) = 48.00 Dir. Conn. (%) = 48.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.50 0.54  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 83.27 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (min) = 5.00 10.00  
 Storage Coeff. (mi n) = 1.87 (ii) 9.42 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.32 0.12  
 PEAK FLOW (cms) = 0.22 0.09  
 TIME TO PEAK (hrs) = 0.50 0.67  
 RUNOFF VOLUME (mm) = 48.17 29.51  
 TOTAL RAINFALL (mm) = 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60

\*TOTALS\*  
 0.266 (iii)  
 0.50  
 38.46  
 49.17  
 0.78

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB STANDHYD ( 0003) Area (ha) = 1.06 Dir. Conn. (%) = 52.00  
 ID= 1 DT= 5.0 min | Total Imp(%) = 52.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.55 0.51  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 84.06 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 1.88 (ii) 9.43 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.32 0.12  
 PEAK FLOW (cms) = 0.24 0.08  
 TIME TO PEAK (hrs) = 0.50 0.67  
 RUNOFF VOLUME (mm) = 48.17 29.51  
 TOTAL RAINFALL (mm) = 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60

\*TOTALS\*  
 0.287 (iii)  
 0.50  
 39.21  
 49.17  
 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)

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VO Output - Prop.txt  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB STANDHYD ( 0004) Area (ha) = 0.22 Dir. Conn. (%) = 75.00  
 ID= 1 DT= 5.0 min | Total Imp(%) = 75.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.17 0.05  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 38.30 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (mi n) = 5.00 5.00  
 Storage Coeff. (mi n) = 1.18 (ii) 4.90 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 5.00  
 Unit Hyd. peak (cms) = 0.33 0.22  
 PEAK FLOW (cms) = 0.08 0.01  
 TIME TO PEAK (hrs) = 0.50 0.50  
 RUNOFF VOLUME (mm) = 48.17 29.51  
 TOTAL RAINFALL (mm) = 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60

\*TOTALS\*  
 0.086 (iii)  
 0.50  
 43.49  
 49.17  
 0.88

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB STANDHYD ( 0002) Area (ha) = 1.05 Dir. Conn. (%) = 51.00  
 ID= 1 DT= 5.0 min | Total Imp(%) = 51.00

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VO Output - Prop.txt

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.54 0.51  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 83.67 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 1.88 (ii) 9.42 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.32 0.12  
 PEAK FLOW (cms) = 0.24 0.08  
 TIME TO PEAK (hrs) = 0.50 0.67  
 RUNOFF VOLUME (mm) = 48.17 29.51  
 TOTAL RAINFALL (mm) = 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60

\*TOTALS\*  
 0.280 (iii)  
 0.50  
 39.02  
 49.17  
 0.79

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010) | AREA OPEAK TPEAK R.V.  
 1 + 2 = 3 (ha) (cms) (hrs) (mm)  
 + ID1 = 1 ( 0001): 1.04 0.256 0.50 38.46  
 + ID2 = 2 ( 0002): 1.05 0.280 0.50 39.02  
 ID = 3 ( 0010): 2.09 0.547 0.50 38.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) | AREA OPEAK TPEAK R.V.  
 3 + 2 = 1 (ha) (cms) (hrs) (mm)  
 + ID1 = 3 ( 0010): 2.09 0.547 0.50 38.74  
 + ID2 = 2 ( 0003): 1.06 0.287 0.50 39.21  
 ID = 1 ( 0010): 3.15 0.833 0.50 38.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) | AREA OPEAK TPEAK R.V.  
 1 + 2 = 3 (ha) (cms) (hrs) (mm)  
 + ID1 = 1 ( 0010): 3.15 0.833 0.50 38.90  
 + ID2 = 2 ( 0004): 0.22 0.086 0.50 43.49  
 ID = 3 ( 0010): 3.37 0.920 0.50 39.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) | AREA OPEAK TPEAK R.V.  
 3 + 2 = 1 (ha) (cms) (hrs) (mm)  
 + ID1 = 3 ( 0010): 3.37 0.920 0.50 39.20  
 + ID2 = 2 ( 0007): 2.05 0.128 0.75 12.72  
 ID = 1 ( 0010): 5.42 0.955 0.50 29.19

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NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB STANDHYD ( 0008) Area (ha) = 14.26 Dir. Conn. (%) = 50.00  
 ID= 1 DT= 5.0 min | Total Imp(%) = 50.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 7.13 7.13  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 308.33 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (mi n) = 5.00 15.00  
 Storage Coeff. (mi n) = 4.11 (ii) 11.65 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 15.00  
 Unit Hyd. peak (cms) = 0.24 0.09  
 PEAK FLOW (cms) = 2.73 1.02  
 TIME TO PEAK (hrs) = 0.50 0.75  
 RUNOFF VOLUME (mm) = 48.17 29.51  
 TOTAL RAINFALL (mm) = 49.17 49.17  
 RUNOFF COEFFICIENT = 0.98 0.60

\*TOTALS\*  
 3.048 (iii)  
 0.50  
 38.84  
 49.17  
 0.79

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\7e735abf  
 Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB STANDHYD ( 0006) Area (ha) = 1.17 Dir. Conn. (%) = 64.00  
 ID= 1 DT= 5.0 min | Total Imp(%) = 64.00

IMPERVIOUS PERVIOUS (i)  
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VO Output - Prop.txt

Surface Area (ha) = 0.75 0.42  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 88.32 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 1.94 (ii) 6.53 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.31 0.14

PEAK FLOW (cms) = 0.33 0.08 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.58 0.374 (iii)  
 RUNOFF VOLUME (mm) = 48.17 29.51 0.50  
 TOTAL RAINFALL (mm) = 49.17 49.17 0.50  
 RUNOFF COEFFICIENT = 0.98 0.60 0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\7e735abf  
 Total = 49.17 mm Comments: 25yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

CALIB STANDHYD ( 0005)  
 ID= 1 DT= 5.0 min Area (ha) = 2.30  
 Total Imp(%) = 70.00 Dir. Conn. (%) = 70.00

IMPERVIOUS PVIOUS (i)

Surface Area (ha) = 1.61 0.69  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 123.83 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 165.21 84.67  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 2.38 (ii) 6.49 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.30 0.14

PEAK FLOW (cms) = 0.69 0.12 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.58 0.765 (iii)  
 RUNOFF VOLUME (mm) = 48.17 29.51 42.57  
 TOTAL RAINFALL (mm) = 49.17 49.17 0.50  
 RUNOFF COEFFICIENT = 0.98 0.60 0.87

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
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VO Output - Prop.txt  
 \*\* SIMULATION : 05 AES 50-yr

READ STORM File name: C:\Users\p001279d\AppData  
 Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d  
 Total = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75
0.17	6.59	0.50	184.63	0.83	32.97
0.25	19.78	0.58	98.91	0.92	19.78
0.33	52.75	0.67	79.13	1.00	6.59

CALIB STANDHYD ( 0030)  
 ID= 1 DT= 5.0 min Area (ha) = 1.58  
 Total Imp(%) = 65.00 Dir. Conn. (%) = 65.00

IMPERVIOUS PVIOUS (i)

Surface Area (ha) = 1.03 0.55  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 102.63 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 184.63 99.26  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 2.03 (ii) 6.34 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.31 0.15

PEAK FLOW (cms) = 0.50 0.12 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.58 0.574 (iii)  
 RUNOFF VOLUME (mm) = 53.95 34.54 47.15  
 TOTAL RAINFALL (mm) = 54.95 54.95 0.50  
 RUNOFF COEFFICIENT = 0.98 0.63 0.86

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d  
 Total = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75
0.17	6.59	0.50	184.63	0.83	32.97
0.25	19.78	0.58	98.91	0.92	19.78
0.33	52.75	0.67	79.13	1.00	6.59

CALIB NASHYD ( 0007)  
 ID= 1 DT= 5.0 min Area (ha) = 2.05 Curve Number (CN) = 70.0  
 Ia (mm) = 5.00 # of Linear Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

VO Output - Prop.txt

ADD HYD ( 0020)  
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0005):	2.30	0.765	0.50	42.57
+ ID2= 2 ( 0006):	1.17	0.374	0.50	41.45
ID = 3 ( 0020):	3.47	1.138	0.50	42.19

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
 3 + 2 = 1

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0020):	3.47	1.138	0.50	42.19
+ ID2= 2 ( 0008):	14.26	3.048	0.50	38.84
ID = 1 ( 0020):	17.73	4.186	0.50	39.50

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

V V I SSSSS U U A L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U A A L  
 V V I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M O O TM  
 0 0 T T H H Y Y M M O O  
 0 0 T T H H Y Y M M O O  
 000 T T T H H Y M M O O

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input File name: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output File name:  
 C:\Users\p001279d\AppData\Local\Civica\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\d38b07  
 05-ba51-4ab1-8948-0127227f1712\sce  
 Summary File name:  
 C:\Users\p001279d\AppData\Local\Civica\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\d38b07  
 05-ba51-4ab1-8948-0127227f1712\sce

DATE: 11/12/2019 TIME: 11:34:07

USER:

COMMENTS:

VO Output - Prop.txt  
 Unit Hyd Opeak (cms) = 0.391

PEAK FLOW (cms) = 0.158 (i)  
 TIME TO PEAK (hrs) = 0.750  
 RUNOFF VOLUME (mm) = 15.681  
 TOTAL RAINFALL (mm) = 64.950  
 RUNOFF COEFFICIENT = 0.285

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d  
 Total = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75
0.17	6.59	0.50	184.63	0.83	32.97
0.25	19.78	0.58	98.91	0.92	19.78
0.33	52.75	0.67	79.13	1.00	6.59

CALIB STANDHYD ( 0001)  
 ID= 1 DT= 5.0 min Area (ha) = 1.04  
 Total Imp(%) = 48.00 Dir. Conn. (%) = 48.00

IMPERVIOUS PVIOUS (i)

Surface Area (ha) = 0.50 0.54  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 83.27 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 184.63 99.26  
 over (mi n) = 5.00 10.00  
 Storage Coeff. (mi n) = 1.79 (ii) 8.87 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.32 0.12

PEAK FLOW (cms) = 0.25 0.10 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.67 0.50  
 RUNOFF VOLUME (mm) = 53.95 34.54 43.85  
 TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
 RUNOFF COEFFICIENT = 0.98 0.63 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PVIOUS LOSSES:  
 CN\* = 90.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData  
 Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d  
 Total = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75
0.17	6.59	0.50	184.63	0.83	32.97
0.25	19.78	0.58	98.91	0.92	19.78
0.33	52.75	0.67	79.13	1.00	6.59

VO Output - Prop.txt

CALIB STANDBYD ( 0003) Area (ha) = 1.06 Total Imp(%) = 52.00 Dir. Conn.(%) = 52.00

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d Comments: 50yr 1hr AES

CALIB STANDBYD ( 0004) Area (ha) = 0.22 Total Imp(%) = 75.00 Dir. Conn.(%) = 75.00

3 + 2 = 1 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 1 + 2 = 3 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d Comments: 50yr 1hr AES

CALIB STANDBYD ( 0008) Area (ha) = 14.26 Total Imp(%) = 50.00 Dir. Conn.(%) = 50.00

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! Page 35

VO Output - Prop.txt

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d Comments: 50yr 1hr AES

CALIB STANDBYD ( 0002) Area (ha) = 1.05 Total Imp(%) = 51.00 Dir. Conn.(%) = 51.00

Surface Area (ha) = 0.51 IMPERVIOUS (i) PERVIOUS (i) 0.51 2.00

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010) 1 + 2 = 3 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

VO Output - Prop.txt

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d Comments: 50yr 1hr AES

CALIB STANDBYD ( 0006) Area (ha) = 1.17 Total Imp(%) = 64.00 Dir. Conn.(%) = 64.00

Surface Area (ha) = 0.42 IMPERVIOUS (i) PERVIOUS (i) 0.42 2.00

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP! (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\ac308a9d Comments: 50yr 1hr AES



VO Output - Prop.txt

CALIB STANDHYD ( 0005) Area (ha)= 2.30 Dir. Conn.(%)= 70.00  
ID= 1 DT= 5.0 min Total Imp(%)= 70.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 1.61 0.69  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 123.83 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 184.63 99.26  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 2.27 (ii) 6.21 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.30 0.15

PEAK FLOW (cms)= 0.78 0.15 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.58 0.867 (iii)  
RUNOFF VOLUME (mm)= 53.95 34.54 48.13  
TOTAL RAINFALL (mm)= 54.95 54.95 54.95  
RUNOFF COEFFICIENT = 0.98 0.63 0.88

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 1 ( 0005):	2.30	0.867	0.50	48.13
+ ID2= 2 ( 0006):	1.17	0.424	0.50	46.96
ID = 3 ( 0020):	3.47	1.292	0.50	47.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 3 ( 0020):	3.47	1.292	0.50	47.73
+ ID2= 2 ( 0008):	14.26	3.489	0.50	44.24
ID = 1 ( 0020):	17.73	4.780	0.50	44.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSSS UUUUU A A LLLLL

000 TTTTT TTTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T H H Y Y M M 000

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VO Output - Prop.txt  
\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Summary filename:  
C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e

DATE: 11/12/2019 TIME: 11:34:05

USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 06 AES 100-yr \*\*\*\*\*

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB STANDHYD ( 0030) Area (ha)= 1.58 Dir. Conn.(%)= 65.00  
ID= 1 DT= 5.0 min Total Imp(%)= 65.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 1.03 0.55  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 102.63 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.95 (ii) 6.09 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.31 0.15

PEAK FLOW (cms)= 0.56 0.14 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.58 0.645 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 52.84  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.87

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - Prop.txt  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB NASHYD ( 0007) Area (ha)= 2.05 Curve Number (CN)= 70.0  
ID= 1 DT= 5.0 min Ia (mm)= 5.00 # of LI near Res. (N)= 3.00  
U. H. Tp (hrs)= 0.20

Unit t Hyd Opeak (cms)= 0.391  
PEAK FLOW (cms)= 0.191 (i)  
TIME TO PEAK (hrs)= 0.750  
RUNOFF VOLUME (mm)= 18.913  
TOTAL RAINFALL (mm)= 60.870  
RUNOFF COEFFICIENT = 0.311

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB STANDHYD ( 0001) Area (ha)= 1.04 Dir. Conn.(%)= 48.00  
ID= 1 DT= 5.0 min Total Imp(%)= 48.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.50 0.54  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.27 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.72 (ii) 8.41 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.28 0.12 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.67 0.345 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 49.42  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87

VO Output - Prop.txt  
RUNOFF COEFFICIENT = 0.98 0.65 0.81

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB STANDHYD ( 0003) Area (ha)= 1.06 Dir. Conn.(%)= 52.00  
ID= 1 DT= 5.0 min Total Imp(%)= 52.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.55 0.51  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 84.06 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (min)= 5.00 10.00  
Storage Coeff. (min)= 1.73 (ii) 8.42 (ii)  
Unit t Hyd. Tpeak (min)= 5.00 10.00  
Unit t Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.30 0.11 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.67 0.369 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 50.23  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.83

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

VO Output - Prop.txt

CALIB STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha)= 0.22  
Total Imp(%)= 75.00 Dir. Conn.(%)= 75.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.17 0.05  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 38.30 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (mi n)= 5.00 5.00  
Storage Coeff. (mi n)= 1.08 (ii) 4.49 (iii)  
Unit Hyd. Tpeak (mi n)= 5.00 5.00  
Unit Hyd. peak (cms)= 0.34 0.23

PEAK FLOW (cms)= 0.09 0.02 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.50 0.109 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 54.84  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.90

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha)= 1.05  
Total Imp(%)= 51.00 Dir. Conn.(%)= 51.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.54 0.51  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.67 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (mi n)= 5.00 10.00  
Storage Coeff. (mi n)= 1.72 (ii) 10.46 (iii)  
Unit Hyd. Tpeak (mi n)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.32 0.12

PEAK FLOW (cms)= 0.30 0.11 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.67 0.361 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 50.03  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.82

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VO Output - Prop.txt

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1.04	0.345	0.50	49.42
1.05	0.361	0.50	50.03
2.09	0.706	0.50	49.73

ID= 3 ( 0010): 2.09 0.706 0.50 49.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
3.15	1.075	0.50	49.90
1.06	0.369	0.50	50.23
3.15	1.075	0.50	49.90

ID= 1 ( 0010): 3.15 1.075 0.50 49.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
3.15	1.075	0.50	49.90
0.22	0.109	0.50	54.84
3.37	1.185	0.50	50.22

ID= 3 ( 0010): 3.37 1.185 0.50 50.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
3.37	1.185	0.50	50.22
2.05	0.191	0.75	18.91
5.42	1.242	0.50	38.38

ID= 1 ( 0010): 5.42 1.242 0.50 38.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

VO Output - Prop.txt

CALIB STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha)= 14.26  
Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 7.13 7.13  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 308.33 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (mi n)= 5.00 15.00  
Storage Coeff. (mi n)= 3.77 (ii) 10.46 (iii)  
Unit Hyd. Tpeak (mi n)= 5.00 15.00  
Unit Hyd. peak (cms)= 0.25 0.09

PEAK FLOW (cms)= 3.46 1.43 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.75 3.950 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 49.83  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB STANDHYD ( 0006)  
ID= 1 DT= 5.0 min

Area (ha)= 1.17  
Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 0.75 0.42  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 88.32 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (mi n)= 5.00 10.00  
Storage Coeff. (mi n)= 1.78 (ii) 10.46 (iii)  
Unit Hyd. Tpeak (mi n)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.32 0.15

PEAK FLOW (cms)= 0.41 0.11 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.58 0.477 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 52.64  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.86

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

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VO Output - Prop.txt

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\57b9b19e  
Total = 60.87 mm Comments: 100yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB STANDHYD ( 0005)  
ID= 1 DT= 5.0 min

Area (ha)= 2.30  
Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha)= 1.51 0.69  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 123.83 40.00  
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 204.52 114.44  
over (mi n)= 5.00 10.00  
Storage Coeff. (mi n)= 2.18 (ii) 5.96 (iii)  
Unit Hyd. Tpeak (mi n)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.31 0.15

PEAK FLOW (cms)= 0.87 0.18 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.58 0.974 (iii)  
RUNOFF VOLUME (mm)= 59.87 39.79 53.85  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.65 0.88

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
2.30	0.974	0.50	53.85
1.17	0.477	0.50	52.64
3.47	1.450	0.50	53.44

ID= 3 ( 0020): 3.47 1.450 0.50 53.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
3.47	1.450	0.50	53.44
1.17	0.477	0.50	52.64
3.47	1.450	0.50	53.44

ID= 1 ( 0020): 3.47 1.450 0.50 53.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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VO Output - Prop.txt
ID1= 3 ( 0020): 3.47 1.450 0.50 53.44
+ ID2= 2 ( 0008): 14.26 3.950 0.50 49.83
ID = 1 ( 0020): 17.73 5.400 0.50 50.54

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W V I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voinput.dat

Output filename: C:\Users\p001279d\AppData\Local\Civica\VHS\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\b3fd7dc-7ea3-4c5d-a71a-6ea787b104d9\vsce
Summary filename: C:\Users\p001279d\AppData\Local\Civica\VHS\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\b3fd7dc-7ea3-4c5d-a71a-6ea787b104d9\vsce

DATE: 11/12/2019

TIME: 11:34:06

USER:

COMMENTS:

\*\*\* SIMULATION : 07 AES 250yr \*\*\*

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\d9c70000
Ptotal = 70.05 mm Comments: 250yr 1hr AES

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show peak flow data at various time intervals.

CALIB STANDHYD ( 0030) Area (ha) = 1.58 Dir. Conn. (%) = 65.00
ID= 1 DT= 5.0 min Total Imp(%) = 65.00

VO Output - Prop.txt

CALIB STANDHYD ( 0001) Area (ha) = 1.04 Dir. Conn. (%) = 48.00
ID= 1 DT= 5.0 min Total Imp(%) = 48.00

Surface Area (ha) = 0.50
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 83.27
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 235.37
Storage Coeff. (mi n) = 1.63 (ii)
Unit Hyd. Tpeak (mi n) = 5.00

PEAK FLOW (cms) = 0.32
TIME TO PEAK (hrs) = 0.50
RUNOFF VOLUME (mm) = 69.05
TOTAL RAINFALL (mm) = 70.05

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\d9c70000
Ptotal = 70.05 mm Comments: 250yr 1hr AES

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show peak flow data at various time intervals.

CALIB STANDHYD ( 0003) Area (ha) = 1.06 Dir. Conn. (%) = 52.00
ID= 1 DT= 5.0 min Total Imp(%) = 52.00

Surface Area (ha) = 0.55
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 84.06
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 235.37
Storage Coeff. (mi n) = 1.63 (ii)
Unit Hyd. Tpeak (mi n) = 5.00

PEAK FLOW (cms) = 0.35
TIME TO PEAK (hrs) = 0.50
RUNOFF VOLUME (mm) = 69.05
TOTAL RAINFALL (mm) = 70.05

VO Output - Prop.txt
IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 1.03
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 102.63
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 235.37
Storage Coeff. (mi n) = 1.84 (ii)
Unit Hyd. Tpeak (mi n) = 5.00

PEAK FLOW (cms) = 0.65
TIME TO PEAK (hrs) = 0.50
RUNOFF VOLUME (mm) = 69.05
TOTAL RAINFALL (mm) = 70.05

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\d9c70000
Ptotal = 70.05 mm Comments: 250yr 1hr AES

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show peak flow data at various time intervals.

CALIB NASHYD ( 0007) Area (ha) = 2.05 Curve Number (CN) = 70.0
ID= 1 DT= 5.0 min U.H. Tp(hrs) = 0.20 # of Linear Res. (N) = 3.00

Unit Hyd. Tpeak (cms) = 0.391
PEAK FLOW (cms) = 0.245 (i)
TIME TO PEAK (hrs) = 0.750
RUNOFF VOLUME (mm) = 24.286
TOTAL RAINFALL (mm) = 70.050

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\d9c70000
Ptotal = 70.05 mm Comments: 250yr 1hr AES

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show peak flow data at various time intervals.

CALIB STANDHYD ( 0004) Area (ha) = 0.22 Dir. Conn. (%) = 75.00
ID= 1 DT= 5.0 min Total Imp(%) = 75.00

Surface Area (ha) = 0.17
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 38.30
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 235.37
Storage Coeff. (mi n) = 1.02 (ii)
Unit Hyd. Tpeak (mi n) = 5.00

PEAK FLOW (cms) = 0.11
TIME TO PEAK (hrs) = 0.50
RUNOFF VOLUME (mm) = 69.05
TOTAL RAINFALL (mm) = 70.05

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\d9c70000
Ptotal = 70.05 mm Comments: 250yr 1hr AES

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show peak flow data at various time intervals.

CALIB STANDHYD ( 0004) Area (ha) = 0.22 Dir. Conn. (%) = 75.00
ID= 1 DT= 5.0 min Total Imp(%) = 75.00

Surface Area (ha) = 0.17
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 38.30
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 235.37
Storage Coeff. (mi n) = 1.02 (ii)
Unit Hyd. Tpeak (mi n) = 5.00

PEAK FLOW (cms) = 0.11
TIME TO PEAK (hrs) = 0.50
RUNOFF VOLUME (mm) = 69.05
TOTAL RAINFALL (mm) = 70.05

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa00b599b\d9c70000
Ptotal = 70.05 mm Comments: 250yr 1hr AES

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show peak flow data at various time intervals.

VO Output - Prop.txt

CALIB STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area	(ha)	1.05		Dir. Conn. (%)	51.00	
Total Imp	(%)	51.00				
		IMPERVIOUS	PERVIOUS (i)			
Surface Area	(ha)	0.54	0.51			
Dep. Storage	(mm)	1.00	2.00			
Average Slope	(%)	1.00	2.00			
Length	(m)	83.67	40.00			
Mannings n		0.013	0.250			
Max. Eff. Inten.	(mm/hr)	235.37	138.35			
over	(mi n)	5.00	10.00			
Storage Coeff.	(mi n)	1.63 (ii)	6.56 (ii)			
Unit Hyd. Tpeak	(mi n)	5.00	10.00			
Unit Hyd. peak	(cms)	0.32	0.14			
PEAK FLOW	(cms)	0.34	0.15	*TOTALS*		
TIME TO PEAK	(hrs)	0.50	0.58	0.50 (iii)		
RUNOFF VOLUME	(mm)	69.05	48.10	58.78		
TOTAL RAINFALL	(mm)	70.05	70.05	70.05		
RUNOFF COEFFICIENT		0.99	0.69	0.84		

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0001):	1.04	0.408	0.50 58.15
+ ID2= 2 ( 0002):	1.05	0.435	0.50 58.78
ID = 3 ( 0010):	2.09	0.843	0.50 58.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0010):	2.09	0.843	0.50 58.47
+ ID2= 2 ( 0003):	1.06	0.444	0.50 58.99
ID = 1 ( 0010):	3.15	1.288	0.50 58.64

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0010):	3.15	1.288	0.50 58.64
+ ID2= 2 ( 0004):	0.22	0.128	0.50 63.80
ID = 3 ( 0010):	3.37	1.415	0.50 58.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

VO Output - Prop.txt

3 + 2 = 1	AREA	OPEAK	TPEAK	R. V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0010):	3.37	1.415	0.50	58.98
+ ID2= 2 ( 0007):	2.05	0.245	0.75	24.29
ID = 1 ( 0010):	5.42	1.493	0.50	45.86

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\d9c70000  
Total = 70.05 mm  
Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

CALIB STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area	(ha)	14.26		Dir. Conn. (%)	50.00	
Total Imp	(%)	50.00				
		IMPERVIOUS	PERVIOUS (i)			
Surface Area	(ha)	7.13	7.13			
Dep. Storage	(mm)	1.00	2.00			
Average Slope	(%)	1.00	2.00			
Length	(m)	308.33	40.00			
Mannings n		0.013	0.250			
Max. Eff. Inten.	(mm/hr)	235.37	138.35			
over	(mi n)	5.00	10.00			
Storage Coeff.	(mi n)	3.56 (ii)	9.76 (ii)			
Unit Hyd. Tpeak	(mi n)	5.00	10.00			
Unit Hyd. peak	(cms)	0.26	0.11			
PEAK FLOW	(cms)	4.04	1.83	*TOTALS*		
TIME TO PEAK	(hrs)	0.50	0.67	5.075 (iii)		
RUNOFF VOLUME	(mm)	69.05	48.10	58.58		
TOTAL RAINFALL	(mm)	70.05	70.05	70.05		
RUNOFF COEFFICIENT		0.99	0.69	0.84		

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\d9c70000  
Total = 70.05 mm  
Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

VO Output - Prop.txt

CALIB STANDHYD ( 0006)  
ID= 1 DT= 5.0 min

Area	(ha)	1.17		Dir. Conn. (%)	64.00	
Total Imp	(%)	64.00				
		IMPERVIOUS	PERVIOUS (i)			
Surface Area	(ha)	0.75	0.42			
Dep. Storage	(mm)	1.00	2.00			
Average Slope	(%)	1.00	2.00			
Length	(m)	88.32	40.00			
Mannings n		0.013	0.250			
Max. Eff. Inten.	(mm/hr)	235.37	138.35			
over	(mi n)	5.00	10.00			
Storage Coeff.	(mi n)	1.68 (ii)	5.66 (ii)			
Unit Hyd. Tpeak	(mi n)	5.00	10.00			
Unit Hyd. peak	(cms)	0.32	0.15			
PEAK FLOW	(cms)	0.48	0.13	*TOTALS*		
TIME TO PEAK	(hrs)	0.50	0.58	0.50 (iii)		
RUNOFF VOLUME	(mm)	69.05	48.10	61.51		
TOTAL RAINFALL	(mm)	70.05	70.05	70.05		
RUNOFF COEFFICIENT		0.99	0.69	0.88		

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\11f23fa1-8a57-479d-b028-9b4fa0b599b\d9c70000  
Total = 70.05 mm  
Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

CALIB STANDHYD ( 0005)  
ID= 1 DT= 5.0 min

Area	(ha)	2.30		Dir. Conn. (%)	70.00	
Total Imp	(%)	70.00				
		IMPERVIOUS	PERVIOUS (i)			
Surface Area	(ha)	1.61	0.69			
Dep. Storage	(mm)	1.00	2.00			
Average Slope	(%)	1.00	2.00			
Length	(m)	123.83	40.00			
Mannings n		0.013	0.250			
Max. Eff. Inten.	(mm/hr)	235.37	138.35			
over	(mi n)	5.00	10.00			
Storage Coeff.	(mi n)	2.06 (ii)	6.56 (ii)			
Unit Hyd. Tpeak	(mi n)	5.00	10.00			
Unit Hyd. peak	(cms)	0.31	0.15			
PEAK FLOW	(cms)	1.01	0.22	*TOTALS*		
TIME TO PEAK	(hrs)	0.50	0.58	1.141 (iii)		
RUNOFF VOLUME	(mm)	69.05	48.10	62.76		
TOTAL RAINFALL	(mm)	70.05	70.05	70.05		
RUNOFF COEFFICIENT		0.99	0.69	0.90		

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
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VO Output - Prop.txt

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0005):	2.30	1.141	0.50 62.76
+ ID2= 2 ( 0006):	1.17	0.559	0.50 61.51
ID = 3 ( 0020):	3.47	1.700	0.50 62.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0020):	3.47	1.700	0.50 62.34
+ ID2= 2 ( 0008):	14.26	5.075	0.50 58.58
ID = 1 ( 0020):	17.73	6.774	0.50 59.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
V V I SSSS U U A L (v 5. 2. 2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM
0 0 T H H Y Y M M 0 0
0 0 T H H Y Y M M 0 0
000 M M M 000
```

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1.n.dat  
Output filename:  
C:\Users\p001279d\AppData\Local\Civica\H5\b8ccee0c-2cc7-4973-8ab8-dd0e50b3a28\ba6448  
a7-c0b3-4f36-8e8c-4fe363c672b1\scce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civica\H5\b8ccee0c-2cc7-4973-8ab8-dd0e50b3a28\ba6448  
a7-c0b3-4f36-8e8c-4fe363c672b1\scce  
DATE: 11/12/2019 TIME: 11:34:03  
USER:  
COMMENTS:

VO Output - Prop.txt

\*\* SIMULATION : 08 Chicago - 3hr 2-yr \*\*

CHICAGO STORM
Ptotal = 33.36 mm
IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity at various time intervals.

CALIB STANDHYD ( 0030)
ID= 1 DT= 5.0 min
Area (ha)= 1.58
Total Imp(%)= 65.00
Dir. Conn.(%)= 65.00

Surface Area (ha)= 1.03
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 102.63
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall intensity.

Max. Eff. Inten. (mm/hr)= 83.04
over (min)= 5.00
Storage Coeff. (mi n)= 2.80
Unit Hyd. Tpeak (mi n)= 5.00
Unit Hyd. peak (cms)= 0.28

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - Prop.txt

CHICAGO STORM
Ptotal = 33.36 mm

IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity.

CALIB STANDHYD ( 0007)
ID= 1 DT= 5.0 min
Area (ha)= 2.05
Total Imp(%)= 75.00
Dir. Conn.(%)= 75.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall intensity.

Unit Hyd. Qpeak (cms)= 0.391
PEAK FLOW (cms)= 0.037 (i)
TIME TO PEAK (hrs)= 1.67
RUNOFF VOLUME (mm)= 5.852
TOTAL RAINFALL (mm)= 33.364
RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM
Ptotal = 33.36 mm
IDF curve parameters: A= 724.700
B= 5.000
C= 0.800

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity.

CALIB

STANDHYD ( 0001)
ID= 1 DT= 5.0 min
Area (ha)= 1.04
Total Imp(%)= 48.00
Dir. Conn.(%)= 48.00

Surface Area (ha)= 0.50
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 83.27
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall intensity.

Max. Eff. Inten. (mm/hr)= 83.04
over (min)= 5.00
Storage Coeff. (mi n)= 2.47
Unit Hyd. Tpeak (mi n)= 5.00
Unit Hyd. peak (cms)= 0.30

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM
Ptotal = 33.36 mm
IDF curve parameters: A= 724.700
B= 5.000
C= 0.800

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity.

CALIB STANDHYD ( 0003)
ID= 1 DT= 5.0 min
Area (ha)= 1.06
Total Imp(%)= 52.00
Dir. Conn.(%)= 52.00

Surface Area (ha)= 0.55
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 38.30
Mannings n = 0.013

VO Output - Prop.txt

CHICAGO STORM
Ptotal = 33.36 mm

IDF curve parameters: A= 724.700
B= 5.000
C= 0.800
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall intensity.

Max. Eff. Inten. (mm/hr)= 83.04
over (min)= 5.00
Storage Coeff. (mi n)= 2.48
Unit Hyd. Tpeak (mi n)= 5.00
Unit Hyd. peak (cms)= 0.29

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM
Ptotal = 33.36 mm
IDF curve parameters: A= 724.700
B= 5.000
C= 0.800

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall intensity.

CALIB STANDHYD ( 0004)
ID= 1 DT= 5.0 min
Area (ha)= 0.22
Total Imp(%)= 75.00
Dir. Conn.(%)= 75.00

Surface Area (ha)= 0.17
Dep. Storage (mm)= 1.00
Average Slope (%)= 1.00
Length (m)= 38.30
Mannings n = 0.013

VO Output - Prop.txt  
 NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

Max. Eff. Inten. (mm/hr) = 83.04 39.00  
 over (min) = 5.00 10.00  
 Storage Coeff. (mi n) = 1.53 (ii) 6.45 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 10.00  
 Unit Hyd. peak (cms) = 0.33 0.14

PEAK FLOW (cms) = 0.04 0.00 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.08 1.00  
 RUNOFF VOLUME (mm) = 32.36 16.51 28.39  
 TOTAL RAINFALL (mm) = 33.36 33.36 33.36  
 RUNOFF COEFFICIENT = 0.97 0.49 0.85

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 0002 )  
 ID= 1 DT= 5.0 min

Area (ha) = 1.05  
 Total Imp(%) = 51.00 Dir. Conn. (%) = 51.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.54 0.51  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 83.67 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29	1.583	6.50	2.33	3.47
0.167	2.99	0.917	83.04	1.667	6.50	2.42	3.13
0.250	3.69	1.000	83.04	1.750	5.29	2.50	3.13
0.333	3.69	1.083	24.13	1.833	5.29	2.58	2.86
0.417	4.88	1.167	24.13	1.917	4.49	2.67	2.86
0.500	4.88	1.250	12.47	2.000	4.49	2.75	2.63
0.583	7.49	1.333	12.47	2.083	3.91	2.83	2.63
0.667	7.49	1.417	8.49	2.167	3.91	2.92	2.44
0.750	18.29	1.500	8.49	2.250	3.47	3.00	2.44

Max. Eff. Inten. (mm/hr) = 83.04 31.18  
 over (mi n) = 5.00 15.00  
 Storage Coeff. (mi n) = 2.47 (ii) 13.72 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 15.00  
 Unit Hyd. peak (cms) = 0.29 0.08

PEAK FLOW (cms) = 0.12 0.03 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.17 0.136 (iii)  
 RUNOFF VOLUME (mm) = 32.36 16.51 24.59  
 TOTAL RAINFALL (mm) = 33.36 33.36 33.36  
 RUNOFF COEFFICIENT = 0.97 0.49 0.74

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
1.04	0.128	1.00	24.11
1.05	0.136	1.00	24.59
2.09	0.264	1.00	24.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
2.09	0.264	1.00	24.35
1.06	0.139	1.00	24.75
3.15	0.403	1.00	24.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
0.22	0.042	1.00	28.39
1.06	0.139	1.00	24.75
3.15	0.403	1.00	24.48

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010 )  
 3 + 2 = 1

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
3.37	0.445	1.00	24.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

VO Output - Prop.txt  
 + ID2= 2 ( 0007): 2.05 0.037 1.17 5.85  
 ID = 1 ( 0010): 5.42 0.465 1.00 17.59

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 0008 )  
 ID= 1 DT= 5.0 min

Area (ha) = 14.26  
 Total Imp(%) = 50.00 Dir. Conn. (%) = 50.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 7.13 7.13  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 308.33 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

Max. Eff. Inten. (mm/hr) = 83.04 31.18  
 over (mi n) = 5.00 20.00  
 Storage Coeff. (mi n) = 5.41 (ii) 16.66 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 20.00  
 Unit Hyd. peak (cms) = 0.21 0.06

PEAK FLOW (cms) = 1.44 0.35 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.25 1.00  
 RUNOFF VOLUME (mm) = 32.36 16.51 24.44  
 TOTAL RAINFALL (mm) = 33.36 33.36 33.36  
 RUNOFF COEFFICIENT = 0.97 0.49 0.73

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - Prop.txt

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 0006 )  
 ID= 1 DT= 5.0 min

Area (ha) = 1.17  
 Total Imp(%) = 64.00 Dir. Conn. (%) = 64.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.75 0.42  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 88.32 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29
0.167	2.99	0.917	83.04
0.250	3.69	1.000	83.04
0.333	3.69	1.083	24.13
0.417	4.88	1.167	24.13
0.500	4.88	1.250	12.47
0.583	7.49	1.333	12.47
0.667	7.49	1.417	8.49
0.750	18.29	1.500	8.49

Max. Eff. Inten. (mm/hr) = 83.04 31.18  
 over (mi n) = 5.00 15.00  
 Storage Coeff. (mi n) = 5.55 (ii) 13.80 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00 15.00  
 Unit Hyd. peak (cms) = 0.29 0.08

PEAK FLOW (cms) = 0.17 0.02 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.17 0.181 (iii)  
 RUNOFF VOLUME (mm) = 32.36 16.51 26.65  
 TOTAL RAINFALL (mm) = 33.36 33.36 33.36  
 RUNOFF COEFFICIENT = 0.97 0.49 0.80

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
 Ptotal = 33.36 mm

IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800

used in: INTENSITY = A / (t + B)^C

VO Output - Prop.txt

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
STANDHYD ( 0005)  
ID= 1 DT= 5.0 min

Area (ha)= 2.30  
Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00

	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	1.61		0.69
Dep. Storage	1.00		2.00
Average Slope	1.00		2.00
Length	123.83		40.00
Mannings n	0.013		0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	2.99	0.833	18.29	1.583	6.50	2.33	3.47
0.167	2.99	0.917	83.04	1.667	6.50	2.42	3.13
0.250	3.69	1.000	83.04	1.750	5.29	2.50	3.13
0.333	3.69	1.083	24.13	1.833	5.29	2.58	2.86
0.417	4.88	1.167	24.13	1.917	4.49	2.67	2.86
0.500	4.88	1.250	12.47	2.000	4.49	2.75	2.63
0.583	7.49	1.333	12.47	2.083	3.91	2.83	2.63
0.667	7.49	1.417	8.49	2.167	3.91	2.92	2.44
0.750	18.29	1.500	8.49	2.250	3.47	3.00	2.44

Max. Eff. Inten. (mm/hr)= 83.04  
over (min)= 5.00  
Storage Coeff. (mi n)= 3.13 (ii)  
Unit Hyd. Tpeak (mi n)= 5.00  
Unit Hyd. peak (cms)= 0.27

\*TOTALS\*  
PEAK FLOW (cms)= 0.36  
TIME TO PEAK (hrs)= 1.00  
RUNOFF VOLUME (mm)= 32.36  
TOTAL RAINFALL (mm)= 33.36  
RUNOFF COEFFICIENT = 0.97

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)  
1 + 2 = 3

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID= 1 ( 0005):	2.30	0.378	1.00	27.60
+ ID2= 2 ( 0006):	1.17	0.181	1.00	26.65
ID= 3 ( 0020):	3.47	0.559	1.00	27.28

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VO Output - Prop.txt

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.50	6.09	1.33	17.98	2.17	4.69
0.67	10.04	1.50	11.61	2.33	4.08
0.83	27.27	1.67	8.50	2.50	3.61

CALIB  
STANDHYD ( 0030)  
ID= 1 DT= 5.0 min

Area (ha)= 1.58  
Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	1.00		2.00
Dep. Storage	1.00		2.00
Average Slope	1.00		2.00
Length	102.63		40.00
Mannings n	0.013		0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61
0.250	4.38	1.000	112.71	1.750	6.69	2.50	3.61
0.333	4.38	1.083	36.59	1.833	6.69	2.58	3.25
0.417	6.09	1.167	36.59	1.917	5.51	2.67	3.25
0.500	6.09	1.250	17.98	2.000	5.51	2.75	2.95
0.583	10.04	1.333	17.98	2.083	4.69	2.83	2.95
0.667	10.04	1.417	11.61	2.167	4.69	2.92	2.70
0.750	27.27	1.500	11.61	2.250	4.08	3.00	2.70

Max. Eff. Inten. (mm/hr)= 112.71  
over (mi n)= 5.00  
Storage Coeff. (mi n)= 2.47 (ii)  
Unit Hyd. Tpeak (mi n)= 5.00  
Unit Hyd. peak (cms)= 0.29

\*TOTALS\*  
PEAK FLOW (cms)= 0.32  
TIME TO PEAK (hrs)= 1.00  
RUNOFF VOLUME (mm)= 44.35  
TOTAL RAINFALL (mm)= 45.35  
RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855  
used in: INTENSITY = A / (t + B)\*C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

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VO Output - Prop.txt

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)  
3 + 2 = 1

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID= 3 ( 0020):	3.47	0.559	1.00	27.28
+ ID2= 2 ( 0008):	14.26	1.559	1.00	24.44
ID= 1 ( 0020):	17.73	2.118	1.00	24.99

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
V V I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Civi ca\VH5\bCcece0-2cc7-4973-8ab8-dd0de50b3a28\5dccc187-a065-4d2a-b3bf-43029a05d4f2\sce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civi ca\VH5\bCcece0-2cc7-4973-8ab8-dd0de50b3a28\5dccc187-a065-4d2a-b3bf-43029a05d4f2\sce

DATE: 11/12/2019

TIME: 11:34:02

USER:

COMMENTS:

\*\*\* SIMULATION : 09 Chicago - 3hr 5-yr \*\*\*

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855  
used in: INTENSITY = A / (t + B)\*C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95

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VO Output - Prop.txt

CALIB  
NASHYD ( 0007)  
ID= 1 DT= 5.0 min

Area (ha)= 2.05  
Ia (mm)= 5.00  
U.H. Tp(hrs)= 0.20  
Curve Number (CN)= 70.0  
# of Linear Res. (N)= 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61
0.250	4.38	1.000	112.71	1.750	6.69	2.50	3.61
0.333	4.38	1.083	36.59	1.833	6.69	2.58	3.25
0.417	6.09	1.167	36.59	1.917	5.51	2.67	3.25
0.500	6.09	1.250	17.98	2.000	5.51	2.75	2.95
0.583	10.04	1.333	17.98	2.083	4.69	2.83	2.95
0.667	10.04	1.417	11.61	2.167	4.69	2.92	2.70
0.750	27.27	1.500	11.61	2.250	4.08	3.00	2.70

Unit Hyd Opeak (cms)= 0.391

PEAK FLOW (cms)= 0.075 (i)  
TIME TO PEAK (hrs)= 1.167  
RUNOFF VOLUME (mm)= 10.889  
TOTAL RAINFALL (mm)= 45.346  
RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 45.35 mm

IDF curve parameters: A=1330.300  
B= 7.938  
C= 0.855  
used in: INTENSITY = A / (t + B)\*C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB  
STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha)= 1.04  
Total Imp(%)= 48.00 Dir. Conn.(%)= 48.00

	(ha)	IMPERVIOUS	PERVIOUS (i)
Surface Area	0.50		0.54
Dep. Storage	1.00		2.00
Average Slope	1.00		2.00
Length	83.27		40.00
Mannings n	0.013		0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	3.43	0.833	27.27	1.583	8.50	2.33	4.08
0.167	3.43	0.917	112.71	1.667	8.50	2.42	3.61

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VO Output - Prop.txt
0.250 4.38 1.000 12.71 1.750 6.69 2.50 3.61
0.333 4.38 1.083 36.59 1.833 6.69 2.58 3.25
0.417 6.09 1.167 36.59 1.917 5.51 2.67 3.25
0.500 6.09 1.250 17.98 2.000 5.51 2.75 2.95
0.583 10.04 1.333 17.98 2.083 4.69 2.83 2.95
0.667 10.04 1.417 11.61 2.167 4.69 2.92 2.70
0.750 27.27 1.500 11.61 2.250 4.08 3.00 2.70

Max. Eff. Inten. (mm/hr) = 112.71 over (min) = 5.00
Storage Coeff. (mi n) = 2.18 (ii) 10.59 (ii)
Unit t Hyd. Tpeak (mi n) = 5.00
Unit t Hyd. peak (cms) = 0.31
PEAK FLOW (cms) = 0.16
TIME TO PEAK (hrs) = 1.00
RUNOFF VOLUME (mm) = 44.35
TOTAL RAINFALL (mm) = 45.35
RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 45.35 mm IDF curve parameters: A=1330.300 B= 7.938 C= 0.855
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show data for times 0.17, 0.33, 0.50, 0.67, 0.83.

CALIB STANDHYD ( 0003) ID= 1 DT= 5.0 min Area (ha) = 1.06 Total Imp(%) = 52.00 Dir. Conn. (%) = 52.00

Surface Area (ha) = 0.55
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 84.06
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show data for times 0.083 to 0.750.

VO Output - Prop.txt
0.667 10.04 1.417 11.61 2.167 4.69 2.92 2.70
0.750 27.27 1.500 11.61 2.250 4.08 3.00 2.70

Max. Eff. Inten. (mm/hr) = 112.71 over (mi n) = 5.00
Storage Coeff. (mi n) = 2.19 (ii) 10.61 (ii)
Unit t Hyd. Tpeak (mi n) = 5.00
Unit t Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.17
TIME TO PEAK (hrs) = 1.00
RUNOFF VOLUME (mm) = 44.35
TOTAL RAINFALL (mm) = 45.35
RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 45.35 mm IDF curve parameters: A=1330.300 B= 7.938 C= 0.855
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show data for times 0.17, 0.33, 0.50, 0.67, 0.83.

CALIB STANDHYD ( 0004) ID= 1 DT= 5.0 min Area (ha) = 0.22 Total Imp(%) = 75.00 Dir. Conn. (%) = 75.00

Surface Area (ha) = 0.17
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 38.30
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show data for times 0.083 to 0.750.

Storage Coeff. (mi n) = 1.37 (ii) 5.70 (ii)
Unit t Hyd. Tpeak (mi n) = 5.00
Unit t Hyd. peak (cms) = 0.33
PEAK FLOW (cms) = 0.05
TIME TO PEAK (hrs) = 1.00
RUNOFF VOLUME (mm) = 44.35
TOTAL RAINFALL (mm) = 45.35
RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 45.35 mm IDF curve parameters: A=1330.300 B= 7.938 C= 0.855
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show data for times 0.17, 0.33, 0.50, 0.67, 0.83.

CALIB STANDHYD ( 0002) ID= 1 DT= 5.0 min Area (ha) = 1.05 Total Imp(%) = 51.00 Dir. Conn. (%) = 51.00

Surface Area (ha) = 0.54
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 83.67
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show data for times 0.083 to 0.750.

TIME TO PEAK (hrs) = 1.00
RUNOFF VOLUME (mm) = 44.35
TOTAL RAINFALL (mm) = 45.35
RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
+ ID1= 1 ( 0001): 1.04 0.185 1.00 34.93
+ ID2= 2 ( 0002): 1.05 0.195 1.00 35.47

ID = 3 ( 0010): 2.09 0.380 1.00 35.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
+ ID1= 3 ( 0010): 2.09 0.380 1.00 35.20
+ ID2= 2 ( 0003): 1.06 0.199 1.00 35.65

ID = 1 ( 0010): 3.15 0.579 1.00 35.35

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 1 + 2 = 3 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
+ ID1= 1 ( 0010): 3.15 0.579 1.00 35.35
+ ID2= 2 ( 0004): 0.22 0.058 1.00 39.81

ID = 3 ( 0010): 3.37 0.637 1.00 35.65

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
+ ID1= 3 ( 0010): 3.37 0.637 1.00 35.65
+ ID2= 2 ( 0007): 2.05 0.075 1.17 10.89

ID = 1 ( 0010): 5.42 0.679 1.00 26.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM Ptotal = 45.35 mm IDF curve parameters: A=1330.300 B= 7.938 C= 0.855
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr, TIME hrs, RAIN mm/hr. Rows show data for times 0.17, 0.33, 0.50, 0.67, 0.83.



VO Output - Prop.txt

Table with 6 columns: Time (hrs), Rain (mm/hr), Time (hrs), Rain (mm/hr), Time (hrs), Rain (mm/hr). Rows show rainfall intensity over time.

CALIB STANDHYD ( 0008) ID= 1 DT= 5.0 min Area (ha)= 14.26 Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i) Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 308.33 40.00 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity.

Max. Eff. Inten. (mm/hr)= 112.71 64.47 over (min)= 5.00 15.00 Storage Coeff. (mi n)= 4.79 (ii) 13.20 (ii) Unit Hyd. Tpeak (mi n)= 5.00 15.00 Unit Hyd. peak (cms)= 0.22 0.08

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM ID= 1 DT= 5.0 min IDf curve parameters: A=1330.300 B= 7.938 C= 0.855 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity.

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CALIB STANDHYD ( 0006) ID= 1 DT= 5.0 min Area (ha)= 1.17 Total Imp(%)= 64.00 Dir. Conn.(%)= 64.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i) Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 88.32 40.00 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity.

Max. Eff. Inten. (mm/hr)= 112.71 64.47 over (min)= 5.00 15.00 Storage Coeff. (mi n)= 2.26 (ii) 10.67 (ii) Unit Hyd. Tpeak (mi n)= 5.00 15.00 Unit Hyd. peak (cms)= 0.30 0.09

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM ID= 1 DT= 5.0 min IDf curve parameters: A=1330.300 B= 7.938 C= 0.855 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity.

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STANDHYD ( 0005) ID= 1 DT= 5.0 min Area (ha)= 2.30 Total Imp(%)= 70.00 Dir. Conn.(%)= 70.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i) Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 123.83 40.00 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity.

Max. Eff. Inten. (mm/hr)= 112.71 64.47 over (min)= 5.00 10.00 Storage Coeff. (mi n)= 2.77 (ii) 7.56 (ii) Unit Hyd. Tpeak (mi n)= 5.00 10.00 Unit Hyd. peak (cms)= 0.28 0.13

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020) 1 + 2 = 3 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID1= 1 ( 0005): 2.30 0.566 1.00 38.92 + ID2= 2 ( 0006): 1.17 0.256 1.00 37.83

ID = 3 ( 0020): 3.47 0.822 1.00 38.55

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020) 3 + 2 = 1 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID1= 3 ( 0020): 3.47 0.822 1.00 38.55 + ID2= 2 ( 0008): 14.26 2.365 1.00 35.30

ID = 1 ( 0020): 17.73 3.187 1.00 35.93

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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VO Output - Prop.txt (v 5.2.2003) V V I SSSSS U U A L V V I SSSSS U U A L V V I SS U U A A A A L V V I SS U U A A A L V V I SSSSS UUUU U A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM 0 0 T T T H H Y Y M M 0 0 0 0 T T T H H Y Y M M 0 0 0 0

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO\vo\in.dat

Output filename: C:\Users\p001279d\AppData\Local\Civi ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\b99f66a0-50db-421a-8092-91bd04dab910\scse Summary filename: C:\Users\p001279d\AppData\Local\Civi ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\b99f66a0-50db-421a-8092-91bd04dab910\scse

DATE: 11/12/2019 TIME: 11:34:03

USER:

COMMENTS:

CHI CAGO STORM ID= 1 DT= 5.0 min IDf curve parameters: A=1496.300 B= 8.250 C= 0.825 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

TRANSFORMED HYETOGRAPH table with columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Shows transformed rainfall intensity.

CALIB STANDHYD ( 0030) ID= 1 DT= 5.0 min Area (ha)= 1.58 Total Imp(%)= 65.00 Dir. Conn.(%)= 65.00

Surface Area (ha)= IMPERVIOUS PERVIOUS (i) Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 102.63 40.00 Mannings n = 0.013 0.250

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VO Output - Prop.txt

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data at various time intervals.

Max. Eff. Inten. (mm/hr) = 158.06
over (min) = 5.00
Storage Coeff. (mi n) = 2.16 (ii)

Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.31
\*TOTALS\* PEAK FLOW (cms) = 0.45

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM IDF curve parameters: A=1496.300
Total = 60.39 mm
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

CALIB STANDHYD ( 0007) Area (ha) = 2.05
Curve Number (CN) = 70.0
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

CALIB STANDHYD ( 0007) Area (ha) = 2.05
Curve Number (CN) = 70.0
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TOTAL RAINFALL (mm) = 60.39
RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM IDF curve parameters: A=1496.300
Total = 60.39 mm
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

CALIB STANDHYD ( 0003) Area (ha) = 1.06
Total Imp(%) = 52.00
Dir. Conn. (%) = 52.00

Surface Area (ha) = 0.55
Impervious (i) = 0.51
Dep. Storage (mm) = 1.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

Max. Eff. Inten. (mm/hr) = 158.06
over (mi n) = 5.00
Storage Coeff. (mi n) = 1.92 (ii)

Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.31
\*TOTALS\* PEAK FLOW (cms) = 0.24

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - Prop.txt

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

Unit Hyd. Tpeak (cms) = 0.391

PEAK FLOW (cms) = 0.134 (i)

TIME TO PEAK (hrs) = 1.167

RUNOFF VOLUME (mm) = 18.646

TOTAL RAINFALL (mm) = 60.393

RUNOFF COEFFICIENT = 0.309

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM IDF curve parameters: A=1496.300
Total = 60.39 mm
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

CALIB STANDHYD ( 0001) Area (ha) = 1.04
Total Imp(%) = 48.00
Dir. Conn. (%) = 48.00

Surface Area (ha) = 0.50
Impervious (i) = 0.54
Dep. Storage (mm) = 1.00

Average Slope (%) = 1.00

Length (m) = 83.27

Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

Max. Eff. Inten. (mm/hr) = 158.06
over (mi n) = 5.00
Storage Coeff. (mi n) = 1.91 (ii)

Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.32
\*TOTALS\* PEAK FLOW (cms) = 0.22

TIME TO PEAK (hrs) = 1.167
RUNOFF VOLUME (mm) = 59.39

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

CHICAGO STORM IDF curve parameters: A=1496.300
Total = 60.39 mm
used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

CALIB STANDHYD ( 0004) Area (ha) = 0.22
Total Imp(%) = 75.00
Dir. Conn. (%) = 75.00

Surface Area (ha) = 0.17
Impervious (i) = 0.05
Dep. Storage (mm) = 1.00

Average Slope (%) = 1.00

Length (m) = 38.30

Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TRANSFORMED HYETOGRAPH, TIME, RAIN. Rows show rainfall data.

Max. Eff. Inten. (mm/hr) = 158.06
over (mi n) = 5.00
Storage Coeff. (mi n) = 1.20 (ii)

Unit Hyd. Tpeak (mi n) = 5.00
Unit Hyd. peak (cms) = 0.33
\*TOTALS\* PEAK FLOW (cms) = 0.07

TIME TO PEAK (hrs) = 1.00
RUNOFF VOLUME (mm) = 59.37
TOTAL RAINFALL (mm) = 60.39

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - Prop.txt

CHI CAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.583	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha)= 1.05  
Total Imp(%)= 51.00 Dir. Conn. (%)= 51.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.54 0.51  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 83.67 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73
0.167	4.89	0.917	158.06	1.667	11.10	2.42	5.14
0.250	6.10	1.000	158.06	1.750	8.94	2.50	5.14
0.333	6.10	1.083	44.35	1.833	8.94	2.58	4.66
0.417	8.21	1.167	44.35	1.917	7.51	2.67	4.66
0.500	8.21	1.250	22.12	2.000	7.51	2.75	4.27
0.583	12.90	1.333	22.12	2.083	6.49	2.83	4.27
0.667	12.90	1.417	14.74	2.167	6.49	2.92	3.95
0.750	33.21	1.500	14.74	2.250	5.73	3.00	3.95

Max. Eff. Inten. (mm/hr)= 158.06 104.59  
over (min)= 5.00 10.00  
Storage Coeff. (mi n)= 1.91 (ii) 8.84 (iii)  
Unit Hyd. Tpeak (mi n)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.31 0.12

PEAK FLOW (cms)= 0.23 0.10 \*TOTALS\*  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 59.39 39.37 49.58  
TOTAL RAINFALL (mm)= 60.39 60.39 60.39  
RUNOFF COEFFICIENT = 0.98 0.65 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN= 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)

VO Output - Prop.txt  
Length (m)= 308.33 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73
0.167	4.89	0.917	158.06	1.667	11.10	2.42	5.14
0.250	6.10	1.000	158.06	1.750	8.94	2.50	5.14
0.333	6.10	1.083	44.35	1.833	8.94	2.58	4.66
0.417	8.21	1.167	44.35	1.917	7.51	2.67	4.66
0.500	8.21	1.250	22.12	2.000	7.51	2.75	4.27
0.583	12.90	1.333	22.12	2.083	6.49	2.83	4.27
0.667	12.90	1.417	14.74	2.167	6.49	2.92	3.95
0.750	33.21	1.500	14.74	2.250	5.73	3.00	3.95

Max. Eff. Inten. (mm/hr)= 158.06 104.59  
over (min)= 5.00 15.00  
Storage Coeff. (mi n)= 4.18 (ii) 11.11 (iii)  
Unit Hyd. Tpeak (mi n)= 5.00 15.00  
Unit Hyd. peak (cms)= 0.24 0.09

PEAK FLOW (cms)= 2.90 1.16 3.530 (iii)  
TIME TO PEAK (hrs)= 1.17 1.00 1.00  
RUNOFF VOLUME (mm)= 59.39 39.37 49.58  
TOTAL RAINFALL (mm)= 60.39 60.39 60.39  
RUNOFF COEFFICIENT = 0.98 0.65 0.82

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN= 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.583	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
STANDHYD ( 0006)  
ID= 1 DT= 5.0 min

Area (ha)= 1.17  
Total Imp(%)= 64.00 Dir. Conn. (%)= 64.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 0.75 0.42  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 88.32 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

VO Output - Prop.txt

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0001):	1.04	0.304	1.00	48.97
+ ID2= 2 ( 0002):	1.05	0.316	1.00	49.58
ID = 3 ( 0010):	2.09	0.619	1.00	49.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

3 + 2 = 1	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0010):	2.09	0.619	1.00	49.28
+ ID2= 2 ( 0003):	1.06	0.321	1.00	49.78
ID = 1 ( 0010):	3.15	0.941	1.00	49.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
1 + 2 = 3

1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0010):	3.15	0.941	1.00	49.44
+ ID2= 2 ( 0004):	0.22	0.087	1.00	54.37
ID = 3 ( 0010):	3.37	1.028	1.00	49.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)  
3 + 2 = 1

3 + 2 = 1	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0010):	3.37	1.028	1.00	49.77
+ ID2= 2 ( 0007):	2.05	0.134	1.17	18.65
ID = 1 ( 0010):	5.42	1.109	1.00	38.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
STANDHYD ( 0008)  
ID= 1 DT= 5.0 min

Area (ha)= 14.26  
Total Imp(%)= 50.00 Dir. Conn. (%)= 50.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 7.13 7.13  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00

VO Output - Prop.txt

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73
0.167	4.89	0.917	158.06	1.667	11.10	2.42	5.14
0.250	6.10	1.000	158.06	1.750	8.94	2.58	4.66
0.333	6.10	1.083	44.35	1.833	8.94	2.50	5.14
0.417	8.21	1.167	44.35	1.917	7.51	2.67	4.66
0.500	8.21	1.250	22.12	2.000	7.51	2.75	4.27
0.583	12.90	1.333	22.12	2.083	6.49	2.83	4.27
0.667	12.90	1.417	14.74	2.167	6.49	2.92	3.95
0.750	33.21	1.500	14.74	2.250	5.73	3.00	3.95

Max. Eff. Inten. (mm/hr)= 158.06 104.59  
over (mi n)= 5.00 10.00  
Storage Coeff. (mi n)= 1.97 (ii) 6.64 (ii)

Unit Hyd. Tpeak (mi n)= 5.00 10.00  
Unit Hyd. peak (cms)= 0.31 0.14  
PEAK FLOW (cms)= 0.33 0.09 \*TOTALS\*  
TIME TO PEAK (hrs)= 1.00 1.08 1.00  
RUNOFF VOLUME (mm)= 59.39 39.37 52.18  
TOTAL RAINFALL (mm)= 60.39 60.39 60.39  
RUNOFF COEFFICIENT = 0.98 0.65 0.86

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN= 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
STANDHYD ( 0005)  
ID= 1 DT= 5.0 min

Area (ha)= 2.30  
Total Imp(%)= 70.00 Dir. Conn. (%)= 70.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 1.61 0.69  
Dep. Storage (mm)= 1.00 2.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 123.83 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	4.89	0.833	33.21	1.583	11.10	2.33	5.73

VO Output - Prop.txt							
0.167	4.89	0.917	158.06	1.667	11.10	2.42	5.14
0.250	6.10	1.000	158.06	1.750	8.94	2.50	5.14
0.333	6.10	1.083	44.35	1.833	8.94	2.58	4.66
0.417	8.21	1.167	44.35	1.917	7.51	2.67	4.66
0.500	8.21	1.250	22.12	2.000	7.51	2.75	4.27
0.583	12.90	1.333	22.12	2.083	6.49	2.83	4.27
0.667	12.90	1.417	14.74	2.167	6.49	2.92	3.95
0.750	33.21	1.500	14.74	2.250	5.73	3.00	3.95

Max. Eff. Inten. (mm/hr) = 158.06  
 over (min) = 5.00  
 Storage Coeff. (mi n) = 2.42 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.30

\*TOTALS\*  
 PEAK FLOW (cms) = 0.70  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 59.39  
 TOTAL RAINFALL (mm) = 60.39  
 RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN1 = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 ( 0005):	2.30	0.825	1.00	53.38
+ ID2 = 2 ( 0006):	1.17	3.404	1.00	52.18
ID = 3 ( 0020):	3.47	1.229	1.00	52.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)				
ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 3 ( 0020):	3.47	1.229	1.00	52.98
+ ID2 = 2 ( 0008):	14.26	3.530	1.00	49.38
ID = 1 ( 0020):	17.73	4.759	1.00	50.08

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U AAAAA L  
 V V I SS U U A A L  
 V V I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y M M 0 0  
 0 0 T T H H Y Y M M 0 0  
 000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

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VO Output - Prop.txt

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output filename:  
 C:\Users\p001279\AppData\Local\Civica\VH5\bcccece0-2cc7-4973-8ab8-dd0de50b3a28\6ce217  
 2f-4475-46e2-8500-ef29a9862c79\scce  
 Summary File name:  
 C:\Users\p001279\AppData\Local\Civica\VH5\bcccece0-2cc7-4973-8ab8-dd0de50b3a28\6ce217  
 2f-4475-46e2-8500-ef29a9862c79\scce

DATE: 11/12/2019

TIME: 11:34:02

USER:

COMMENTS:

\*\*\*\*\*  
 \*\* SIMULATION : 11 Chicago - 3hr 100-yr \*\*  
 \*\*\*\*\*

CHI CAGO STORM IDF curve parameters: A=1499.500  
 Ptotal = 71.76 mm B= 3.298  
 C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB STANDHYD ( 0030) Area (ha) = 1.58  
 Total Imp(%) = 65.00 Dir. Conn. (%) = 65.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 1.03 0.55  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 102.63 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.35	0.833	36.28	1.583	13.33	2.33	7.33
0.167	6.35	0.917	192.16	1.667	13.33	2.42	6.64
0.250	7.76	1.000	192.16	1.750	10.97	2.50	6.64
0.333	7.76	1.083	47.74	1.833	10.97	2.58	6.07
0.417	10.16	1.167	47.74	1.917	9.37	2.67	6.07
0.500	10.16	1.250	24.88	2.000	9.37	2.75	5.61
0.583	15.26	1.333	24.88	2.083	8.21	2.83	5.61
0.667	15.26	1.417	17.22	2.167	8.21	2.92	5.22
0.750	36.28	1.500	17.22	2.250	7.33	3.00	5.22

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Max. Eff. Inten. (mm/hr) = 192.16  
 over (mi n) = 5.00  
 Storage Coeff. (mi n) = 2.00 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.31

\*TOTALS\*  
 PEAK FLOW (cms) = 0.55  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 70.76  
 TOTAL RAINFALL (mm) = 71.76  
 RUNOFF COEFFICIENT = 0.99

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN1 = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1499.500  
 Ptotal = 71.76 mm B= 3.298  
 C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB NASHYD ( 0007) Area (ha) = 2.05 Curve Number (CN) = 70.0  
 ID = 1 DT = 5.0 min Ia (mm) = 5.00 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.35	0.833	36.28	1.583	13.33	2.33	7.33
0.167	6.35	0.917	192.16	1.667	13.33	2.42	6.64
0.250	7.76	1.000	192.16	1.750	10.97	2.50	6.64
0.333	7.76	1.083	47.74	1.833	10.97	2.58	6.07
0.417	10.16	1.167	47.74	1.917	9.37	2.67	6.07
0.500	10.16	1.250	24.88	2.000	9.37	2.75	5.61
0.583	15.26	1.333	24.88	2.083	8.21	2.83	5.61
0.667	15.26	1.417	17.22	2.167	8.21	2.92	5.22
0.750	36.28	1.500	17.22	2.250	7.33	3.00	5.22

Unit Hyd Opeak (cms) = 0.391

PEAK FLOW (cms) = 0.184 (i)  
 TIME TO PEAK (hrs) = 1.167  
 RUNOFF VOLUME (mm) = 25.328  
 TOTAL RAINFALL (mm) = 71.757  
 RUNOFF COEFFICIENT = 0.353

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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VO Output - Prop.txt

CHI CAGO STORM IDF curve parameters: A=1499.500  
 Ptotal = 71.76 mm B= 3.298  
 C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB STANDHYD ( 0001) Area (ha) = 1.04  
 Total Imp(%) = 48.00 Dir. Conn. (%) = 48.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 0.50 0.54  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 83.27 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	6.35	0.833	36.28	1.583	13.33	2.33	7.33
0.167	6.35	0.917	192.16	1.667	13.33	2.42	6.64
0.250	7.76	1.000	192.16	1.750	10.97	2.50	6.64
0.333	7.76	1.083	47.74	1.833	10.97	2.58	6.07
0.417	10.16	1.167	47.74	1.917	9.37	2.67	6.07
0.500	10.16	1.250	24.88	2.000	9.37	2.75	5.61
0.583	15.26	1.333	24.88	2.083	8.21	2.83	5.61
0.667	15.26	1.417	17.22	2.167	8.21	2.92	5.22
0.750	36.28	1.500	17.22	2.250	7.33	3.00	5.22

Max. Eff. Inten. (mm/hr) = 192.16  
 over (mi n) = 5.00  
 Storage Coeff. (mi n) = 1.76 (ii)  
 Unit Hyd. Tpeak (mi n) = 5.00  
 Unit Hyd. peak (cms) = 0.32

\*TOTALS\*  
 PEAK FLOW (cms) = 0.27  
 TIME TO PEAK (hrs) = 1.00  
 RUNOFF VOLUME (mm) = 70.76  
 TOTAL RAINFALL (mm) = 71.76  
 RUNOFF COEFFICIENT = 0.99

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN1 = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1499.500  
 Ptotal = 71.76 mm B= 3.298  
 C= 0.794

used in: INTENSITY = A / (t + B)^C

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VO Output - Prop.txt

Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts.

CALIB STANDBYD ( 0003) ID= 1 DT= 5.0 min Area (ha)= 1.06 Total Imp(%)= 52.00 Dir. Conn.(%)= 52.00

Surface Area (ha)= 0.55 IMPERVIOUS PERVIOUS (i) Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 84.06 40.00 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed rainfall data.

Max. Eff. Inten. (mm/hr)= 192.16 136.59 over (min)= 5.00 10.00 Storage Coeff. (mi n)= 1.77 (ii) 8.00 (ii) Unit Hyd. Tpeak (mi n)= 5.00 10.00 Unit Hyd. peak (cms)= 0.32 0.13

PEAK FLOW (cms)= 0.29 0.13 0.405 (iii) TIME TO PEAK (hrs)= 1.00 1.08 1.00 RUNOFF VOLUME (mm)= 70.76 49.66 60.63 TOTAL RAINFALL (mm)= 71.76 71.76 71.76 RUNOFF COEFFICIENT = 0.99 0.69 0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN= 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDf curve parameters: A=1499.500 B= 3.298 C= 0.794 Total= 71.76 mm used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts.

CALIB STANDBYD ( 0004) ID= 1 DT= 5.0 min Area (ha)= 0.22 Total Imp(%)= 75.00 Dir. Conn.(%)= 75.00

Surface Area (ha)= 0.17 0.05 Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 38.30 40.00 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed rainfall data.

Max. Eff. Inten. (mm/hr)= 192.16 136.59 over (mi n)= 5.00 5.00 Storage Coeff. (mi n)= 1.11 (ii) 4.61 (ii) Unit Hyd. Tpeak (mi n)= 5.00 5.00 Unit Hyd. peak (cms)= 0.34 0.22

PEAK FLOW (cms)= 0.09 0.02 0.108 (iii) TIME TO PEAK (hrs)= 1.00 1.00 1.00 RUNOFF VOLUME (mm)= 70.76 49.66 65.47 TOTAL RAINFALL (mm)= 71.76 71.76 71.76 RUNOFF COEFFICIENT = 0.99 0.69 0.91

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN= 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDf curve parameters: A=1499.500 B= 3.298 C= 0.794 Total= 71.76 mm used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts.

CALIB STANDBYD ( 0002) ID= 1 DT= 5.0 min Area (ha)= 1.05 Total Imp(%)= 51.00 Dir. Conn.(%)= 51.00

Surface Area (ha)= 0.54 0.51 IMPERVIOUS PERVIOUS (i) Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 83.67 40.00 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed rainfall data.

Max. Eff. Inten. (mm/hr)= 192.16 136.59 over (mi n)= 5.00 10.00 Storage Coeff. (mi n)= 1.77 (ii) 8.00 (ii) Unit Hyd. Tpeak (mi n)= 5.00 10.00 Unit Hyd. peak (cms)= 0.32 0.13

PEAK FLOW (cms)= 0.29 0.13 0.398 (iii) TIME TO PEAK (hrs)= 1.00 1.08 1.00 RUNOFF VOLUME (mm)= 70.76 49.66 60.63 TOTAL RAINFALL (mm)= 71.76 71.76 71.76 RUNOFF COEFFICIENT = 0.99 0.69 0.84

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN= 90.0 Ia = Dep. Storage (Above) (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT. (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010) 1 + 2 = 3 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID1= 1 ( 0001): 1.04 0.384 1.00 59.78 + ID2= 2 ( 0002): 1.05 0.398 1.00 60.42 ID = 3 ( 0010): 2.09 0.782 1.00 60.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)

ID1= 3 ( 0010): 2.09 0.782 1.00 60.10 + ID2= 2 ( 0003): 1.06 0.405 1.00 60.63

ID = 1 ( 0010): 3.15 1.187 1.00 60.28

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 1 + 2 = 3 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) ID1= 1 ( 0010): 3.15 1.187 1.00 60.28 + ID2= 2 ( 0004): 0.22 0.108 1.00 65.47 ID = 3 ( 0010): 3.37 1.295 1.00 60.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010) 3 + 2 = 1 AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm) ID1= 3 ( 0010): 3.37 1.295 1.00 60.62 + ID2= 2 ( 0007): 2.05 0.184 1.17 25.33 ID = 1 ( 0010): 5.42 1.412 1.00 47.27

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM IDf curve parameters: A=1499.500 B= 3.298 C= 0.794 Total= 71.76 mm used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs Storm time step = 10.00 min Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show time intervals and corresponding rainfall amounts.

CALIB STANDBYD ( 0008) ID= 1 DT= 5.0 min Area (ha)= 14.26 Total Imp(%)= 50.00 Dir. Conn.(%)= 50.00

Surface Area (ha)= 7.13 7.13 IMPERVIOUS PERVIOUS (i) Dep. Storage (mm)= 1.00 2.00 Average Slope (%)= 1.00 2.00 Length (m)= 308.33 40.00 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show transformed rainfall data.



VO Output - Prop.txt  
THAN THE STORAGE COEFFICIENT  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO Output - Prop.txt

CHICAGO STORM  
Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
B= 2.188  
C= 0.778

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB  
STANDHYD ( 0007)  
ID= 1 DT= 5.0 min

Area (ha) = 2.05  
Curve Number (CN) = 70.0  
Ia (mm) = 5.00  
U.H. Tp(hrs) = 0.20  
# of Linear Res. (N) = 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Unit Hyd. Peak (cms) = 0.391

PEAK FLOW (cms) = 0.215 (i)

TIME TO PEAK (hrs) = 1.167

RUNOFF VOLUME (mm) = 29.435

TOTAL RAINFALL (mm) = 78.293

RUNOFF COEFFICIENT = 0.376

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
B= 2.188  
C= 0.778

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

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STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha) = 1.06  
Total Imp(%) = 52.00  
Dir. Conn. (%) = 52.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.55  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 84.06  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13

over (min) = 5.00

Storage Coeff. (mi n) = 1.70 (ii)

Unit Hyd. Tpeak (mi n) = 5.00

Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.33

TIME TO PEAK (hrs) = 1.00

RUNOFF VOLUME (mm) = 77.29

TOTAL RAINFALL (mm) = 78.29

RUNOFF COEFFICIENT = 0.99

\*TOTALS\*  
0.459 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 90.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
B= 2.188  
C= 0.778

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB  
STANDHYD ( 0004)  
ID= 1 DT= 5.0 min

Area (ha) = 0.22  
Total Imp(%) = 75.00  
Dir. Conn. (%) = 75.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.17

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CALIB  
STANDHYD ( 0001)  
ID= 1 DT= 5.0 min

Area (ha) = 1.04  
Total Imp(%) = 48.00  
Dir. Conn. (%) = 48.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.50  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 83.27  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13

over (mi n) = 5.00

Storage Coeff. (mi n) = 1.69 (ii)

Unit Hyd. Tpeak (mi n) = 5.00

Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.30

TIME TO PEAK (hrs) = 1.00

RUNOFF VOLUME (mm) = 77.29

TOTAL RAINFALL (mm) = 78.29

RUNOFF COEFFICIENT = 0.99

\*TOTALS\*  
0.437 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 90.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
B= 2.188  
C= 0.778

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

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STANDHYD ( 0003)  
ID= 1 DT= 5.0 min

Area (ha) = 1.06  
Total Imp(%) = 52.00  
Dir. Conn. (%) = 52.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.55  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 84.06  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13

over (mi n) = 5.00

Storage Coeff. (mi n) = 1.06 (ii)

Unit Hyd. Tpeak (mi n) = 5.00

Unit Hyd. peak (cms) = 0.34

PEAK FLOW (cms) = 0.10

TIME TO PEAK (hrs) = 1.00

RUNOFF VOLUME (mm) = 77.29

TOTAL RAINFALL (mm) = 78.29

RUNOFF COEFFICIENT = 0.99

\*TOTALS\*  
0.121 (iii)

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 90.0 Ia = Dep. Storage (Above)

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.

(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 78.29 mm

IDF curve parameters: A=1498.100  
B= 2.188  
C= 0.778

used in: INTENSITY =  $A / (t + B)^C$

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB  
STANDHYD ( 0002)  
ID= 1 DT= 5.0 min

Area (ha) = 1.05  
Total Imp(%) = 51.00  
Dir. Conn. (%) = 51.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 0.54  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 83.67  
Mannings n = 0.013

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VO Output - Prop.txt  
NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---				---			
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13  
over (min) = 5.00  
Storage Coeff. (mi n) = 1.69 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.32  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 78.29  
RUNOFF COEFFICIENT = 0.99

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0010)				
1 + 2 = 1	AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)	
ID1 = 1 ( 0001):	1.04	0.437	1.00	66.06
+ ID2 = 2 ( 0002):	1.05	0.451	1.00	66.70
ID = 3 ( 0010):	2.09	0.888	1.00	66.38

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)				
3 + 2 = 1	AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)	
ID1 = 1 ( 0001):	2.09	0.888	1.00	66.38
+ ID2 = 2 ( 0003):	1.06	0.459	1.00	66.92
ID = 1 ( 0010):	3.15	1.347	1.00	66.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0010)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)	
ID1 = 1 ( 0010):	3.15	1.347	1.00	66.56
+ ID2 = 2 ( 0004):	0.22	0.121	1.00	71.87
ID = 3 ( 0010):	3.37	1.468	1.00	66.91

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM				
Total = 78.29 mm	IDF curve parameters:	A=1498.100	B= 2.188	C= 0.778
used in:	INTENSITY	= A / (t + B) <sup>C</sup>		
Duration of storm	=	3.00 hrs		
Storm time step	=	10.00 min		
Time to peak ratio	=	0.33		

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB STANDHYD ( 0006)				
ID = 1 DT = 5.0 min	Area	(ha)	Total Imp (%) = 64.00	Dir. Conn. (%) = 64.00

Surface Area (ha) = 0.75  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 88.32  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---				---			
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 1.75 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.32

PEAK FLOW (cms) = 0.44  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 78.29  
RUNOFF COEFFICIENT = 0.99

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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VO Output - Prop.txt

ADD HYD ( 0010)				
3 + 2 = 1	AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)	
ID1 = 3 ( 0010):	3.37	1.468	1.00	66.91
+ ID2 = 2 ( 0007):	2.05	0.215	1.17	29.43
ID = 1 ( 0010):	5.42	1.609	1.00	52.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM				
Total = 78.29 mm	IDF curve parameters:	A=1498.100	B= 2.188	C= 0.778
used in:	INTENSITY	= A / (t + B) <sup>C</sup>		
Duration of storm	=	3.00 hrs		
Storm time step	=	10.00 min		
Time to peak ratio	=	0.33		

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB STANDHYD ( 0008)				
ID = 1 DT = 5.0 min	Area	(ha) = 14.26	Total Imp (%) = 50.00	Dir. Conn. (%) = 50.00

Surface Area (ha) = 7.13  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 308.33  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---				---			
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 3.70 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.25

PEAK FLOW (cms) = 4.00  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 78.29  
RUNOFF COEFFICIENT = 0.99

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

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VO Output - Prop.txt

CHI CAGO STORM				
Total = 78.29 mm	IDF curve parameters:	A=1498.100	B= 2.188	C= 0.778
used in:	INTENSITY	= A / (t + B) <sup>C</sup>		
Duration of storm	=	3.00 hrs		
Storm time step	=	10.00 min		
Time to peak ratio	=	0.33		

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB STANDHYD ( 0005)				
ID = 1 DT = 5.0 min	Area	(ha) = 2.30	Total Imp (%) = 70.00	Dir. Conn. (%) = 70.00

Surface Area (ha) = 1.61  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 123.83  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 5.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---				---			
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.083	7.21	0.833	37.27	1.583	14.57	2.33	8.28
0.167	7.21	0.917	214.13	1.667	14.57	2.42	7.53
0.250	8.74	1.000	214.13	1.750	12.13	2.50	7.53
0.333	8.74	1.083	48.43	1.833	12.13	2.58	6.92
0.417	11.28	1.167	48.43	1.917	10.45	2.67	6.92
0.500	11.28	1.250	26.15	2.000	10.45	2.75	6.41
0.583	16.53	1.333	26.15	2.083	9.22	2.83	6.41
0.667	16.53	1.417	18.53	2.167	9.22	2.92	5.98
0.750	37.27	1.500	18.53	2.250	8.28	3.00	5.98

Max. Eff. Inten. (mm/hr) = 214.13  
over (mi n) = 5.00  
Storage Coeff. (mi n) = 2.14 (ii)  
Unit Hyd. Tpeak (mi n) = 5.00  
Unit Hyd. peak (cms) = 0.31

PEAK FLOW (cms) = 0.95  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 77.29  
TOTAL RAINFALL (mm) = 78.29  
RUNOFF COEFFICIENT = 0.99

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 90.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0020)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R. V.
(ha)	(cms)	(hrs)	(mm)	

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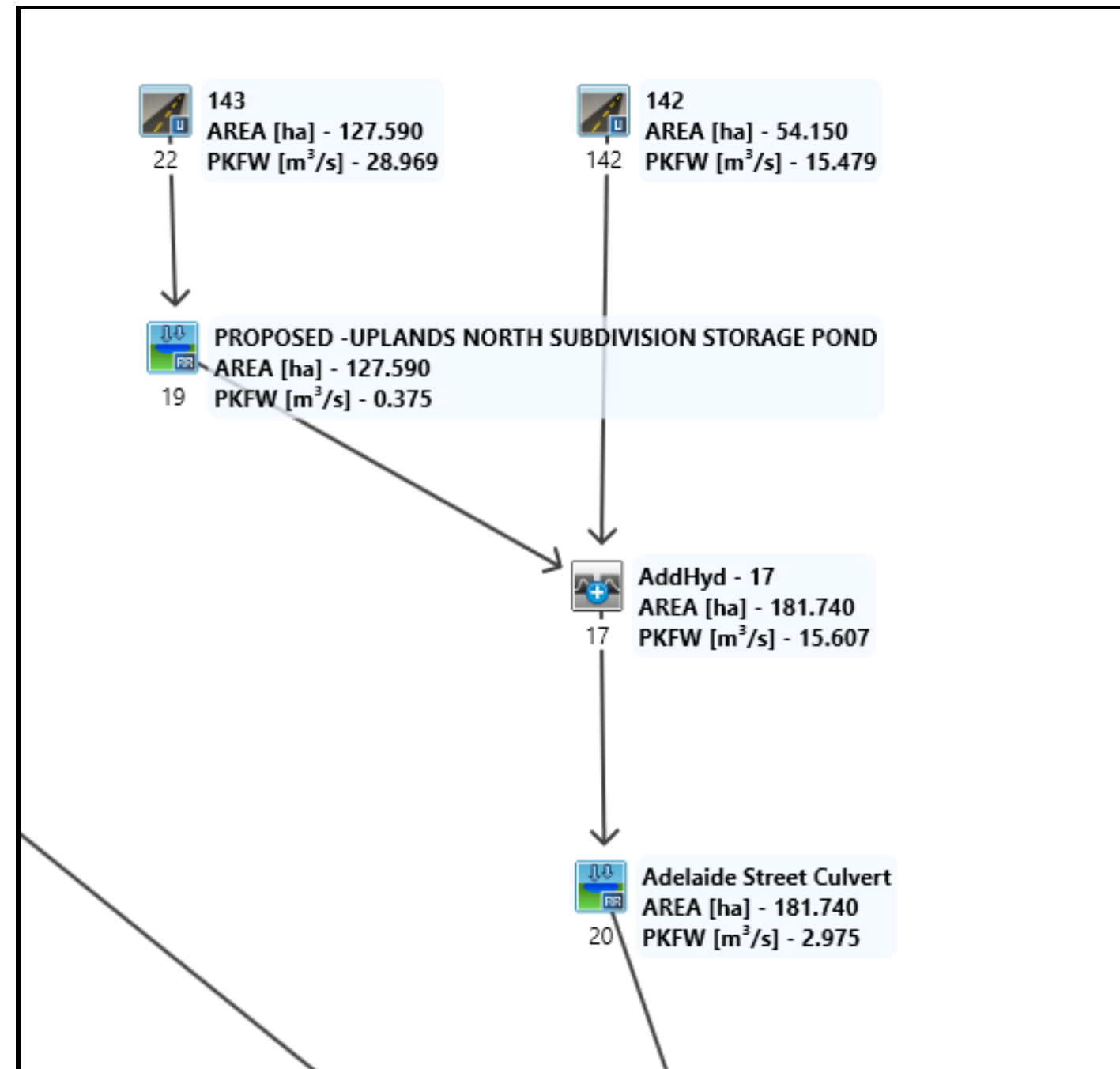
VO Output - Prop. txt				
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0005):	2.30	1.153	1.00	70.81
+ ID2= 2 ( 0006):	1.17	0.568	1.00	69.51
-----				
ID = 3 ( 0020):	3.47	1.721	1.00	70.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0020)				
3 + 2 = 1	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
ID1= 3 ( 0020):	3.47	1.721	1.00	70.37
+ ID2= 2 ( 0008):	14.26	5.530	1.00	66.49
-----				
ID = 1 ( 0020):	17.73	7.350	1.00	67.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

### Visual OTTHYMO Modelling Schematic-3 - Upstream Drainage



VO output - Prop Uncontrolled Upstream.txt

```

*****
V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
W I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000
Developed and Distributed by Civica Infrastructure
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```

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYM0 5.2\VO2\voi.n.dat  
Output filename:  
C:\Users\p001279d\AppData\Local\Civica\VHS\b8ccee0-2cc7-4973-8ab8-dd0e50b3a28\4b43d35c-dfed-478a-a977-cb0364927108\scce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civica\VHS\b8ccee0-2cc7-4973-8ab8-dd0e50b3a28\4b43d35c-dfed-478a-a977-cb0364927108\scce

DATE: 11/28/2019 TIME: 11:04:52  
USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 01 AES 2-Yr \*\*\*\*\*

```

READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\376738e9
Ptotal = 34.03 mm      Comments: 2yr 1hr AES

TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.08  0.00      0.42  61.25     0.75  32.67     1.08  4.08
0.17  4.08      0.50  114.34    0.83  20.42
0.25  12.25     0.58  61.25     0.92  12.25
0.33  32.67     0.67  49.00     1.00  4.08

```

```

CALIB
NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0
ID= 1 DT=10.0 min Ia (mm) = 22.82 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

```

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

```

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.167  2.04      0.500  87.80     0.833  26.54     1.17  2.04
0.333  22.46     0.667  55.13     1.000  8.17

```

VO output - Prop Uncontrolled Upstream.txt

```

TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.08  0.00      0.42  61.25     0.75  32.67     1.08  4.08
0.17  4.08      0.50  114.34    0.83  20.42
0.25  12.25     0.58  61.25     0.92  12.25
0.33  32.67     0.67  49.00     1.00  4.08

```

```

CALIB
NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0
ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

```

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

```

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.167  2.04      0.500  87.80     0.833  26.54     1.17  2.04
0.333  22.46     0.667  55.13     1.000  8.17

```

```

Unit Hyd Opeak (cms) = 3.476
PEAK FLOW (cms) = 0.312 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 3.353
TOTAL RAINFALL (mm) = 34.030
RUNOFF COEFFICIENT = 0.099

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\376738e9
Ptotal = 34.03 mm      Comments: 2yr 1hr AES

TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.08  0.00      0.42  61.25     0.75  32.67     1.08  4.08
0.17  4.08      0.50  114.34    0.83  20.42
0.25  12.25     0.58  61.25     0.92  12.25
0.33  32.67     0.67  49.00     1.00  4.08

```

```

CALIB
STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00
ID= 1 DT=10.0 min Total Imp(%) = 30.00

```

```

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 4.50 10.50
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 3.00
Length (m) = 258.20 40.00
Mannings n = 0.015 0.200

```

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

```

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.167  2.04      0.500  87.80     0.833  26.54     1.17  2.04
0.333  22.46     0.667  55.13     1.000  8.17

```

Max. Eff. Inten. (mm/hr) = 87.80 15.83  
Page 3

```

VO output - Prop Uncontrolled Upstream.txt
0.167  2.04  0.500  87.80  0.833  26.54  1.17  2.04
0.333  22.46  0.667  55.13  1.000  8.17

```

Unit Hyd Opeak (cms) = 17.469

```

PEAK FLOW (cms) = 0.452 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 0.977
TOTAL RAINFALL (mm) = 34.030
RUNOFF COEFFICIENT = 0.029

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\376738e9
Ptotal = 34.03 mm      Comments: 2yr 1hr AES

TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.08  0.00      0.42  61.25     0.75  32.67     1.08  4.08
0.17  4.08      0.50  114.34    0.83  20.42
0.25  12.25     0.58  61.25     0.92  12.25
0.33  32.67     0.67  49.00     1.00  4.08

```

```

CALIB
NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

```

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

```

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.167  2.04      0.500  87.80     0.833  26.54     1.17  2.04
0.333  22.46     0.667  55.13     1.000  8.17

```

Unit Hyd Opeak (cms) = 17.302

```

PEAK FLOW (cms) = 0.564 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 1.209
TOTAL RAINFALL (mm) = 34.030
RUNOFF COEFFICIENT = 0.036

```

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

ADD HYD ( 0002)
1 + 2 = 3
AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
ID1= 1 ( 0152): 90.60 0.564 0.833 1.21
+ ID2= 2 ( 0153): 91.47 0.452 0.83 0.98
-----
ID = 3 ( 0002): 182.07 1.016 0.83 1.09

```

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```

READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\376738e9
Ptotal = 34.03 mm      Comments: 2yr 1hr AES
Page 2

```

VO output - Prop Uncontrolled Upstream.txt

```

Storage Coeff. over (mi n) = 10.00 20.00
Unit Hyd. Tpeak (mi n) = 5.18 (ii) 16.61 (ii)
Unit Hyd. peak (cms) = 0.15 0.06
PEAK FLOW (cms) = 0.93 0.33 *TOTALS*
TIME TO PEAK (hrs) = 0.50 0.83 1.013 (iii)
RUNOFF VOLUME (mm) = 33.03 7.65 15.01
TOTAL RAINFALL (mm) = 34.03 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.22 0.44

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 71.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

```

READ STORM      Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\376738e9
Ptotal = 34.03 mm      Comments: 2yr 1hr AES

TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.08  0.00      0.42  61.25     0.75  32.67     1.08  4.08
0.17  4.08      0.50  114.34    0.83  20.42
0.25  12.25     0.58  61.25     0.92  12.25
0.33  32.67     0.67  49.00     1.00  4.08

```

```

CALIB
STANDHYD ( 0142) Area (ha) = 59.20 Dir. Conn. (%) = 64.00
ID= 1 DT=10.0 min Total Imp(%) = 67.00

Surface Area (ha) = 39.66 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 16.04
Average Slope (%) = 1.00 3.00
Length (m) = 628.23 40.00
Mannings n = 0.015 0.250

```

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

```

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN      TIME RAIN      TIME RAIN      TIME RAIN
hrs  mm/hr     hrs  mm/hr     hrs  mm/hr     hrs  mm/hr
0.167  2.04      0.500  87.80     0.833  26.54     1.17  2.04
0.333  22.46     0.667  55.13     1.000  8.17

```

```

Max. Eff. Inten. (mm/hr) = 87.80 10.47
Storage Coeff. over (mi n) = 10.00 30.00
Unit Hyd. Tpeak (mi n) = 6.83 (ii) 24.24 (ii)
Unit Hyd. peak (cms) = 0.12 0.04
PEAK FLOW (cms) = 6.69 0.27 *TOTALS*
TIME TO PEAK (hrs) = 0.50 1.17 6.693 (iii)
RUNOFF VOLUME (mm) = 33.03 4.02 22.59
TOTAL RAINFALL (mm) = 34.03 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.12 0.66

```

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 76.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

VO output - Prop Uncontrolled Upstream.txt  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	61.25	0.75	32.67	1.08	4.08
0.17	4.08	0.50	114.34	0.83	20.42		
0.25	12.25	0.58	61.25	0.92	12.25		
0.33	32.67	0.67	49.00	1.00	4.08		

CALIB STANDHYD ( 0022)  
ID= 1 DT=10.0 min

Area (ha)= 123.05  
Total Imp(%)= 61.00 Dir. Conn.(%)= 49.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 75.06 47.99  
Dep. Storage (mm)= 2.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 905.72 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	2.04	0.500	87.80	0.833	26.54	1.17	2.04
0.333	22.46	0.667	55.13	1.000	8.17		

Max. Eff. Inten. (mm/hr)= 87.80 20.43  
over (min)= 10.00 30.00  
Storage Coeff. (min)= 10.10 (ii) 23.42 (ii)  
Unit Hyd. Tpeak (min)= 10.00 30.00  
Unit Hyd. peak (cms)= 0.11 0.04

PEAK FLOW (cms)= 9.73 1.58 10.082 (iii)  
TIME TO PEAK (hrs)= 0.50 0.67  
RUNOFF VOLUME (mm)= 32.03 7.77 19.66  
TOTAL RAINFALL (mm)= 34.03 34.03  
RUNOFF COEFFICIENT = 0.94 0.23

\*TOTALS\*  
10.082 (iii)

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0142):	59.20	6.693	0.50	22.59
+ ID2= 2 ( 0022):	123.05	10.082	0.67	19.66
ID = 3 ( 0017):	182.25	16.621	0.50	20.61

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Page 5

VO output - Prop Uncontrolled Upstream.txt

ADD HYD ( 0003) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 1421):	15.00	1.013	0.50	15.01
+ ID2= 2 ( 0151):	18.20	0.312	0.83	3.35
ID = 3 ( 0003):	33.20	1.190	0.67	8.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) 3 + 2 = 1	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0003):	33.20	1.190	0.67	8.62
+ ID2= 2 ( 0017):	182.25	16.621	0.50	20.61
ID = 1 ( 0003):	215.45	17.682	0.50	18.76

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0003):	215.45	17.682	0.50	18.76
+ ID2= 2 ( 0002):	182.07	1.016	0.83	1.09
ID = 3 ( 0003):	397.52	17.787	0.67	10.67

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5. 2. 003)  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSSS UUUUU A A LLLLL

000 TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 H H Y Y M M 000

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo\ni.nat

Output filename:  
C:\Users\p001279d\AppData\Local\Civica\VS\5b8ccee0-2cc7-4973-8ab8-dd0de50b3a28e10156  
ad-2343-4ecf-816f-c8c59262c665\sc  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civica\VS\5b8ccee0-2cc7-4973-8ab8-dd0de50b3a28e10156  
ad-2343-4ecf-816f-c8c59262c665\sc

DATE: 11/28/2019

TIME: 11:04:53

USER:

COMMENTS:

Page 6

VO output - Prop Uncontrolled Upstream.txt

\* \* \* \* \*  
SIMULATION : 02 AES 5-Yr  
\* \* \* \* \*

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB NASHYD ( 0153)  
ID= 1 DT=10.0 min

Area (ha)= 91.47 Curve Number (CN)= 69.0  
Ia (mm)= 22.82 # of Li near Res. (N)= 3.00  
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	2.08	0.500	89.37	0.833	27.02	1.17	2.08
0.333	22.86	0.667	56.12	1.000	8.31		

Unit Hyd Opeak (cms)= 17.469  
PEAK FLOW (cms)= 0.504 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 1.081  
TOTAL RAINFALL (mm)= 34.640  
RUNOFF COEFFICIENT = 0.031

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB NASHYD ( 0152)  
ID= 1 DT=10.0 min

Area (ha)= 90.60 Curve Number (CN)= 70.0  
Ia (mm)= 21.77 # of Li near Res. (N)= 3.00  
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Page 7

VO output - Prop Uncontrolled Upstream.txt

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	2.08	0.500	89.37	0.833	27.02	1.17	2.08
0.333	22.86	0.667	56.12	1.000	8.31		

Unit Hyd Opeak (cms)= 17.302

PEAK FLOW (cms)= 0.621 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 1.325  
TOTAL RAINFALL (mm)= 34.640  
RUNOFF COEFFICIENT = 0.038

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002) 1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0152):	90.60	0.621	0.83	1.33
+ ID2= 2 ( 0153):	91.47	0.504	0.83	1.08
ID = 3 ( 0002):	182.07	1.125	0.83	1.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	62.35	0.75	33.25	1.08	4.16
0.17	4.16	0.50	116.39	0.83	20.78		
0.25	12.47	0.58	62.35	0.92	12.47		
0.33	33.25	0.67	49.88	1.00	4.16		

CALIB NASHYD ( 0151)  
ID= 1 DT=10.0 min

Area (ha)= 18.20 Curve Number (CN)= 68.0  
Ia (mm)= 11.95 # of Li near Res. (N)= 3.00  
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	2.08	0.500	89.37	0.833	27.02	1.17	2.08
0.333	22.86	0.667	56.12	1.000	8.31		

Unit Hyd Opeak (cms)= 3.476

PEAK FLOW (cms)= 0.327 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 3.526  
TOTAL RAINFALL (mm)= 34.640  
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM | Filename: C:\Users\p001279d\AppData  
Page 8

VO output - Prop Uncontrolled Upstream.txt  
 ata\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c44996e4  
 Ptotal = 34.64 mm Comments: 5yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25
0.17	4.16	0.50	116.39	0.83	20.78
0.25	12.47	0.58	62.35	0.92	12.47
0.33	33.25	0.67	49.88	1.00	4.16

CALIB  
 STANDHYD ( 1421)  
 ID= 1 DT=10.0 min

Area (ha) = 15.00 Dir. Conn. (%) = 29.00  
 Total Imp (%) = 30.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.50	10.50
Dep. Storage (mm)	1.00	2.00
Average Slope (%)	1.00	3.00
Length (m)	258.20	40.00
Mannings n	0.015	0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	---	TRANSFORMED	HYETOGRAPH	---	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.08	0.500	89.37	0.833	27.02	1.17	2.08
0.333	22.86	0.667	56.12	1.000	8.31		

Max. Eff. Inten. (mm/hr) = 89.37 over (min) = 10.00  
 Storage Coeff. (mi n) = 5.14 (ii) 16.42 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit t Hyd. peak (cms) = 0.15 0.06

\*TOTALS\*  
 PEAK FLOW (cms) = 0.95 0.34 1.035 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.50 0.50  
 RUNOFF VOLUME (mm) = 33.64 7.91 15.37  
 TOTAL RAINFALL (mm) = 34.64 34.64 34.64  
 RUNOFF COEFFICIENT = 0.97 0.23 0.44

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PEROVIOUS LOSSES: CN\* = 71.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.64 mm Comments: 5yr 1hr AES

Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c44996e4

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25
0.17	4.16	0.50	116.39	0.83	20.78
0.25	12.47	0.58	62.35	0.92	12.47
0.33	33.25	0.67	49.88	1.00	4.16

CALIB  
 STANDHYD ( 0142)  
 ID= 1 DT=10.0 min

Area (ha) = 59.20 Page 9

VO output - Prop Uncontrolled Upstream.txt  
 Total Imp (%) = 67.00 Dir. Conn. (%) = 64.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	39.66	19.54
Dep. Storage (mm)	1.00	16.04
Average Slope (%)	1.00	3.00
Length (m)	628.23	40.00
Mannings n	0.015	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	---	TRANSFORMED	HYETOGRAPH	---	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.08	0.500	89.37	0.833	27.02	1.17	2.08
0.333	22.86	0.667	56.12	1.000	8.31		

Max. Eff. Inten. (mm/hr) = 89.37 over (min) = 10.00  
 Storage Coeff. (mi n) = 8.77 (ii) 23.87 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 30.00  
 Unit t Hyd. peak (cms) = 0.12 0.04

\*TOTALS\*  
 PEAK FLOW (cms) = 6.83 0.29 6.839 (iii)  
 TIME TO PEAK (hrs) = 0.50 1.00 0.50  
 RUNOFF VOLUME (mm) = 33.64 4.25 23.06  
 TOTAL RAINFALL (mm) = 34.64 34.64 34.64  
 RUNOFF COEFFICIENT = 0.97 0.12 0.67

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PEROVIOUS LOSSES: CN\* = 76.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.64 mm Comments: 5yr 1hr AES

Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c44996e4

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	62.35	0.75	33.25
0.17	4.16	0.50	116.39	0.83	20.78
0.25	12.47	0.58	62.35	0.92	12.47
0.33	33.25	0.67	49.88	1.00	4.16

CALIB  
 STANDHYD ( 0022)  
 ID= 1 DT=10.0 min

Area (ha) = 123.05 Dir. Conn. (%) = 49.00  
 Total Imp (%) = 61.00

	IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	75.06	47.99
Dep. Storage (mm)	2.00	5.00
Average Slope (%)	1.00	2.00
Length (m)	905.72	40.00
Mannings n	0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	---	TRANSFORMED	HYETOGRAPH	---	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.08	0.500	89.37	0.833	27.02	1.17	2.08

VO output - Prop Uncontrolled Upstream.txt  
 0.333 22.86 | 0.667 56.12 | 1.000 8.31

Max. Eff. Inten. (mm/hr) = 89.37 over (min) = 10.00  
 Storage Coeff. (mi n) = 10.02 (ii) 23.15 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 30.00  
 Unit t Hyd. peak (cms) = 0.11 0.04

\*TOTALS\*  
 PEAK FLOW (cms) = 9.95 1.65 10.297 (iii)  
 TIME TO PEAK (hrs) = 0.50 1.00 0.67  
 RUNOFF VOLUME (mm) = 32.64 8.04 20.10  
 TOTAL RAINFALL (mm) = 34.64 34.64 34.64  
 RUNOFF COEFFICIENT = 0.94 0.23 0.58

(i) CN PROCEDURE SELECTED FOR PEROVIOUS LOSSES: CN\* = 69.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017)  
 1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 1 ( 0142): 59.20	6.839	0.50	23.06
+ ID2 = 2 ( 0022): 123.05	10.297	0.67	20.10
ID = 3 ( 0017): 182.25	16.997	0.50	21.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 1 ( 1421): 15.00	1.035	0.50	15.37
+ ID2 = 2 ( 0151): 18.20	0.327	0.83	3.53
ID = 3 ( 0003): 33.20	1.227	0.67	8.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 3 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 3 ( 0003): 33.20	1.227	0.67	8.88
+ ID2 = 2 ( 0171): 182.25	16.997	0.50	21.06
ID = 1 ( 0003): 215.45	18.086	0.50	19.18

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 1 + 2 = 3

AREA	OPEAK	TPEAK	R.V.
(ha)	(cms)	(hrs)	(mm)
ID1 = 1 ( 0003): 215.45	18.086	0.50	19.18
+ ID2 = 2 ( 0002): 182.07	1.125	0.83	1.20
ID = 3 ( 0003): 397.52	18.224	0.67	10.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

VO output - Prop Uncontrolled Upstream.txt

V V I SSSS U U A L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U A A A A L  
 V V I SS U U A A L  
 V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y M M 0 0  
 0 0 T T H H Y Y M M 0 0  
 000 T T H H Y Y M M 000

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\W02\voi n.dat  
 Output filename:  
 C:\Users\p001279d\AppData\Local\CI\vi ca\VH5\b8ccee00-2cc7-4973-8ab8-dd0de50b3a28\05d6ed  
 76-6e21-4b6f-bf98-07f6bf6f7e7e\se  
 Summary filename:  
 C:\Users\p001279d\AppData\Local\CI\vi ca\VH5\b8ccee00-2cc7-4973-8ab8-dd0de50b3a28\05d6ed  
 76-6e21-4b6f-bf98-07f6bf6f7e7e\se

DATE: 11/28/2019 TIME: 11:04:50

USER:

COMMENTS:

\*\*\*\*\*  
 \* SIMULATION : 03 AES 10-Yr  
 \*\*\*\*\*

READ STORM  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61
0.17	4.95	0.50	138.63	0.83	24.76
0.25	14.85	0.58	74.27	0.92	14.85
0.33	39.61	0.67	59.41	1.00	4.95

CALIB  
 NASHYD ( 0153)  
 ID= 1 DT=10.0 min

Area (ha) = 91.47 Curve Number (CN) = 69.0  
 U.H. Tp (hrs) = 0.20 # of Linear Res. (N) = 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	---	TRANSFORMED	HYETOGRAPH	---	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.08	0.500	89.37	0.833	27.02	1.17	2.08

VO output - Prop Uncontrolled Upstream.txt							
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48
0.333	27.23	0.667	66.84	1.000	9.90		

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 1.212 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 2.499  
 TOTAL RAINFALL (mm) = 41.260  
 RUNOFF COEFFICIENT = 0.061

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900

Comments: 10yr 1hr AES

READ STORM		File name:		Comments:	
Ptotal = 41.26 mm		C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900		10yr 1hr AES	

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0  
 ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Linear Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

READ STORM		File name:		Comments:	
Ptotal = 41.26 mm		C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900		10yr 1hr AES	

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 1.391 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 2.883  
 TOTAL RAINFALL (mm) = 41.260  
 RUNOFF COEFFICIENT = 0.070

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)		AREA		OPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)	(mm)	(cms)	(mm)	(hrs)	(mm)	(mm)	(mm)
1D1 = 1 ( 0152):		90.60	1.391	0.83	2.88				
+ 1D2 = 2 ( 0153):		91.47	1.212	0.83	2.50				
-----									
ID = 3 ( 0002):		182.07	2.603	0.83	2.69				

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900

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Total = 41.26 mm

Comments: 10yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0  
 ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Linear Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

READ STORM		File name:		Comments:	
Ptotal = 41.26 mm		C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900		10yr 1hr AES	

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

Unit Hyd Opeak (cms) = 3.476

PEAK FLOW (cms) = 0.510 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 5.622  
 TOTAL RAINFALL (mm) = 41.260  
 RUNOFF COEFFICIENT = 0.136

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900

Comments: 10yr 1hr AES

READ STORM		File name:		Comments:	
Ptotal = 41.26 mm		C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900		10yr 1hr AES	

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB STANDHYD ( 1421) Area (ha) = 15.00  
 ID= 1 DT=10.0 min Total Imp(%) = 30.00 Dir. Conn. (%) = 29.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 4.50 10.50  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 258.20 40.00  
 Mannings n = 0.015 0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48
0.333	27.23	0.667	66.84	1.000	9.90		

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Max. Eff. Inten. (mm/hr) =	106.45	24.81
over (min) =	10.00	20.00
Storage Coeff. (mi n) =	4.80 (ii)	14.34 (ii)
Unit Hyd. Tpeak (mi n) =	10.00	20.00
Unit Hyd. peak (cms) =	0.15	0.07

\*TOTALS\*

PEAK FLOW (cms) =	1.16	0.50	1.285 (iii)
TIME TO PEAK (hrs) =	0.50	0.83	0.50
RUNOFF VOLUME (mm) =	40.26	10.90	19.42
TOTAL RAINFALL (mm) =	41.26	41.26	41.26
RUNOFF COEFFICIENT =	0.98	0.26	0.47

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900

Comments: 10yr 1hr AES

READ STORM		File name:		Comments:	
Ptotal = 41.26 mm		C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900		10yr 1hr AES	

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB STANDHYD ( 0142) Area (ha) = 59.20  
 ID= 1 DT=10.0 min Total Imp(%) = 67.00 Dir. Conn. (%) = 64.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 39.66 19.54  
 Dep. Storage (mm) = 1.00 16.04  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 628.23 40.00  
 Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48
0.333	27.23	0.667	66.84	1.000	9.90		

Max. Eff. Inten. (mm/hr) = 106.45 17.47  
 over (min) = 10.00 30.00  
 Storage Coeff. (mi n) = 8.18 (ii) 20.74 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 30.00  
 Unit Hyd. peak (cms) = 0.12 0.05

\*TOTALS\*  
 PEAK FLOW (cms) = 8.42 1.53 8.449 (iii)  
 TIME TO PEAK (hrs) = 0.50 1.00 0.50  
 RUNOFF VOLUME (mm) = 40.26 7.05 28.30  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.17 0.69

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 76.0 Ia = Dep. Storage (Above)
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- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900

Comments: 10yr 1hr AES

READ STORM		File name:		Comments:	
Ptotal = 41.26 mm		C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\c9d32900		10yr 1hr AES	

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB STANDHYD ( 0022) Area (ha) = 123.05  
 ID= 1 DT=10.0 min Total Imp(%) = 61.00 Dir. Conn. (%) = 49.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 75.06 47.99  
 Dep. Storage (mm) = 2.00 5.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 905.72 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48
0.333	27.23	0.667	66.84	1.000	9.90		

Max. Eff. Inten. (mm/hr) = 106.45 30.07  
 over (min) = 10.00 30.00  
 Storage Coeff. (mi n) = 9.35 (ii) 20.76 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 30.00  
 Unit Hyd. peak (cms) = 0.11 0.05

\*TOTALS\*  
 PEAK FLOW (cms) = 12.38 2.42 12.706 (iii)  
 TIME TO PEAK (hrs) = 0.50 1.00 0.50  
 RUNOFF VOLUME (mm) = 39.26 11.24 24.97  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.95 0.27 0.61

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017)		AREA		OPEAK		TPEAK		R.V.	
1 + 2 = 3		(ha)	(cms)	(hrs)	(mm)	(mm)	(mm)	(mm)	(mm)
1D1 = 1 ( 0142):		59.20	8.449	0.50	28.30				
+ 1D2 = 2 ( 0022):		123.05	12.706	0.50	24.97				
-----									
ID = 3 ( 0017):		182.25	21.155	0.50	26.05				

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.  
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Table with 5 columns: ADD HYD, AREA, OPEAK, TPEAK, R.V. Rows for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 5 columns: ADD HYD, AREA, OPEAK, TPEAK, R.V. Rows for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Table with 5 columns: ADD HYD, AREA, OPEAK, TPEAK, R.V. Rows for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

Input file name: C:\Program Files (x86)\Visual OTTHYM0 5.2\VO2\vo1n.dat
Output file name: C:\Users\p001279d\AppData\Local\CI\ci\ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\428f12...

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input file name: C:\Program Files (x86)\Visual OTTHYM0 5.2\VO2\vo1n.dat
Output file name: C:\Users\p001279d\AppData\Local\CI\ci\ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\428f12...

DATE: 11/28/2019 TIME: 11:04:52
USER:

COMMENTS:

\*\*\* SIMULATION : 04 AES 25-Yr \*\*\*

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf25yr 1hr AES
Total = 49.17 mm

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.08, 0.17, 0.25, 0.33.

CALIB NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0
ID= 1 DT=10.0 min Ia (mm) = 22.82 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.167, 0.333.

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 2.334 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 4.815
TOTAL RAINFALL (mm) = 49.170
RUNOFF COEFFICIENT = 0.098

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf25yr 1hr AES
Total = 49.17 mm

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.08, 0.25, 0.33.

CALIB NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.
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Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.167, 0.333.

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 2.564 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 5.367
TOTAL RAINFALL (mm) = 49.170
RUNOFF COEFFICIENT = 0.109

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

Table with 5 columns: ADD HYD, AREA, OPEAK, TPEAK, R.V. Rows for ID=1, ID=2, ID=3.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf25yr 1hr AES
Total = 49.17 mm

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.08, 0.17, 0.25, 0.33.

CALIB NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0
ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.167, 0.333.

Unit Hyd Opeak (cms) = 3.476

PEAK FLOW (cms) = 0.765 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 6.608
TOTAL RAINFALL (mm) = 49.170
RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf25yr 1hr AES
Total = 49.17 mm

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.08, 0.17, 0.25, 0.33.

CALIB STANDHYD ( 1421) Area Total (ha) = 15.00 Imp(%) = 30.00 Dir. Conn. (%) = 29.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 4.50 10.50
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 3.00
Length (m) = 258.20 40.00
Mannings n = 0.015 0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.167, 0.333.

Max. Eff. Inten. (mm/hr) over (min) = 126.86 33.60 10.00 20.00
Storage Coeff. (min) = 4.47 (ii) 12.93 (ii)
Unit Hyd. Tpeak (min) = 10.00 20.00
Unit Hyd. peak (cms) = 0.15 0.07

PEAK FLOW (cms) = 1.40 0.70 \*TOTALS\* 1.594 (iii)
TIME TO PEAK (hrs) = 0.50 0.83 0.83 0.50
RUNOFF VOLUME (mm) = 48.17 14.90 24.55
TOTAL RAINFALL (mm) = 49.17 49.17 49.17
RUNOFF COEFFICIENT = 0.98 0.30 0.50

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 71.0 Ia = Dep Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf25yr 1hr AES
Total = 49.17 mm

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows for 0.08, 0.17, 0.25, 0.33.

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CALIB  
STANDHYD ( 0142)  
ID= 1 DT=10.0 min

Area (ha)= 59.20  
Total Imp(%)= 67.00

Dir. Conn.(%)= 64.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 39.66 19.54  
Dep. Storage (mm)= 1.00 16.04  
Average Slope (%)= 1.00 3.00  
Length (m)= 628.23 40.00  
Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.95	0.500	126.86	1.17	2.95
0.333	32.45	0.667	79.66	1.000	11.80

Max. Eff. Inten. (mm/hr)= 126.86 31.71  
over (min)= 10.00 20.00  
Storage Coeff. (mi n)= 7.62 (ii) 17.52 (ii)  
Unit t Hyd. Tpeak (mi n)= 10.00 20.00  
Unit t Hyd. peak (cms)= 0.12 0.06

PEAK FLOW (cms)= 10.36 0.94 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.83 10.476 (iii)  
RUNOFF VOLUME (mm)= 48.17 11.00 34.79  
TOTAL RAINFALL (mm)= 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.22 0.71

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 76.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf

Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

CALIB  
STANDHYD ( 0022)  
ID= 1 DT=10.0 min

Area (ha)= 123.05  
Total Imp(%)= 61.00

Dir. Conn.(%)= 49.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha)= 75.06 47.99  
Dep. Storage (mm)= 2.00 5.00  
Average Slope (%)= 1.00 2.00  
Length (m)= 905.72 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20
0.17	5.90	0.50	165.21	0.83	29.50
0.25	17.70	0.58	88.51	0.92	17.70
0.33	47.20	0.67	70.80	1.00	5.90

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hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr

0.167 2.95 0.500 126.86 0.833 38.35 1.17 2.95

0.333 32.45 0.667 79.66 1.000 11.80

Max. Eff. Inten. (mm/hr)= 126.86 46.15  
over (min)= 10.00 20.00  
Storage Coeff. (mi n)= 8.71 (ii) 18.33 (ii)  
Unit t Hyd. Tpeak (mi n)= 10.00 20.00  
Unit t Hyd. peak (cms)= 0.12 0.06

PEAK FLOW (cms)= 15.35 3.77 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.83 16.814 (iii)  
RUNOFF VOLUME (mm)= 47.17 15.51 0.67  
TOTAL RAINFALL (mm)= 49.17 49.17 31.02  
RUNOFF COEFFICIENT = 0.96 0.32 0.63

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017)  
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 ( 0142):	59.20	10.476	0.50	34.79
+ ID2 = 2 ( 0022):	123.05	16.814	0.67	31.02
ID = 3 ( 0017):	182.25	26.724	0.50	32.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 ( 1421):	15.00	1.594	0.50	24.55
+ ID2 = 2 ( 0151):	18.20	5.765	0.83	8.61
ID = 3 ( 0003):	33.20	2.250	0.67	15.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
3 + 2 = 1

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 0 ( 0003):	33.20	2.250	0.67	15.81
+ ID2 = 2 ( 0017):	182.25	26.724	0.50	32.25
ID = 1 ( 0003):	215.45	28.543	0.50	29.71

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 ( 0003):	215.45	28.543	0.50	29.71
+ ID2 = 2 ( 0002):	182.07	4.898	0.83	5.09
ID = 3 ( 0003):	397.52	31.784	0.67	18.43

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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VO output - Prop Uncontrolled Upstream.txt

V V I SSSSS U U A L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
V V I SSSSS UUUU A A LLLLL

000 TTTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input file name: C:\Program Files (x86)\Visual OTTHYM0 5.2\VO2\voin.dat

Output file name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf

Summary file name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\54806dbf

DATE: 11/28/2019 TIME: 11:04:52

USER:

COMMENTS:

\*\* SIMULATION : 05 AES 50-Yr \*\*

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\2c2fb217d

Ptotal = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75
0.17	6.59	0.50	184.63	0.83	32.97
0.25	19.78	0.58	98.91	0.92	19.78
0.33	52.75	0.67	79.13	1.00	6.59

CALIB  
NASHYD ( 0153)  
ID= 1 DT=10.0 min

Area (ha)= 91.47  
Ia (mm)= 22.82

Curve Number (CN)= 69.0  
# of Li near Res. (N)= 3.00

U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86
0.333	36.27	0.667	89.02	1.000	13.19

Page 23

VO output - Prop Uncontrolled Upstream.txt

0.333 36.27 0.667 89.02 1.000 13.19

Unit Hyd Opeak (cms)= 17.469

PEAK FLOW (cms)= 3.288 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 6.875  
TOTAL RAINFALL (mm)= 54.950  
RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\2c2fb217d

Ptotal = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75
0.17	6.59	0.50	184.63	0.83	32.97
0.25	19.78	0.58	98.91	0.92	19.78
0.33	52.75	0.67	79.13	1.00	6.59

CALIB  
NASHYD ( 0152)  
ID= 1 DT=10.0 min

Area (ha)= 90.60  
Ia (mm)= 21.77

Curve Number (CN)= 70.0  
# of Li near Res. (N)= 3.00

U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---					
TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86
0.333	36.27	0.667	89.02	1.000	13.19

Unit Hyd Opeak (cms)= 17.302

PEAK FLOW (cms)= 3.552 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 7.549  
TOTAL RAINFALL (mm)= 54.950  
RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1 = 1 ( 0152):	90.60	3.552	0.83	7.55
+ ID2 = 2 ( 0153):	91.47	3.288	0.83	6.88
ID = 3 ( 0002):	182.07	6.841	0.83	7.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\2c2fb217d

Ptotal = 54.95 mm Comments: 50yr 1hr AES

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VO output - Prop Uncontrolled Upstream.txt									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59		
0.17	6.59	0.50	184.63	0.83	32.97				
0.25	19.78	0.58	98.91	0.92	19.78				
0.33	52.75	0.67	79.13	1.00	6.59				

CALIB  
STANDHYD ( 0151)  
ID= 1 DT=10.0 min

Area (ha) = 18.20  
Imp (%) = 11.95  
U.H. Tp (hrs) = 0.20

Curve Number (CN) = 68.0  
# of Li near Res. (N) = 3.00

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30		
0.333	36.27	0.667	89.02	1.000	13.19				

Unit Hyd Opeak (cms) = 3.476  
PEAK FLOW (cms) = 0.972 (i)  
TIME TO PEAK (hrs) = 0.833  
RUNOFF VOLUME (mm) = 11.081  
TOTAL RAINFALL (mm) = 54.950  
RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\2c9b217d  
Ptotal = 54.95 mm  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 1421)  
ID= 1 DT=10.0 min

Area (ha) = 15.00  
Total Imp (%) = 30.00  
Dir. Conn. (%) = 29.00

Surface Area (ha) = 4.50  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 258.20  
Mannings n = 0.015

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30		
0.333	36.27	0.667	89.02	1.000	13.19				

Max. Eff. Inten. (mm/hr) over (min) = 141.77 / 10.00  
Page 25

VO output - Prop Uncontrolled Upstream.txt									
Storage Coeff.	Unit Hyd. Tpeak	Unit Hyd. peak	PEAK FLOW	TIME TO PEAK	RUNOFF VOLUME	TOTAL RAINFALL	RUNOFF COEFFICIENT	Area	Imp
(mi n)	(mi n)	(cms)	(cms)	(hrs)	(mm)	(mm)		(ha)	(%)
4.28	10.00	0.15	1.58	0.50	53.95	54.95	0.98	18.20	11.95
12.12	20.00	0.07	0.87	0.83	18.08	54.95	0.33		
			1.851	0.67	28.48				
			0.83	0.50	54.95				
			0.67	0.50	54.95				
			0.83	0.50	54.95				
			0.83	0.50	54.95				
			0.83	0.50	54.95				

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN= 71.0 Ua = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\2c9b217d  
Ptotal = 54.95 mm  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0142)  
ID= 1 DT=10.0 min

Area (ha) = 59.20  
Total Imp (%) = 67.00  
Dir. Conn. (%) = 64.00

Surface Area (ha) = 39.66  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 628.23  
Mannings n = 0.015

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30		
0.333	36.27	0.667	89.02	1.000	13.19				

Max. Eff. Inten. (mm/hr) over (min) = 141.77 / 10.00  
Storage Coeff. (mi n) = 7.29 (ii)  
Unit Hyd. Tpeak (mi n) = 10.00  
Unit Hyd. peak (cms) = 0.13  
PEAK FLOW (cms) = 11.80  
TIME TO PEAK (hrs) = 0.50  
RUNOFF VOLUME (mm) = 53.95  
TOTAL RAINFALL (mm) = 54.95  
RUNOFF COEFFICIENT = 0.98

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN= 76.0 Ua = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.

VO output - Prop Uncontrolled Upstream.txt  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\2c9b217d  
Ptotal = 54.95 mm  
Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB  
STANDHYD ( 0022)  
ID= 1 DT=10.0 min

Area (ha) = 123.05  
Total Imp (%) = 61.00  
Dir. Conn. (%) = 49.00

Surface Area (ha) = 75.06  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 905.72  
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH									
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30		
0.333	36.27	0.667	89.02	1.000	13.19				

Max. Eff. Inten. (mm/hr) over (min) = 141.77 / 10.00  
Storage Coeff. (mi n) = 8.33 (ii)  
Unit Hyd. Tpeak (mi n) = 10.00  
Unit Hyd. peak (cms) = 0.12  
PEAK FLOW (cms) = 17.56  
TIME TO PEAK (hrs) = 0.50  
RUNOFF VOLUME (mm) = 52.95  
TOTAL RAINFALL (mm) = 54.95  
RUNOFF COEFFICIENT = 0.96

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN= 69.0 Ua = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0142):	59.20	11.983	0.50	39.65
+ ID2= 2 ( 0022):	123.05	19.346	0.67	35.58
ID = 3 ( 0017):	182.25	30.718	0.50	36.90

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

VO output - Prop Uncontrolled Upstream.txt

ADD HYD ( 0003)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 1421):	15.00	1.851	0.67	28.48
+ ID2= 2 ( 0151):	18.20	0.972	0.83	11.08
ID = 3 ( 0003):	33.20	2.725	0.67	18.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)				
3 + 2 = 1	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0003):	33.20	2.725	0.67	18.94
+ ID2= 2 ( 0017):	182.25	30.718	0.50	36.90
ID = 1 ( 0003):	215.45	32.868	0.50	34.13

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)				
1 + 2 = 3	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0003):	215.45	32.868	0.50	34.13
+ ID2= 2 ( 0002):	182.07	6.841	0.83	7.21
ID = 3 ( 0003):	397.52	37.781	0.67	21.80

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A A L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M O O O TM  
0 0 T T H H Y Y M M O O 0  
0 0 T T H H Y Y M M O O 0  
0 0 T T H H Y Y M M O O 0  
0 0 T T H H Y Y M M O O 0  
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2voin.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Temp\ca\WH5b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\162043  
9b-ba95-45ff-adfe-c79412ef22c6\scse  
Summary filename:  
C:\Users\p001279d\AppData\Local\Temp\ca\WH5b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\162043  
9b-ba95-45ff-adfe-c79412ef22c6\scse

DATE: 11/28/2019

TIME: 11:04:51

USER:

COMMENTS:

\*\* SIMULATION : 06 AES 100-Yr \*\*

READ STORM
Filename: C:\Users\p001279d\AppData
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data at 0.08, 0.17, 0.25, and 0.33 hours.

CALIB
NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0
ID= 1 DT=10.0 min Ia (mm) = 22.82 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall data.

Unit Hyd Opeak (cms) = 17.469
PEAK FLOW (cms) = 4.367 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 9.267
TOTAL RAINFALL (mm) = 60.870
RUNOFF COEFFICIENT = 0.152

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM
Filename: C:\Users\p001279d\AppData
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

CALIB
NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

VO output - Prop Uncontrolled Upstream.txt
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

CALIB
STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00
ID= 1 DT=10.0 min Total Imp(%) = 30.00

Surface Area (ha) = 4.50 IMPERVIOUS (%) = 100.00
Dep. Storage (mm) = 1.00 PERVIOUS (%) = 0.00
Average Slope (%) = 1.00
Length (m) = 258.20
Mannings n = 0.015

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall data.

Max. Eff. Inten. (mm/hr) = 157.04 over (min) = 10.00
Storage Coeff. (mi n) = 4.11 (ii) 11.44 (iii)
Unit Hyd. Tpeak (mi n) = 10.00
Unit Hyd. peak (cms) = 0.16

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM
Filename: C:\Users\p001279d\AppData
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

CALIB
STANDHYD ( 0142) Area (ha) = 59.20
ID= 1 DT=10.0 min Page 31

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 4.664 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 10.064
TOTAL RAINFALL (mm) = 60.870
RUNOFF COEFFICIENT = 0.165

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)
1 + 2 = 3

Table with 5 columns: AREA (ha), OPEAK (cms), TPEAK (hrs), R.V. (mm). Rows show peak flow data for different scenarios.

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM
Filename: C:\Users\p001279d\AppData
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

CALIB
NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0
ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall data.

Unit Hyd Opeak (cms) = 3.476
PEAK FLOW (cms) = 1.201 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 13.838
TOTAL RAINFALL (mm) = 60.870
RUNOFF COEFFICIENT = 0.227

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM
Filename: C:\Users\p001279d\AppData
Page 30

VO output - Prop Uncontrolled Upstream.txt
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

CALIB
STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00
ID= 1 DT=10.0 min Total Imp(%) = 30.00

Surface Area (ha) = 4.50 IMPERVIOUS (%) = 100.00
Dep. Storage (mm) = 1.00 PERVIOUS (%) = 0.00
Average Slope (%) = 1.00
Length (m) = 258.20
Mannings n = 0.015

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall data.

Max. Eff. Inten. (mm/hr) = 157.04 over (min) = 10.00
Storage Coeff. (mi n) = 4.11 (ii) 11.44 (iii)
Unit Hyd. Tpeak (mi n) = 10.00
Unit Hyd. peak (cms) = 0.16

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM
Filename: C:\Users\p001279d\AppData
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

CALIB
STANDHYD ( 0142) Area (ha) = 59.20
ID= 1 DT=10.0 min Page 31

VO output - Prop Uncontrolled Upstream.txt
Total Imp(%) = 67.00 Dir. Conn. (%) = 64.00
ID= 1 DT=10.0 min

IMPERVIOUS (%) = 100.00
PERVIOUS (%) = 0.00
Surface Area (ha) = 39.66
Dep. Storage (mm) = 1.00
Average Slope (%) = 1.00
Length (m) = 628.23
Mannings n = 0.015

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall data.

Max. Eff. Inten. (mm/hr) = 157.04 over (min) = 10.00
Storage Coeff. (mi n) = 7.00 (ii) 15.27 (iii)
Unit Hyd. Tpeak (mi n) = 10.00
Unit Hyd. peak (cms) = 0.13

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 76.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM
Filename: C:\Users\p001279d\AppData
ata\Local\Temp\
c0a758ec-b7cc-4a17-823b-afaf544e7a0a\8e50d755
Ptotal = 60.87 mm
Comments: 100yr 1hr AES

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data.

CALIB
STANDHYD ( 0022) Area (ha) = 123.05 Dir. Conn. (%) = 49.00
ID= 1 DT=10.0 min Total Imp(%) = 61.00

IMPERVIOUS (%) = 100.00
PERVIOUS (%) = 0.00
Surface Area (ha) = 75.06
Dep. Storage (mm) = 2.00
Average Slope (%) = 1.00
Length (m) = 905.72
Mannings n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show transformed rainfall data.

Max. Eff. Inten. (mm/hr)= 157.04 65.97  
over (min)= 10.00 20.00  
Storage Coeff. (min)= 8.00 (ii) 16.34 (ii)  
Unit Hyd. Tpeak (min)= 10.00 20.00  
Unit Hyd. peak (cms)= 0.12 0.06  
\*TOTALS\*  
PEAK FLOW (cms)= 19.86 5.77 22.018 (iii)  
TIME TO PEAK (hrs)= 0.50 0.83 0.67  
RUNOFF VOLUME (mm)= 58.87 22.55 40.35  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.97 0.37 0.66

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PREVIOUS LOSSES:  
CN = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0017)  
1 + 2 = 3  
ID1= 1 (0142): 59.20 13.556 0.50 44.73  
+ ID2= 2 (0022): 123.05 22.018 0.67 40.35  
ID = 3 (0017): 182.25 34.903 0.50 41.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0003)  
1 + 2 = 3  
ID1= 1 (1421): 15.00 2.140 0.67 32.64  
+ ID2= 2 (0151): 18.20 1.201 0.83 13.84  
ID = 3 (0003): 33.20 3.247 0.67 22.33

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0003)  
3 + 2 = 3  
ID1= 3 (0003): 33.20 3.247 0.67 22.33  
+ ID2= 2 (0017): 182.25 34.903 0.50 41.77  
ID = 1 (0003): 215.45 37.414 0.50 38.77

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD (0003)  
1 + 2 = 3  
ID1= 1 (0003): 215.45 37.414 0.50 38.77  
+ ID2= 2 (0002): 182.07 9.030 0.83 9.66  
ID = 3 (0003): 397.52 44.338 0.67 25.44

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A A L  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
W V I SSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 0 0

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voinput.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Ci vi ca\VHS\b8c9cece0-2cc7-4973-8ab8-dd0de50b3a28\1ba0d7de-f2a0-4bc7-a25d-ede68bf8f4f5cse  
Summary filename:  
C:\Users\p001279d\AppData\Local\Ci vi ca\VHS\b8c9cece0-2cc7-4973-8ab8-dd0de50b3a28\1ba0d7de-f2a0-4bc7-a25d-ede68bf8f4f5cse

DATE: 11/28/2019

TIME: 11:04:51

USER:

COMMENTS:

\*\* SIMULATION : 07 AES 250-Yr \*\*

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\3a4314b1  
Total = 70.05 mm  
Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

CALIB  
NASHYD (0153) Area (ha)= 91.47 Curve Number (CN)= 69.0  
Ia (mm)= 22.82 # of Linear Res. (N)= 3.00  
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	4.20	0.500	180.73	0.833	54.64	1.17	4.20
0.333	46.23	0.667	113.48	1.000	16.81		

PEAK FLOW (cms)= 6.211 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 13.466  
TOTAL RAINFALL (mm)= 70.050  
RUNOFF COEFFICIENT = 0.192

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\3a4314b1  
Total = 70.05 mm  
Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

CALIB  
NASHYD (0152) Area (ha)= 90.60 Curve Number (CN)= 70.0  
Ia (mm)= 21.77 # of Linear Res. (N)= 3.00  
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	4.20	0.500	180.73	0.833	54.64	1.17	4.20
0.333	46.23	0.667	113.48	1.000	16.81		

Unit Hyd Opeak (cms)= 17.302

PEAK FLOW (cms)= 6.554 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 14.448  
TOTAL RAINFALL (mm)= 70.050  
RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD (0002)  
1 + 2 = 3  
ID1= 1 (0152): 90.60 6.554 0.83 14.45  
+ ID2= 2 (0153): 91.47 6.211 0.83 13.47  
ID = 3 (0002): 182.07 12.764 0.83 13.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\3a4314b1  
Total = 70.05 mm  
Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	4.20	0.500	180.73	0.833	54.64	1.17	4.20
0.333	46.23	0.667	113.48	1.000	16.81		

0.08 0.00 0.42 126.09 0.75 67.25 1.08 8.41  
0.17 8.41 0.50 235.37 0.83 42.03  
0.25 25.22 0.58 126.09 0.92 25.22  
0.33 67.25 0.67 100.87 1.00 8.41

CALIB  
NASHYD (0151) Area (ha)= 18.20 Curve Number (CN)= 68.0  
Ia (mm)= 11.95 # of Linear Res. (N)= 3.00  
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	4.20	0.500	180.73	0.833	54.64	1.17	4.20
0.333	46.23	0.667	113.48	1.000	16.81		

Unit Hyd Opeak (cms)= 3.476

PEAK FLOW (cms)= 1.585 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 18.510  
TOTAL RAINFALL (mm)= 70.050  
RUNOFF COEFFICIENT = 0.264

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\3a4314b1  
Total = 70.05 mm  
Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

CALIB  
STANDHYD (1421) Area (ha)= 15.00 Di r. Conn. (%)= 29.00  
Total Imp(%)= 30.00

Surface Area	(ha)	IMPERVIOUS	PERVIOUS (i)
Dep. Storage	(mm)	1.00	2.00
Average Slope	(%)	1.00	3.00
Length	(m)	258.20	40.00
Mannings n		0.015	0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	4.20	0.500	180.73	0.833	54.64	1.17	4.20
0.333	46.23	0.667	113.48	1.000	16.81		

Max. Eff. Inten. (mm/hr)= 180.73 60.12  
over (min)= 10.00 20.00  
Storage Coeff. (min)= 3.88 (ii) 10.58 (ii)  
Unit Hyd. Tpeak (min)= 10.00 20.00

VO output - Prop Uncontrolled Upstream.txt  
Unit Hyd. peak (cms) = 0.16 0.08  
\*TOTALS\*  
PEAK FLOW (cms) = 2.06 1.35 2.614 (iii)  
TIME TO PEAK (hrs) = 0.50 0.83 0.67  
RUNOFF VOLUME (mm) = 69.05 27.20 39.34  
TOTAL RAINFALL (mm) = 70.05 70.05 70.05  
RUNOFF COEFFICIENT = 0.99 0.39 0.56

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\3a4314b1  
Ptotal = 70.05 mm Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

CALIB STANDHYD ( 0142) Area (ha) = 59.20 Dir. Conn. (%) = 64.00  
ID= 1 DT=10.0 min Total Imp(%) = 67.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 39.66 19.54  
Dep. Storage (mm) = 1.00 16.04  
Average Slope (%) = 1.00 3.00  
Length (m) = 628.23 40.00  
Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	4.20	0.500	180.73	0.833	54.64	1.17	4.20
0.333	46.23	0.667	113.48	1.000	16.81		

Max. Eff. Inten. (mm/hr) = 180.73 64.67  
over (min) = 10.00 20.00  
Storage Coeff. (min) = 6.62 (ii) 14.06 (iii)  
Unit Hyd. Tpeak (min) = 10.00 20.00  
Unit Hyd. peak (cms) = 0.13 0.07

\*TOTALS\*  
PEAK FLOW (cms) = 15.63 2.25 16.048 (iii)  
TIME TO PEAK (hrs) = 0.50 0.83 0.50  
RUNOFF VOLUME (mm) = 69.05 23.77 52.75  
TOTAL RAINFALL (mm) = 70.05 70.05 70.05  
RUNOFF COEFFICIENT = 0.99 0.34 0.75

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 76.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Uncontrolled Upstream.txt

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\c0a758ec-b7cc-4a17-823b-afaf544e7a0a\3a4314b1  
Ptotal = 70.05 mm Comments: 250yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41
0.17	8.41	0.50	235.37	0.83	42.03		
0.25	25.22	0.58	126.09	0.92	25.22		
0.33	67.25	0.67	100.87	1.00	8.41		

CALIB STANDHYD ( 0022) Area (ha) = 123.05 Dir. Conn. (%) = 49.00  
ID= 1 DT=10.0 min Total Imp(%) = 61.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 75.06 47.99  
Dep. Storage (mm) = 2.00 5.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 905.72 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	4.20	0.500	180.73	0.833	54.64	1.17	4.20
0.333	46.23	0.667	113.48	1.000	16.81		

Max. Eff. Inten. (mm/hr) = 180.73 82.63  
over (min) = 10.00 20.00  
Storage Coeff. (min) = 7.56 (ii) 15.18 (ii)  
Unit Hyd. Tpeak (min) = 10.00 20.00  
Unit Hyd. peak (cms) = 0.12 0.07

\*TOTALS\*  
PEAK FLOW (cms) = 23.46 7.52 26.304 (iii)  
TIME TO PEAK (hrs) = 0.50 0.83 0.67  
RUNOFF VOLUME (mm) = 68.05 28.57 47.92  
TOTAL RAINFALL (mm) = 70.05 70.05 70.05  
RUNOFF COEFFICIENT = 0.97 0.41 0.68

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
1 + 2 = 3				
ID1= 1 ( 0142):	59.20	16.048	0.50	52.75
+ ID2= 2 ( 0022):	123.05	26.304	0.67	47.92
ID = 3 ( 0017):	182.25	41.561	0.50	49.49

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)

VO output - Prop Uncontrolled Upstream.txt

1 + 2 = 3  
ID1= 1 ( 1421): 15.00 2.614 0.67 39.34  
+ ID2= 2 ( 0151): 18.20 1.585 0.83 18.51  
ID = 3 ( 0003): 33.20 4.121 0.67 27.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3  
ID1= 3 ( 0003): 33.20 4.121 0.67 27.92  
+ ID2= 2 ( 0017): 182.25 41.561 0.50 49.49  
ID = 1 ( 0003): 215.45 44.734 0.67 46.16

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3  
ID1= 1 ( 0003): 215.45 44.734 0.67 46.16  
+ ID2= 2 ( 0002): 182.07 12.764 0.83 13.95  
ID = 3 ( 0003): 397.52 55.247 0.67 31.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 5.2.2003)  
V V I SS U U AAAA L  
V V I SS U U A A L  
V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vojn.dat

Output filename:  
C:\Users\p001279d\AppData\Local\CVI\vi ca\VHS\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\6ce217  
2f-4475-46e2-8500-ef29a9862c79\sce  
Summary filename:  
C:\Users\p001279d\AppData\Local\CVI\vi ca\VHS\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\6ce217  
2f-4475-46e2-8500-ef29a9862c79\sce

DATE: 11/28/2019

TIME: 11:04:49

USER:

COMMENTS:

VO output - Prop Uncontrolled Upstream.txt

\*\* SIMULATION : New DesignStormChicago - 3hr \*\*

CHI CAGO STORM IDF curve parameters: A=1499.500  
Ptotal = 71.76 mm B= 3.298  
C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.67	15.26	1.50	17.22	2.33	7.33	3.00	5.22
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0  
ID= 1 DT=10.0 min Ia (mm) = 22.82 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 3.540 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 14.306  
TOTAL RAINFALL (mm) = 71.757  
RUNOFF COEFFICIENT = 0.199

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1499.500  
Ptotal = 71.76 mm B= 3.298  
C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0  
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 3.855 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 15.322  
TOTAL RAINFALL (mm) = 71.757

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Uncontrolled Upstream.txt  
RUNOFF COEFFICIENT = 0.214

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002) 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0152):	90.60	3.855	1.17	15.32
+ ID2= 2 ( 0153):	91.47	3.540	1.17	14.31
-----				
ID = 3 ( 0002):	182.07	7.395	1.17	14.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm

IDF curve parameters: A=1499.500  
B= 3.298  
C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB NASHYD ( 0151) ID= 1 DT=10.0 min

Area (ha) = 18.20  
Curve Number (CN) = 68.0  
Ia (mm) = 11.95  
# of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd. Peak (cms) = 3.476

PEAK FLOW (cms) = 1.141 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 19.427  
TOTAL RAINFALL (mm) = 71.757  
RUNOFF COEFFICIENT = 0.271

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm

IDF curve parameters: A=1499.500  
B= 3.298  
C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

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VO output - Prop Uncontrolled Upstream.txt

CALIB STANDHYD ( 1421) ID= 1 DT=10.0 min

Area (ha) = 15.00  
Total Imp(%) = 30.00  
Dir. Conn. (%) = 29.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 4.50  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 258.20  
Mannings n = 0.015

Max. Eff. Inten. (mm/hr) = 192.16  
over (min) = 10.00  
Storage Coeff. (mi n) = 3.79 (ii)  
Unit Hyd. Tpeak (mi n) = 10.00  
Unit Hyd. peak (cms) = 0.16

PEAK FLOW (cms) = 2.19  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 70.76  
TOTAL RAINFALL (mm) = 71.76  
RUNOFF COEFFICIENT = 0.99

PERVIOUS (ii)  
10.08  
20.00  
20.00  
0.08

\*TOTALS\*  
2.752 (iii)  
40.61  
71.76  
0.57

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 71.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm

IDF curve parameters: A=1499.500  
B= 3.298  
C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB STANDHYD ( 0142) ID= 1 DT=10.0 min

Area (ha) = 59.20  
Total Imp(%) = 67.00  
Dir. Conn. (%) = 64.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 39.66  
Dep. Storage (mm) = 1.00  
Average Slope (%) = 1.00  
Length (m) = 628.23  
Mannings n = 0.015

Max. Eff. Inten. (mm/hr) = 192.16  
over (min) = 10.00  
Storage Coeff. (mi n) = 6.46 (ii)  
Unit Hyd. Tpeak (mi n) = 0.00  
Unit Hyd. peak (cms) = 0.13

PEAK FLOW (cms) = 16.63  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 70.76

PERVIOUS (ii)  
14.30 (ii)  
20.00  
0.07

\*TOTALS\*  
17.251 (iii)  
1.00  
54.26

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VO output - Prop Uncontrolled Upstream.txt  
TOTAL RAINFALL (mm) = 71.76  
RUNOFF COEFFICIENT = 0.99

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 76.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm

IDF curve parameters: A=1499.500  
B= 3.298  
C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.26	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

CALIB STANDHYD ( 0022) ID= 1 DT=10.0 min

Area (ha) = 123.05  
Total Imp(%) = 61.00  
Dir. Conn. (%) = 49.00

IMPERVIOUS PERVIOUS (i)

Surface Area (ha) = 75.06  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 905.72  
Mannings n = 0.013

Max. Eff. Inten. (mm/hr) = 192.16  
over (min) = 10.00  
Storage Coeff. (mi n) = 7.38 (ii)  
Unit Hyd. Tpeak (mi n) = 10.00  
Unit Hyd. peak (cms) = 0.13

PEAK FLOW (cms) = 25.19  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 69.76  
TOTAL RAINFALL (mm) = 71.76  
RUNOFF COEFFICIENT = 0.97

PERVIOUS (ii)  
14.56 (ii)  
20.00  
0.07

\*TOTALS\*  
28.054 (iii)  
1.00  
49.35  
71.76  
0.69

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 69.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017) 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0142):	59.20	17.251	1.00	54.26
+ ID2= 2 ( 0022):	123.05	28.054	1.00	49.35
-----				
ID = 3 ( 0017):	182.25	45.305	1.00	50.94

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VO output - Prop Uncontrolled Upstream.txt

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 1421):	15.00	2.752	1.00	40.61
+ ID2= 2 ( 0151):	18.20	1.141	1.17	19.43
-----				
ID = 3 ( 0003):	33.20	3.684	1.00	29.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) 3 + 2 = 1

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0003):	33.20	3.684	1.00	29.00
+ ID2= 2 ( 0017):	182.25	45.305	1.00	50.94
-----				
ID = 1 ( 0003):	215.45	48.988	1.00	47.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) 1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0003):	215.45	48.988	1.00	47.56
+ ID2= 2 ( 0002):	182.07	7.395	1.17	14.81
-----				
ID = 3 ( 0003):	397.52	53.953	1.00	32.56

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 5. 2. 003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L L L L L  
VV I SSSS UUUU A A L L L L L

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0 0  
0 0 T T H H Y Y M M 0 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual THYMO 5.2\VO2\voi.n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Civica\WH5\bcccece0-2cc7-4973-8ab8-dd0de50b3a28\581c2b  
c8-2656-473b-8f53-f58ea57cce01\sce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civica\WH5\bcccece0-2cc7-4973-8ab8-dd0de50b3a28\581c2b  
c8-2656-473b-8f53-f58ea57cce01\sce

DATE: 11/28/2019

TIME: 11:04:49

USER:

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COMMENTS:

\*\* SIMULATION : New DesigStormChicago - 3hr \*\*

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

CALIB NASHYD ( 0153)
Area (ha)= 91.47 Curve Number (CN)= 69.0
Ia (mm)= 22.82 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.20

Unit Hyd Opeak (cms)= 17.469
PEAK FLOW (cms)= 4.517 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 17.674
TOTAL RAINFALL (mm)= 78.293
RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

CALIB NASHYD ( 0152)
Area (ha)= 90.60 Curve Number (CN)= 70.0
Ia (mm)= 21.77 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.20

VO output - Prop Uncontrolled Upstream.txt
Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

CALIB STANDHYD ( 1421)
Area (ha)= 15.00
Total Imp(%)= 30.00 Dir. Conn.(%)= 29.00

Surface Area (ha)= 10.50
Dep. Storage (mm)= 2.00
Average Slope (%)= 3.00
Length (m)= 40.00
Mannings n = 0.200

Max. Eff. Inten. (mm/hr)= 214.13
over (min)= 10.00
Storage Coeff. (mi n)= 3.63 (ii)
Unit Hyd. Tpeak (mi n)= 10.00
Unit Hyd. peak (cms)= 0.16

PEAK FLOW (cms)= 2.45 1.63 4.079 (iii)
TIME TO PEAK (hrs)= 1.00 1.00 1.00
RUNOFF VOLUME (mm)= 77.29 32.61 45.57
TOTAL RAINFALL (mm)= 78.29 78.29 78.29
RUNOFF COEFFICIENT = 0.99 0.42 0.58

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN= 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

CALIB STANDHYD ( 0142)
Area (ha)= 59.20
Total Imp(%)= 67.00 Dir. Conn.(%)= 64.00

Surface Area (ha)= 39.66
Dep. Storage (mm)= 16.04
Average Slope (%)= 1.00
Length (m)= 628.23
Mannings n = 0.015

Max. Eff. Inten. (mm/hr)= 214.13
over (mi n)= 10.00
Storage Coeff. (mi n)= 6.18 (ii) 13.31 (ii)

Unit Hyd Opeak (cms)= 17.302
PEAK FLOW (cms)= 4.872 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 18.816
TOTAL RAINFALL (mm)= 78.293
RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0152): 90.60 4.872 1.17 18.82
+ ID2= 2 ( 0153): 91.47 4.517 1.17 17.67
ID = 3 ( 0002): 182.07 9.388 1.17 18.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

CALIB NASHYD ( 0151)
Area (ha)= 18.20 Curve Number (CN)= 68.0
Ia (mm)= 11.95 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.20

Unit Hyd Opeak (cms)= 3.476
PEAK FLOW (cms)= 1.364 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 23.064
TOTAL RAINFALL (mm)= 78.293
RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

VO output - Prop Uncontrolled Upstream.txt
Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN= 76.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM
Ptotal = 78.29 mm
IDF curve parameters: A=1498.100
B= 2.188
C= 0.778
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rain intensity at various time intervals.

CALIB STANDHYD ( 0022)
Area (ha)= 123.05
Total Imp(%)= 61.00 Dir. Conn.(%)= 49.00

Surface Area (ha)= 75.06
Dep. Storage (mm)= 5.00
Average Slope (%)= 1.00
Length (m)= 905.72
Mannings n = 0.013

Max. Eff. Inten. (mm/hr)= 214.13
over (mi n)= 10.00
Storage Coeff. (mi n)= 7.07 (ii) 13.76 (ii)
Unit Hyd. Tpeak (mi n)= 10.00
Unit Hyd. peak (cms)= 0.13

PEAK FLOW (cms)= 28.45 7.46 31.955 (iii)
TIME TO PEAK (hrs)= 1.00 1.17 1.00
RUNOFF VOLUME (mm)= 76.29 34.29 54.87
TOTAL RAINFALL (mm)= 78.29 78.29 78.29
RUNOFF COEFFICIENT = 0.97 0.44 0.70

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN= 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Uncontrolled Upstream.txt  
1 + 2 = 3  
AREA OPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID1= 1 ( 0142): 59.20 19.569 1.00 60.08  
+ ID2= 2 ( 0022): 123.05 31.955 1.00 54.87  
ID = 3 ( 0017): 182.25 51.524 1.00 56.56  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3  
AREA OPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID1= 1 ( 1421): 15.00 4.079 1.00 45.57  
+ ID2= 2 ( 0151): 18.20 1.364 1.17 23.06  
ID = 3 ( 0003): 33.20 5.252 1.00 33.23  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
3 + 2 = 1  
AREA OPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID1= 3 ( 0003): 33.20 5.252 1.00 33.23  
+ ID2= 2 ( 0017): 182.25 51.524 1.00 56.56  
ID = 1 ( 0003): 215.45 56.776 1.00 52.97  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 1  
AREA OPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID1= 1 ( 0003): 215.45 56.776 1.00 52.97  
+ ID2= 2 ( 0002): 182.07 9.388 1.17 18.24  
ID = 3 ( 0003): 397.52 63.719 1.00 37.06  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5. 2. 2003)  
V V I SS U U A A L  
V V I SS U U A A A L  
V V I SS U U A A L  
V V I SSSSS UUUUU A A LLLLL  
000 TTTT TTTT H H Y Y M M M 000 TM  
0 0 T T H H Y Y M M M 0 0  
0 0 T T H H Y Y M M M 0 0  
000 T H Y Y M M 000  
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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo.in.dat  
Output filename:  
C:\Users\p001279\AppData\Local\Civi.ca\WH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\b9f66a0-50db-421a-8092-91bd04dad910\scse  
Summary filename:  
C:\Users\p001279\AppData\Local\Civi.ca\WH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\b9f66a0-50db-421a-8092-91bd04dad910\scse  
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VO output - Prop Uncontrolled Upstream.txt  
DATE: 11/28/2019 TIME: 11:04:50  
USER:  
COMMENTS:

CHICAGO STORM  
Ptotal = 60.39 mm  
IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
NASHYD ( 0153)  
ID= 1 DT=10.0 min  
Area (ha) = 91.47 Curve Number (CN) = 69.0  
La (mm) = 22.82 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.469  
PEAK FLOW (cms) = 2.068 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 9.065  
TOTAL RAINFALL (mm) = 60.393  
RUNOFF COEFFICIENT = 0.150  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 60.39 mm  
IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

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VO output - Prop Uncontrolled Upstream.txt  
CALIB  
NASHYD ( 0152)  
ID= 1 DT=10.0 min  
Area (ha) = 90.60 Curve Number (CN) = 70.0  
La (mm) = 21.77 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20  
Unit Hyd Opeak (cms) = 17.302  
PEAK FLOW (cms) = 2.307 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 9.852  
TOTAL RAINFALL (mm) = 60.393  
RUNOFF COEFFICIENT = 0.163  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
1 + 2 = 3  
AREA OPEAK TPEAK R.V.  
(ha) (cms) (hrs) (mm)  
ID1= 1 ( 0152): 90.60 2.307 1.17 9.85  
+ ID2= 2 ( 0153): 91.47 2.068 1.17 9.07  
ID = 3 ( 0002): 182.07 4.375 1.17 9.46  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHICAGO STORM  
Ptotal = 60.39 mm  
IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
NASHYD ( 0151)  
ID= 1 DT=10.0 min  
Area (ha) = 18.20 Curve Number (CN) = 68.0  
La (mm) = 11.95 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20  
Unit Hyd Opeak (cms) = 3.476  
PEAK FLOW (cms) = 0.787 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 13.608  
TOTAL RAINFALL (mm) = 60.393  
RUNOFF COEFFICIENT = 0.225  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 60.39 mm  
IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

CHICAGO STORM  
Ptotal = 60.39 mm  
IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33  
Page 51

VO output - Prop Uncontrolled Upstream.txt  
Time to peak ratio = 0.33  
TIME RAIN TIME RAIN TIME RAIN TIME RAIN  
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr  
0.17 4.89 1.00 158.06 1.83 8.94 2.67 4.66  
0.33 6.10 1.17 44.35 2.00 7.51 2.83 4.27  
0.50 8.21 1.33 22.12 2.17 6.49 3.00 3.95  
0.67 12.90 1.50 14.74 2.33 5.73  
0.83 33.21 1.67 11.10 2.50 5.14

CALIB  
STANDHYD ( 1421)  
ID= 1 DT=10.0 min  
Area (ha) = 15.00 Total Imp(%) = 30.00 Dir. Conn. (%) = 29.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.50 10.50
Dep. Storage (mm)	1.00 2.00
Average Slope (%)	1.00 3.00
Length (m)	258.20 40.00
Mannings n	0.015 0.200
Max. Eff. Inten. (mm/hr) over (min)	158.06 50.65
Storage Coeff. (min)	10.00 20.00
Unit Hyd. Peak (min)	4.10 (ii) 11.27 (ii)
Unit Hyd. peak (cms)	0.16 0.08
PEAK FLOW (cms)	1.78 0.83
TIME TO PEAK (hrs)	1.00 1.17
RUNOFF VOLUME (mm)	59.39 21.24
TOTAL RAINFALL (mm)	60.39 60.39
RUNOFF COEFFICIENT	0.98 0.35

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN\* = 71.0 La = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT.  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 60.39 mm  
IDF curve parameters: A=1496.300  
B= 5.250  
C= 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB  
STANDHYD ( 0142)  
ID= 1 DT=10.0 min  
Area (ha) = 59.20 Total Imp(%) = 67.00 Dir. Conn. (%) = 64.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	39.66 19.54
Dep. Storage (mm)	1.00 16.04
Average Slope (%)	1.00 3.00

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VO output - Prop Uncontrolled Upstream.txt  
 Length (m) = 628.23 40.00  
 Mannings n = 0.015 0.250  
 Max. Eff. Inten. (mm/hr) = 158.06 34.58  
 over (min) = 10.00 20.00  
 Storage Coeff. (min) = 6.98 (ii) 16.54 (ii)  
 Unit Hyd. Tpeak (min) = 10.00 20.00  
 Unit Hyd. Tpeak (cms) = 0.13 0.06  
 PEAK FLOW (cms) = 13.37 0.95 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.00 13.715 (iii)  
 RUNOFF VOLUME (mm) = 59.39 17.51 44.32  
 TOTAL RAINFALL (mm) = 60.39 60.39 60.39  
 RUNOFF COEFFICIENT = 0.98 0.29 0.73

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 76.0 Ia = Dep Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM | IDF curve parameters: A=1496.300  
 Ptotal = 60.39 mm | B= 5.250  
 C= 0.825  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94
0.33	6.10	1.17	44.35	2.00	7.51
0.50	8.21	1.33	22.12	2.17	6.49
0.67	12.90	1.50	14.74	2.33	5.73
0.83	33.21	1.67	11.10	2.50	5.14

CALIB STANHYD ( 0022) | Area (ha) = 123.05  
 ID= 1 DT=10.0 min | Total Imp(%) = 61.00 Dir. Conn. (%) = 49.00

Surface Area (ha) = 75.06 IMPERVIOUS PERVIOUS (i)  
 Dep. Storage (mm) = 2.00 5.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 905.72 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 158.06 68.41  
 over (min) = 10.00 20.00  
 Storage Coeff. (min) = 7.98 (ii) 16.20 (ii)  
 Unit Hyd. Tpeak (min) = 10.00 20.00  
 Unit Hyd. Tpeak (cms) = 0.12 0.06  
 PEAK FLOW (cms) = 20.19 4.26 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.17 1.00  
 RUNOFF VOLUME (mm) = 58.39 22.25 39.96  
 TOTAL RAINFALL (mm) = 60.39 60.39 60.39  
 RUNOFF COEFFICIENT = 0.97 0.37 0.66

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 69.0 Ia = Dep Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
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VO output - Prop Uncontrolled Upstream.txt  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017) | AREA OPEAK TPEAK R.V.  
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1 = 1 ( 0142): 59.20 13.715 1.00 44.32  
 + ID2 = 2 ( 0022): 123.05 22.103 1.00 39.96  
 ID = 3 ( 0017): 182.25 35.817 1.00 41.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) | AREA OPEAK TPEAK R.V.  
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1 = 1 ( 1421): 15.00 2.165 1.00 32.30  
 + ID2 = 2 ( 0151): 18.20 0.787 1.17 13.61  
 ID = 3 ( 0003): 33.20 2.750 1.00 22.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) | AREA OPEAK TPEAK R.V.  
 3 + 2 = 1 | (ha) (cms) (hrs) (mm)  
 ID1 = 3 ( 0003): 33.20 2.750 1.00 22.05  
 + ID2 = 2 ( 0017): 182.25 35.817 1.00 41.37  
 ID = 1 ( 0003): 215.45 38.567 1.00 38.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) | AREA OPEAK TPEAK R.V.  
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1 = 1 ( 0003): 215.45 38.567 1.00 38.40  
 + ID2 = 2 ( 0002): 182.07 4.375 1.17 9.46  
 ID = 3 ( 0003): 397.52 40.913 1.00 25.14

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U A A L  
 V V I SSSSS UUUUU A A LLLL  
 000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y M M M 0 0  
 000 T T H H Y Y M M 000  
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

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VO output - Prop Uncontrolled Upstream.txt

Output filename: C:\Users\p001279\AppData\Local\CVI\vi\ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\ba6448  
 a7-c0b3-4f36-8ebc-4fe363c672b1\scse  
 Summary filename: C:\Users\p001279\AppData\Local\CVI\vi\ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\ba6448  
 a7-c0b3-4f36-8ebc-4fe363c672b1\scse

DATE: 11/28/2019 TIME: 11:04:50

USER:

COMMENTS:

\*\* SIMULATION : New DesignStormChicago - 3hr \*\*

CHI CAGO STORM | IDF curve parameters: A= 724.700  
 Ptotal = 33.36 mm | B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29
0.33	3.69	1.17	24.13	2.00	4.49
0.50	4.88	1.33	12.47	2.17	3.91
0.67	7.49	1.50	8.49	2.33	3.47
0.83	18.29	1.67	6.50	2.50	3.13

CALIB NASHYD ( 0153) | Area (ha) = 91.47 Curve Number (CN) = 69.0  
 ID= 1 DT=10.0 min | Ia (mm) = 22.82 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.469  
 PEAK FLOW (cms) = 0.134 (i)  
 TIME TO PEAK (hrs) = 1.833  
 RUNOFF VOLUME (mm) = 0.869  
 TOTAL RAINFALL (mm) = 33.364  
 RUNOFF COEFFICIENT = 0.026

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM | IDF curve parameters: A= 724.700  
 Ptotal = 33.36 mm | B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29
0.33	3.69	1.17	24.13	2.00	4.49

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VO output - Prop Uncontrolled Upstream.txt

0.50 4.88 1.33 12.47 2.17 3.91 3.00 2.44  
 0.83 18.29 1.67 6.50 2.50 3.13

CALIB NASHYD ( 0152) | Area (ha) = 90.60 Curve Number (CN) = 70.0  
 ID= 1 DT=10.0 min | Ia (mm) = 21.77 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.302  
 PEAK FLOW (cms) = 0.168 (i)  
 TIME TO PEAK (hrs) = 1.667  
 RUNOFF VOLUME (mm) = 1.087  
 TOTAL RAINFALL (mm) = 33.364  
 RUNOFF COEFFICIENT = 0.333

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002) | AREA OPEAK TPEAK R.V.  
 1 + 2 = 3 | (ha) (cms) (hrs) (mm)  
 ID1 = 1 ( 0152): 90.60 0.168 1.67 1.09  
 + ID2 = 2 ( 0153): 91.47 0.134 1.83 0.87  
 ID = 3 ( 0002): 182.07 0.299 1.83 0.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM | IDF curve parameters: A= 724.700  
 Ptotal = 33.36 mm | B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29
0.33	3.69	1.17	24.13	2.00	4.49
0.50	4.88	1.33	12.47	2.17	3.91
0.67	7.49	1.50	8.49	2.33	3.47
0.83	18.29	1.67	6.50	2.50	3.13

CALIB NASHYD ( 0151) | Area (ha) = 18.20 Curve Number (CN) = 68.0  
 ID= 1 DT=10.0 min | Ia (mm) = 11.95 # of Linear Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 3.476  
 PEAK FLOW (cms) = 0.134 (i)  
 TIME TO PEAK (hrs) = 1.333  
 RUNOFF VOLUME (mm) = 3.169  
 TOTAL RAINFALL (mm) = 33.364  
 RUNOFF COEFFICIENT = 0.095

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM | IDF curve parameters: A= 724.700

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VO output - Prop Uncontrolled Upstream.txt

Total = 33.36 mm
B = 5.000
C = 0.800
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00
Total Imp (%) = 30.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 4.50 10.50
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 3.00
Length (m) = 258.20 40.00
Mannings n = 0.015 0.200

Max. Eff. Inten. (mm/hr) = 83.04 11.44
Storage Coeff. (mi n) = 10.00 20.00
Unit t Hyd. Tpeak (mi n) = 10.00 (ii) 20.00 (ii)
Unit t Hyd. peak (cms) = 0.14 0.06
PEAK FLOW (cms) = 0.88 0.20
TIME TO PEAK (hrs) = 1.00 1.17
RUNOFF VOLUME (mm) = 32.36 7.37
TOTAL RAINFALL (mm) = 33.36 33.36
RUNOFF COEFFICIENT = 0.97 0.22

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A= 724.700 B= 5.000
Ptotal = 33.36 mm C= 0.800

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB STANDHYD ( 0142) Area (ha) = 59.20 Page 57

VO output - Prop Uncontrolled Upstream.txt

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017) AREA OPEAK TPEAK R.V.
1 + 2 = 3
ID1 = 1 ( 0142): 59.20 6.354 1.00 22.07
+ ID2 = 2 ( 0022): 123.05 9.679 1.00 19.18
ID = 3 ( 0017): 182.25 16.032 1.00 20.12

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) AREA OPEAK TPEAK R.V.
1 + 2 = 3
ID1 = 1 ( 1421): 15.00 0.971 1.00 14.62
+ ID2 = 2 ( 0151): 18.20 0.134 1.33 3.17
ID = 3 ( 0003): 33.20 1.039 1.00 8.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) AREA OPEAK TPEAK R.V.
3 + 2 = 1
ID1 = 3 ( 0003): 33.20 1.039 1.00 8.34
+ ID2 = 2 ( 0017): 182.25 16.032 1.00 20.12
ID = 1 ( 0003): 215.45 17.071 1.00 18.30

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) AREA OPEAK TPEAK R.V.
1 + 2 = 3
ID1 = 1 ( 0003): 215.45 17.071 1.00 18.30
+ ID2 = 2 ( 0002): 182.07 0.299 1.83 0.98
ID = 3 ( 0003): 397.52 17.071 1.00 10.37

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SS U U A A L
V V I SSSS UUUU A A LLLL
000 TTTT TTTT H H Y Y M M 000 TM
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M 000

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VO output - Prop Uncontrolled Upstream.txt
Total Imp (%) = 67.00 Dir. Conn. (%) = 64.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 39.66 19.54
Dep. Storage (mm) = 1.00 16.04
Average Slope (%) = 1.00 3.00
Length (m) = 628.23 40.00
Mannings n = 0.015 0.250

Max. Eff. Inten. (mm/hr) = 83.04 3.39
Storage Coeff. (mi n) = 10.00 40.00
Unit t Hyd. Tpeak (mi n) = 10.00 (ii) 33.23 (ii)
Unit t Hyd. peak (cms) = 0.11 0.03
PEAK FLOW (cms) = 6.35 0.11
TIME TO PEAK (hrs) = 1.00 1.83
RUNOFF VOLUME (mm) = 32.36 3.78
TOTAL RAINFALL (mm) = 33.36 33.36
RUNOFF COEFFICIENT = 0.97 0.11

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 76.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A= 724.700 B= 5.000
Ptotal = 33.36 mm C= 0.800

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB STANDHYD ( 0022) Area (ha) = 123.05 Dir. Conn. (%) = 49.00
Total Imp (%) = 61.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 75.06 47.99
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 1.00 2.00
Length (m) = 905.72 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 83.04 14.73
Storage Coeff. (mi n) = 10.32 (ii) 25.51 (ii)
Unit t Hyd. Tpeak (mi n) = 10.00 30.00
Unit t Hyd. peak (cms) = 0.11 0.04
PEAK FLOW (cms) = 9.46 0.93
TIME TO PEAK (hrs) = 1.00 1.50
RUNOFF VOLUME (mm) = 31.36 7.47
TOTAL RAINFALL (mm) = 33.36 33.36
RUNOFF COEFFICIENT = 0.94 0.22

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!
(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Uncontrolled Upstream.txt
\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vivaldi\OTTHYMO 5.2\VO2\voi.n.dat
Output filename:
C:\Users\p001279d\AppData\Local\Ci vi ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\5dccc187-a065-4d2a-b3bf-43029a05d4f2\Sce
Summary File name:
C:\Users\p001279d\AppData\Local\Ci vi ca\VH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\5dccc187-a065-4d2a-b3bf-43029a05d4f2\Sce

DATE: 11/28/2019 TIME: 11:04:49
USER:

COMMENTS:
\*\*\*\*\*
\*\* SIMULATION : New DesignStormChicago - 3hr \*\*

CHI CAGO STORM IDF curve parameters: A=1330.300 B= 7.938
Ptotal = 45.35 mm C= 0.855

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Rows show rainfall intensity over time.

CALIB NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0
ID= 1 DT=10.0 min Ia (mm) = 22.82 # of Linear Res. (N) = 3.00
U. H. Tp (hrs) = 0.20

Unit t Hyd Opeak (cms) = 17.469
PEAK FLOW (cms) = 0.768 (i)
TIME TO PEAK (hrs) = 1.333
RUNOFF VOLUME (mm) = 3.617
TOTAL RAINFALL (mm) = 45.346
RUNOFF COEFFICIENT = 0.080

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300 B= 7.938
Ptotal = 45.35 mm C= 0.855

used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

VO output - Prop Uncontrolled Upstream.txt

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data at various time intervals.

CALIB NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.20
Unit Hyd Opeak (cms) = 17.302
PEAK FLOW (cms) = 0.887 (i)
TIME TO PEAK (hrs) = 1.333
RUNOFF VOLUME (mm) = 4.088
TOTAL RAINFALL (mm) = 45.346
RUNOFF COEFFICIENT = 0.090

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002) AREA OPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
+ ID1= 1 ( 0152): 90.60 0.887 1.33 4.09
+ ID2= 2 ( 0153): 91.47 0.768 1.33 3.62
ID = 3 ( 0002): 182.07 1.656 1.33 3.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300
Ptotal = 45.35 mm B= 7.938
C= 0.855
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data for Chicago storm.

CALIB NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0
ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Linear Res. (N) = 3.00
U.H. Tp(hrs) = 0.20
Unit Hyd Opeak (cms) = 3.476
PEAK FLOW (cms) = 0.377 (i)
TIME TO PEAK (hrs) = 1.167
RUNOFF VOLUME (mm) = 7.103
TOTAL RAINFALL (mm) = 45.346
RUNOFF COEFFICIENT = 0.157

VO output - Prop Uncontrolled Upstream.txt
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300
Ptotal = 45.35 mm B= 7.938
C= 0.855
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data for Chicago storm.

CALIB STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00
ID= 1 DT=10.0 min Total Imp(%) = 30.00

Surface Area (ha) = 4.50 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 10.50
Average Slope (%) = 1.00 3.00
Length (m) = 258.20 40.00
Mannings n = 0.015 0.200
Max. Eff. Inten. (mm/hr) = 112.71 28.23
over (min) = 10.00 20.00
Storage Coeff. (min) = 4.69 (ii) 13.75 (ii)
Unit Hyd. Tpeak (min) = 10.00 20.00
Unit Hyd. peak (cms) = 0.15 0.07
PEAK FLOW (cms) = 1.24 0.44 \*TOTALS\*
TIME TO PEAK (hrs) = 1.00 1.17 1.00
RUNOFF VOLUME (mm) = 44.35 12.92 57.27
TOTAL RAINFALL (mm) = 45.35 45.35 90.70
RUNOFF COEFFICIENT = 0.98 0.28 0.49

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300
Ptotal = 45.35 mm B= 7.938
C= 0.855
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data for Chicago storm.

VO output - Prop Uncontrolled Upstream.txt

CALIB STANDHYD ( 0142) Area (ha) = 59.20 Dir. Conn. (%) = 64.00
ID= 1 DT=10.0 min Total Imp(%) = 67.00

Surface Area (ha) = 19.54 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 16.04
Average Slope (%) = 1.00 3.00
Length (m) = 628.23 40.00
Mannings n = 0.015 0.250
Max. Eff. Inten. (mm/hr) = 112.71 12.43
over (min) = 10.00 30.00
Storage Coeff. (min) = 7.99 (ii) 22.38 (ii)
Unit Hyd. Tpeak (min) = 10.00 30.00
Unit Hyd. peak (cms) = 0.12 0.05
PEAK FLOW (cms) = 9.13 0.39 \*TOTALS\*
TIME TO PEAK (hrs) = 1.00 1.50 9.185 (iii)
RUNOFF VOLUME (mm) = 44.35 9.01 53.36
TOTAL RAINFALL (mm) = 45.35 45.35 90.70
RUNOFF COEFFICIENT = 0.98 0.20 0.70

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 76.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300
Ptotal = 45.35 mm B= 7.938
C= 0.855
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 8 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr). Rows show rainfall data for Chicago storm.

CALIB STANDHYD ( 0022) Area (ha) = 123.05 Dir. Conn. (%) = 49.00
ID= 1 DT=10.0 min Total Imp(%) = 61.00

Surface Area (ha) = 75.06 IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 2.00 47.99
Average Slope (%) = 1.00 5.00
Length (m) = 905.72 40.00
Mannings n = 0.013 0.250
Max. Eff. Inten. (mm/hr) = 112.71 28.46
over (min) = 10.00 30.00
Storage Coeff. (min) = 9.14 (ii) 20.80 (ii)
Unit Hyd. Tpeak (min) = 10.00 30.00
Unit Hyd. peak (cms) = 0.11 0.05
PEAK FLOW (cms) = 13.71 2.03 \*TOTALS\*
14.213 (iii)

VO output - Prop Uncontrolled Upstream.txt
TIME TO PEAK (hrs) = 1.00 1.33 1.00
RUNOFF VOLUME (mm) = 43.35 13.39 56.74
TOTAL RAINFALL (mm) = 45.35 45.35 90.70
RUNOFF COEFFICIENT = 0.96 0.30 0.62

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0017) AREA OPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
+ ID1= 1 ( 0142): 59.20 9.185 1.00 31.63
+ ID2= 2 ( 0022): 123.05 14.213 1.00 28.07
ID = 3 ( 0017): 182.25 23.398 1.00 29.22

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) AREA OPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
+ ID1= 1 ( 1421): 15.00 1.433 1.00 22.03
+ ID2= 2 ( 0151): 18.20 0.377 1.17 7.10
ID = 3 ( 0003): 33.20 1.665 1.00 13.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) AREA OPEAK TPEAK R.V.
3 + 2 = 1 (ha) (cms) (hrs) (mm)
+ ID1= 3 ( 0003): 33.20 1.665 1.00 13.85
+ ID2= 2 ( 0017): 182.25 23.398 1.00 29.22
ID = 1 ( 0003): 215.45 25.064 1.00 26.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003) AREA OPEAK TPEAK R.V.
1 + 2 = 3 (ha) (cms) (hrs) (mm)
+ ID1= 1 ( 0003): 215.45 25.064 1.00 26.85
+ ID2= 2 ( 0002): 182.07 1.665 1.33 3.85
ID = 3 ( 0003): 397.52 25.355 1.00 16.32

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

VO output - Prop Controlled Upstream.txt

VO output - Prop Controlled Upstream.txt

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Values: 0.167, 2.04, 0.500, 87.80, 0.833, 26.54, 1.17, 2.04.

Unit Hyd Opeak (cms)= 17.469
PEAK FLOW (cms)= 0.452 (i)
TIME TO PEAK (hrs)= 0.833
RUNOFF VOLUME (mm)= 0.977
TOTAL RAINFALL (mm)= 34.030
RUNOFF COEFFICIENT = 0.029

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A L
V V I SS U U A A A A L
V V I SSSSS U U A A L
W I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M 000 TM
O O T T H H Y Y M M O O
O O T T H H Y Y M M O O
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYM0 5.2\VO2\voi.n.dat
Output filename:
C:\Users\p001279d\AppData\Local\Civica\VAHS\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\7ee98b6e-84af-4924-99ef-6f5d3b07ef37\sce
Summary filename:
C:\Users\p001279d\AppData\Local\Civica\VAHS\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\7ee98b6e-84af-4924-99ef-6f5d3b07ef37\sce

DATE: 11/28/2019 TIME: 10:09:37
USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 01 AES 2-Yr \*\*\*\*\*

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\376738e9
Ptotal = 34.03 mm Comments: 2yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

CALIB NASHVD ( 0153) Area (ha)= 91.47 Curve Number (CN)= 69.0
ID= 1 DT=10.0 min Ia (mm)= 22.82 # of Li near Res. (N)= 3.00
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Values: 0.167, 2.04, 0.500, 87.80, 0.833, 26.54, 1.17, 2.04.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\376738e9
Ptotal = 34.03 mm Comments: 2yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

CALIB NASHVD ( 0152) Area (ha)= 90.60 Curve Number (CN)= 70.0
ID= 1 DT=10.0 min Ia (mm)= 21.77 # of Li near Res. (N)= 3.00
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.04 0.500 87.80 0.833 26.54 1.17 2.04
0.333 22.46 0.667 55.13 1.000 8.17 1.17 2.04
Unit Hyd Opeak (cms)= 17.302
PEAK FLOW (cms)= 0.564 (i)
TIME TO PEAK (hrs)= 0.833
RUNOFF VOLUME (mm)= 1.209
TOTAL RAINFALL (mm)= 34.030
RUNOFF COEFFICIENT = 0.036

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)
1 + 2 = 3 90.60 0.564 0.833 1.21
+ ID2= 2 ( 0153): 91.47 0.452 0.83 0.98
ID = 3 ( 0002): 182.07 1.016 0.83 1.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\376738e9
Ptotal = 34.03 mm Comments: 2yr 1hr AES
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VO output - Prop Controlled Upstream.txt

VO output - Prop Controlled Upstream.txt

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Values: 0.167, 2.04, 0.500, 87.80, 0.833, 26.54, 1.17, 2.04.

Storage over (min)= 10.00 20.00
Unit Hyd. Tpeak (min)= 5.18 (ii) 16.61 (ii)
Unit Hyd. peak (cms)= 0.15 0.06
PEAK FLOW (cms)= 0.93 0.33
TIME TO PEAK (hrs)= 0.50 0.83
RUNOFF VOLUME (mm)= 33.03 7.65
TOTAL RAINFALL (mm)= 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.22

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHVD ( 0151) Area (ha)= 18.20 Curve Number (CN)= 68.0
ID= 1 DT=10.0 min Ia (mm)= 11.95 # of Li near Res. (N)= 3.00
U.H. Tp(hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Values: 0.167, 2.04, 0.500, 87.80, 0.833, 26.54, 1.17, 2.04.

Unit Hyd Opeak (cms)= 3.476
PEAK FLOW (cms)= 0.312 (i)
TIME TO PEAK (hrs)= 0.833
RUNOFF VOLUME (mm)= 3.353
TOTAL RAINFALL (mm)= 34.030
RUNOFF COEFFICIENT = 0.099

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\376738e9
Ptotal = 34.03 mm Comments: 2yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

CALIB STANDHYD ( 1421) Area (ha)= 15.00 Dir. Conn. (%)= 29.00
Total Imp(%)= 30.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 4.50 10.50
Dep. Storage (mm)= 1.00 2.00
Average Slope (%)= 1.00 3.00
Length (m)= 258.20 40.00
Mannings n = 0.015 0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

Table with 8 columns: TIME, RAIN, TIME, RAIN, TIME, RAIN, TIME, RAIN. Values: 0.167, 2.04, 0.500, 87.80, 0.833, 26.54, 1.17, 2.04.

Max. Eff. Inten. (mm/hr)= 87.80 15.83
Page 3

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\376738e9
Ptotal = 34.03 mm Comments: 2yr 1hr AES
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 61.25 0.75 32.67 1.08 4.08
0.17 4.08 0.50 114.34 0.83 20.42
0.25 12.25 0.58 61.25 0.92 12.25
0.33 32.67 0.67 49.00 1.00 4.08

CALIB STANDHYD ( 0142) Area (ha)= 54.15 Dir. Conn. (%)= 64.00
Total Imp(%)= 67.00
IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 36.28 17.87
Dep. Storage (mm)= 1.00 16.04
Average Slope (%)= 1.00 3.00
Length (m)= 600.83 40.00
Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.04 0.500 87.80 0.833 26.54 1.17 2.04
0.333 22.46 0.667 55.13 1.000 8.17 1.17 2.04
Max. Eff. Inten. (mm/hr)= 87.80 10.47
Storage over (min)= 10.00 30.00
Storage Coeff. (min)= 8.60 (ii) 24.01 (ii)
Unit Hyd. Tpeak (min)= 10.00 30.00
Unit Hyd. peak (cms)= 0.12 0.04

PEAK FLOW (cms)= 6.19 0.25
TIME TO PEAK (hrs)= 0.50 1.17
RUNOFF VOLUME (mm)= 33.03 4.02
TOTAL RAINFALL (mm)= 34.03 34.03
RUNOFF COEFFICIENT = 0.97 0.12

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 76.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL

VO output - Prop Controlled Upstream.txt  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 34.03 mm  
 Filename: C:\Users\p001279d\AppData  
 at\Local Temp  
 fb082f85-bf5b-44d7-8765-aa036ef20a94\376738e9  
 Comments: 2yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	61.25	0.75	32.67	1.08	4.08
0.17	4.08	0.50	114.34	0.83	20.42		
0.25	12.25	0.58	61.25	0.92	12.25		
0.33	32.67	0.67	49.00	1.00	4.08		

STANDHYD ( 0022)  
 ID= 1 DT=10.0 min  
 Area (ha) = 127.59  
 Total Imp(%) = 61.00  
 Dir. Conn. (%) = 49.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 77.83	49.76
Dep. Storage (mm) = 2.00	5.00
Average Slope (%) = 1.00	2.00
Length (m) = 922.38	40.00
Mannings n = 0.013	0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.04	0.500	87.80	0.833	26.54
0.333	22.46	0.667	55.13	1.000	8.17

Max. Eff. Inten. (mm/hr) = 87.80  
 over (min) = 10.00  
 Storage Coeff. (min) = 10.21 (ii)  
 Unit t. Tpeak (min) = 10.00  
 Unit t Hyd. peak (cms) = 0.11

PEAK FLOW (cms) = 10.03  
 TIME TO PEAK (hrs) = 0.50  
 RUNOFF VOLUME (mm) = 32.03  
 TOTAL RAINFALL (mm) = 34.03  
 RUNOFF COEFFICIENT = 0.94

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 69.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019)  
 IN= 2---> OUT= 1  
 DT= 10.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

AREA (ha) = 127.590  
 OPEAK (cms) = 10.432  
 TPEAK (hrs) = 0.67  
 R. V. (mm) = 19.66

INFLOW : ID= 2 ( 0022)  
 OUTFLOW : ID= 1 ( 0026)

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VO output - Prop Controlled Upstream.txt

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
12.66	251.31	0.0300	0.0000	Main Channel	
0.30	14.58	0.0300	0.0000	Main Channel	
76.79	251.08	0.0300	0.0000	Main Channel	
87.59	252.17	0.0300	0.0000	Main Channel	
90.94	252.44	0.0300	0.0500	Main Channel	
98.65	253.00	0.0500	0.0500	Main Channel	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.10	251.15	671E+04	4.4	0.82	25.64
0.20	251.25	148E+05	15.9	1.36	15.43
0.30	251.35	230E+05	32.9	1.80	11.67
0.40	251.45	315E+05	54.5	2.18	9.63
0.49	251.55	402E+05	80.5	2.53	8.32
0.59	251.64	491E+05	110.6	2.84	7.40
0.69	251.74	582E+05	144.6	3.13	6.71
0.79	251.84	676E+05	182.5	3.41	6.17
0.89	251.94	771E+05	224.1	3.67	5.74
0.99	252.04	869E+05	269.3	3.91	5.38
1.09	252.14	969E+05	318.2	4.15	5.07
1.19	252.24	107E+06	370.2	4.36	4.82
1.29	252.34	118E+06	425.6	4.57	4.60
1.39	252.44	128E+06	484.6	4.77	4.41
1.49	252.54	140E+06	554.9	5.02	4.19
1.59	252.64	151E+06	632.3	5.28	3.99
1.69	252.75	163E+06	714.0	5.52	3.81
1.80	252.85	176E+06	799.8	5.75	3.66
1.90	252.95	188E+06	889.9	5.97	3.52

INFLOW : ID= 2 ( 0017)  
 OUTFLOW : ID= 1 ( 0015)

AREA (ha) = 181.74  
 OPEAK (cms) = 6.21  
 TPEAK (hrs) = 0.50  
 R. V. (mm) = 20.51

MAX DEPTH (m) = 0.11  
 MAX VEL (m/s) = 0.88

RESERVOIR ( 0020)  
 IN= 2---> OUT= 1  
 DT= 10.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

AREA (ha) = 181.740  
 OPEAK (cms) = 3.458  
 TPEAK (hrs) = 0.83  
 R. V. (mm) = 20.51

INFLOW : ID= 2 ( 0015)  
 OUTFLOW : ID= 1 ( 0020)

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 16.57  
 TIME SHIFT OF PEAK FLOW (min) = 80.00  
 MAXIMUM STORAGE USED (ha. m.) = 0.9762

ADD HYD ( 0003)  
 1 + 2 = 3

ID1	AREA	OPEAK	TPEAK	R. V.
( 1421)	(ha)	(cms)	(hrs)	(mm)
+ ID2 = 2 ( 0151)	15.00	1.013	0.50	15.01
	18.20	0.312	0.83	3.35

ID = 3 ( 0003): 33.20 1.190 0.67 8.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 1 + 2 = 3

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VO output - Prop Controlled Upstream.txt  
 OUTFLOW: ID= 1 ( 0019) 127.590 0.187 2.00 19.63

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 1.79  
 TIME SHIFT OF PEAK FLOW (min) = 80.00  
 MAXIMUM STORAGE USED (ha. m.) = 2.3929

ROUTE CHN( 0026)  
 IN= 2---> OUT= 1

Routing time step (min) = 10.00

Distance	Elevation	Manning	Main Channel
0.00	274.12	0.0500	
5.20	273.17	0.0300	
10.58	272.11	0.0300	Main Channel
11.50	272.00	0.0300	Main Channel
56.67	272.00	0.0300	Main Channel
64.61	272.44	0.0300	Main Channel
68.47	272.73	0.0300	Main Channel
73.75	273.06	0.0300 / 0.0500	Main Channel
79.53	273.29	0.0500	Main Channel

TRAVEL TIME TABLE

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.07	272.07	104E+04	0.9	0.29	19.48
0.13	272.13	211E+04	2.8	0.46	12.41
0.20	272.20	322E+04	5.6	0.59	9.56
0.26	272.26	436E+04	9.1	0.71	7.96
0.33	272.33	554E+04	13.3	0.82	6.93
0.40	272.40	676E+04	18.2	0.91	6.19
0.46	272.46	801E+04	23.7	1.01	5.62
0.53	272.53	928E+04	30.0	1.09	5.16
0.60	272.60	106E+05	36.8	1.18	4.80
0.66	272.66	119E+05	44.2	1.26	4.49
0.73	272.73	133E+05	52.2	1.33	4.24
0.79	272.79	147E+05	60.7	1.40	4.03
0.86	272.86	161E+05	69.7	1.47	3.84
0.93	272.93	175E+05	79.3	1.54	3.68
0.99	272.99	190E+05	89.5	1.60	3.54
1.06	273.06	205E+05	100.3	1.66	3.41
1.14	273.14	223E+05	114.9	1.74	3.24
1.22	273.22	242E+05	130.7	1.83	3.09
1.29	273.29	262E+05	147.5	1.91	2.96

INFLOW : ID= 2 ( 0019) 127.59 0.19 2.00 19.63  
 OUTFLOW : ID= 1 ( 0026) 127.59 0.19 2.67 19.63

ADD HYD ( 0017)  
 1 + 2 = 3

ID1	AREA	OPEAK	TPEAK	R. V.
( 0142)	(ha)	(cms)	(hrs)	(mm)
+ ID2 = 2 ( 0026)	54.15	6.198	0.50	22.59
	127.59	0.185	2.67	19.63

ID = 3 ( 0017): 181.74 6.207 0.50 20.51

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN( 0015)  
 IN= 2---> OUT= 1

Routing time step (min) = 10.00

Distance	Elevation	Manning	Main Channel
3.65	252.95	0.0500 / 0.0300	Main Channel

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TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
127.59	251.31	0.0300	0.0000	Main Channel	
128.89	251.08	0.0300	0.0000	Main Channel	
130.19	252.17	0.0300	0.0000	Main Channel	
131.49	252.44	0.0300	0.0500	Main Channel	
132.79	253.00	0.0500	0.0500	Main Channel	

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu. m.)	(cms)	(m/s)	(min)
0.10	251.15	671E+04	4.4	0.82	25.64
0.20	251.25	148E+05	15.9	1.36	15.43
0.30	251.35	230E+05	32.9	1.80	11.67
0.40	251.45	315E+05	54.5	2.18	9.63
0.49	251.55	402E+05	80.5	2.53	8.32
0.59	251.64	491E+05	110.6	2.84	7.40
0.69	251.74	582E+05	144.6	3.13	6.71
0.79	251.84	676E+05	182.5	3.41	6.17
0.89	251.94	771E+05	224.1	3.67	5.74
0.99	252.04	869E+05	269.3	3.91	5.38
1.09	252.14	969E+05	318.2	4.15	5.07
1.19	252.24	107E+06	370.2	4.36	4.82
1.29	252.34	118E+06	425.6	4.57	4.60
1.39	252.44	128E+06	484.6	4.77	4.41
1.49	252.54	140E+06	554.9	5.02	4.19
1.59	252.64	151E+06	632.3	5.28	3.99
1.69	252.75	163E+06	714.0	5.52	3.81
1.80	252.85	176E+06	799.8	5.75	3.66
1.90	252.95	188E+06	889.9	5.97	3.52

INFLOW : ID= 2 ( 0017)  
 OUTFLOW : ID= 1 ( 0015)

AREA (ha) = 181.74  
 OPEAK (cms) = 6.21  
 TPEAK (hrs) = 0.50  
 R. V. (mm) = 20.51

MAX DEPTH (m) = 0.11  
 MAX VEL (m/s) = 0.88

RESERVOIR ( 0003)  
 IN= 2---> OUT= 1  
 DT= 10.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha. m.)	(cms)	(ha. m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

AREA (ha) = 181.740  
 OPEAK (cms) = 3.458  
 TPEAK (hrs) = 0.83  
 R. V. (mm) = 20.51

INFLOW : ID= 2 ( 0015)  
 OUTFLOW : ID= 1 ( 0020)

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 16.57  
 TIME SHIFT OF PEAK FLOW (min) = 80.00  
 MAXIMUM STORAGE USED (ha. m.) = 0.9762

ADD HYD ( 0003)  
 1 + 2 = 3

ID1	AREA	OPEAK	TPEAK	R. V.
( 1421)	(ha)	(cms)	(hrs)	(mm)
+ ID2 = 2 ( 0151)	15.00	1.013	0.50	15.01
	18.20	0.312	0.83	3.35

ID = 3 ( 0003): 33.20 1.190 0.67 8.62

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 1 + 2 = 3

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VO output - Prop Controlled Upstream.txt
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 62.35 0.75 33.25 1.08 4.16
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB
NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0
ID= 1 DT=10.0 min Ia (mm) = 22.82 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.08 0.500 89.37 0.833 27.02 1.17 2.08
0.333 22.86 0.667 56.12 1.000 8.31

Unit Hyd Opeak (cms) = 17.469
PEAK FLOW (cms) = 0.504 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 1.081
TOTAL RAINFALL (mm) = 34.640
RUNOFF COEFFICIENT = 0.031

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData
Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c44996e4
Ptotal = 34.64 mm Comments: 5yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 62.35 0.75 33.25 1.08 4.16
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB
NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.08 0.500 89.37 0.833 27.02 1.17 2.08
0.333 22.86 0.667 56.12 1.000 8.31

Unit Hyd Opeak (cms) = 17.302
PEAK FLOW (cms) = 0.621 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 1.325
TOTAL RAINFALL (mm) = 34.640
RUNOFF COEFFICIENT = 0.038

VO output - Prop Controlled Upstream.txt
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)
1 + 2 = 3
AREA OPEAK TPEAK R.V.
(ha) (cms) (hrs) (mm)
ID1= 1 ( 0152): 90.60 0.621 0.83 1.33
+ ID2= 2 ( 0153): 91.47 0.504 0.83 1.08
ID = 3 ( 0002): 182.07 1.125 0.83 1.20

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData
Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c44996e4
Ptotal = 34.64 mm Comments: 5yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 62.35 0.75 33.25 1.08 4.16
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB
NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0
ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Li near Res. (N) = 3.00
U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.08 0.500 89.37 0.833 27.02 1.17 2.08
0.333 22.86 0.667 56.12 1.000 8.31

Unit Hyd Opeak (cms) = 3.476
PEAK FLOW (cms) = 0.327 (i)
TIME TO PEAK (hrs) = 0.833
RUNOFF VOLUME (mm) = 3.526
TOTAL RAINFALL (mm) = 34.640
RUNOFF COEFFICIENT = 0.102

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData
Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c44996e4
Ptotal = 34.64 mm Comments: 5yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 62.35 0.75 33.25 1.08 4.16
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

VO output - Prop Controlled Upstream.txt
CALIB
STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00
ID= 1 DT=10.0 min Total Imp(%) = 30.00
Surface Area (ha) IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 1.00 2.00
Average Slope (%) = 1.00 3.00
Length (m) = 258.20 40.00
Mannings n = 0.015 0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.08 0.500 89.37 0.833 27.02 1.17 2.08
0.333 22.86 0.667 56.12 1.000 8.31

Max. Eff. Inten. (mm/hr) = 89.37 16.38
over (mi n) 10.00 20.00
Storage Coeff. (mi n) = 5.14 (ii) 16.42 (ii)
Unit Hyd. Tpeak (mi n) = 10.00 4.25
Unit Hyd. peak (cms) = 0.15 0.06
PEAK FLOW (cms) = 0.95 0.34
TIME TO PEAK (hrs) = 0.50 0.83
RUNOFF VOLUME (mm) = 33.64 7.91
TOTAL RAINFALL (mm) = 34.64 34.64
RUNOFF COEFFICIENT = 0.97 0.44

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData
Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c44996e4
Ptotal = 34.64 mm Comments: 5yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 62.35 0.75 33.25 1.08 4.16
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB
STANDHYD ( 0142) Area (ha) = 54.15 Dir. Conn. (%) = 64.00
ID= 1 DT=10.0 min Total Imp(%) = 67.00
Surface Area (ha) IMPERVIOUS PERVIOUS (i)
Dep. Storage (mm) = 36.28 17.87
Average Slope (%) = 1.00 3.00
Length (m) = 600.83 40.00
Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.08 0.500 89.37 0.833 27.02 1.17 2.08
0.333 22.86 0.667 56.12 1.000 8.31

VO output - Prop Controlled Upstream.txt
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.08 0.500 89.37 0.833 27.02 1.17 2.08
0.333 22.86 0.667 56.12 1.000 8.31
Max. Eff. Inten. (mm/hr) = 89.37 11.03
over (mi n) 10.00 30.00
Storage Coeff. (mi n) = 8.54 (ii) 23.63 (ii)
Unit Hyd. Tpeak (mi n) = 10.00 30.00
Unit Hyd. peak (cms) = 0.12 0.04
PEAK FLOW (cms) = 6.33 0.27
TIME TO PEAK (hrs) = 0.50 1.00
RUNOFF VOLUME (mm) = 33.64 4.25
TOTAL RAINFALL (mm) = 34.64 34.64
RUNOFF COEFFICIENT = 0.97 0.12

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:
CN\* = 76.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL
THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData
Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c44996e4
Ptotal = 34.64 mm Comments: 5yr 1hr AES

TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.08 0.00 0.42 62.35 0.75 33.25 1.08 4.16
0.17 4.16 0.50 116.39 0.83 20.78
0.25 12.47 0.58 62.35 0.92 12.47
0.33 33.25 0.67 49.88 1.00 4.16

CALIB
STANDHYD ( 0022) Area (ha) = 127.59 Dir. Conn. (%) = 49.00
ID= 1 DT=10.0 min Total Imp(%) = 61.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha) = 77.83 49.76
Dep. Storage (mm) = 2.00 5.00
Average Slope (%) = 1.00 2.00
Length (m) = 922.28 40.00
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH
TIME RAIN TIME RAIN TIME RAIN TIME RAIN
hrs mm/hr hrs mm/hr hrs mm/hr hrs mm/hr
0.167 2.08 0.500 89.37 0.833 27.02 1.17 2.08
0.333 22.86 0.667 56.12 1.000 8.31

Max. Eff. Inten. (mm/hr) = 89.37 21.19
over (mi n) 10.00 30.00
Storage Coeff. (mi n) = 10.13 (ii) 23.26 (ii)
Unit Hyd. Tpeak (mi n) = 10.00 30.00
Unit Hyd. peak (cms) = 0.11 0.04
PEAK FLOW (cms) = 10.26 1.70
TIME TO PEAK (hrs) = 0.50 1.00
RUNOFF VOLUME (mm) = 32.64 8.04
TOTAL RAINFALL (mm) = 34.64 34.64
RUNOFF COEFFICIENT = 0.94 0.23

VO output - Prop Controlled Upstream.txt

- (i) CN PROCEDURE SELECTED FOR PEROVIOUS LOSSES: CN = 69.0
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019 )  
IN= 2--> OUT= 1  
DT= 10.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3120
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
127.590	10.655	0.67	20.10
127.590	0.189	2.00	20.07

PEAK FLOW REDUCTION [Qout/Oin] (%) = 1.77  
TIME SHIFT OF PEAK FLOW (min) = 80.00  
MAXIMUM STORAGE USED (ha.m.) = 2.4477

ROUTE CHN( 0026 )  
IN= 2--> OUT= 1  
Routing time step (min)' = 10.00

Distance	Elevation	Manning	
0.00	274.12	0.0500	
5.20	273.17	0.0300	Main Channel
10.58	272.11	0.0300	Main Channel
11.50	272.00	0.0300	Main Channel
56.67	272.00	0.0300	Main Channel
64.61	272.44	0.0300	Main Channel
68.47	272.73	0.0300	Main Channel
73.75	273.06	0.0300 / 0.0500	Main Channel
79.53	273.29	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.07	272.07	1.04E+04	0.9	0.29	19.48
0.13	272.13	2.11E+04	2.8	0.46	12.41
0.20	272.20	3.22E+04	5.6	0.59	9.56
0.26	272.26	4.36E+04	9.1	0.71	7.96
0.33	272.33	5.54E+04	13.3	0.82	6.93
0.40	272.40	6.76E+04	18.2	0.91	6.19
0.46	272.46	8.01E+04	23.7	1.01	5.62
0.53	272.53	9.28E+04	30.0	1.09	5.16
0.60	272.60	1.06E+05	36.8	1.18	4.80
0.66	272.66	1.19E+05	44.2	1.26	4.49
0.73	272.73	1.33E+05	52.7	1.33	4.24
0.79	272.79	1.47E+05	60.7	1.40	4.03
0.86	272.86	1.61E+05	69.7	1.47	3.84
0.93	272.93	1.75E+05	79.3	1.54	3.68
0.99	272.99	1.90E+05	89.5	1.60	3.54
1.06	273.06	2.05E+05	100.3	1.66	3.41
1.14	273.14	2.23E+05	114.9	1.74	3.24
1.22	273.22	2.42E+05	130.7	1.83	3.09
1.29	273.29	2.62E+05	147.5	1.91	2.96

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
127.59	0.19	2.00	20.07	0.01	0.29
127.59	0.19	2.67	20.07	0.01	0.29

VO output - Prop Controlled Upstream.txt

ADD HYD ( 0017 )  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
154.15	6.333	0.50	23.06
127.59	0.188	2.67	20.07
181.74	6.342	0.50	20.96

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN( 0015 )  
IN= 2--> OUT= 1  
Routing time step (min)' = 10.00

DATA FOR SECTION ( 1.1 )

Distance	Elevation	Manning	
0.00	252.95	0.0500	
3.65	252.53	0.0300	Main Channel
12.66	251.31	0.0300	Main Channel
14.58	251.05	0.0300	Main Channel
76.79	251.08	0.0300	Main Channel
87.59	252.17	0.0300	Main Channel
90.94	252.44	0.0300 / 0.0500	Main Channel
98.65	253.00	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.20	251.25	1.48E+05	15.9	1.36	15.43
0.30	251.35	2.30E+05	32.9	1.80	11.67
0.40	251.45	3.15E+05	54.5	2.18	9.63
0.49	251.55	4.02E+05	80.5	2.53	8.32
0.59	251.64	4.91E+05	110.6	2.84	7.40
0.69	251.74	5.82E+05	144.6	3.13	6.71
0.79	251.84	6.76E+05	182.5	3.41	6.17
0.89	251.94	7.71E+05	224.1	3.67	5.74
0.99	252.04	8.69E+05	269.3	3.91	5.38
1.09	252.14	9.69E+05	318.2	4.15	5.07
1.19	252.24	1.07E+06	370.2	4.36	4.82
1.29	252.34	1.18E+06	425.6	4.57	4.60
1.39	252.44	1.28E+06	484.6	4.77	4.41
1.49	252.54	1.40E+06	554.9	5.02	4.19
1.59	252.64	1.51E+06	632.3	5.28	3.99
1.69	252.75	1.63E+06	714.0	5.52	3.81
1.80	252.85	1.76E+06	799.8	5.75	3.66
1.90	252.95	1.88E+06	889.9	5.97	3.52

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
181.74	6.34	0.50	20.96	0.12	0.88
181.74	3.53	0.83	20.96	0.08	0.82

RESERVOIR ( 0020 )  
IN= 2--> OUT= 1  
DT= 10.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
181.740	3.530	0.83	20.96
181.740	0.585	2.17	20.96

VO output - Prop Controlled Upstream.txt

PEAK FLOW REDUCTION [Qout/Oin] (%) = 16.57  
TIME SHIFT OF PEAK FLOW (min) = 80.00  
MAXIMUM STORAGE USED (ha.m.) = 0.9964

ADD HYD ( 0003 )  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
15.00	1.035	0.50	15.37
18.20	0.327	0.83	3.53
33.20	1.227	0.67	8.88

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
182.07	1.125	0.83	1.20
215.27	2.176	0.83	2.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
215.27	2.176	0.83	2.39
181.74	0.585	2.17	20.96
397.01	2.391	0.83	10.89

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
W I SSSS UUUU A A LLLLL  
000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual\OTHYMO 5.2\VO2\vo1.n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Ci\vi\ca\WH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\F34F00  
c8-c6d7-4736-98ef-6a6ee4c06c3\scse  
Summary filename:  
C:\Users\p001279d\AppData\Local\Ci\vi\ca\WH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\F34F00  
c8-c6d7-4736-98ef-6a6ee4c06c3\scse

VO output - Prop Controlled Upstream.txt

USER:

COMMENTS:

\*\*\* SIMULATION : 03 AES 10-Yr \*\*\*

READ STORM  
Total = 41.26 mm

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.08	0.00	0.50	106.45	0.833	32.18	1.17	2.48
0.17	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

CALIB NASHYD ( 0153 )  
ID= 1 DT=10.0 min

Area (ha)	Curve Number (CN)	Area (mm)	# of Linear Res. (N)
91.47	69.0	22.82	3.00
U.H. Tp(hrs)		0.20	

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48
0.333	27.23	0.667	66.84	1.000	9.90		

Unit Hyd Opeak (cms) = 17.469  
PEAK FLOW (cms) = 1.212 (i)  
TIME TO PEAK (hrs) = 0.833  
RUNOFF VOLUME (mm) = 2.499  
TOTAL RAINFALL (mm) = 41.260  
RUNOFF COEFFICIENT = 0.661

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Total = 41.26 mm

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95
0.14	4.95	0.50	138.63	0.83	24.76		
0.25	14.85	0.58	74.27	0.92	14.85		
0.33	39.61	0.67	59.41	1.00	4.95		

VO output - Prop Controlled Upstream.txt  
 NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0  
 ID = 1 DT=10.0 min Ia (mm) = 21.77 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48		
0.333	27.23	0.667	66.84	1.000	9.90				

Unit Hyd Opeak (cms) = 17.302  
 PEAK FLOW (cms) = 1.391 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 2.883  
 TOTAL RAINFALL (mm) = 41.260  
 RUNOFF COEFFICIENT = 0.070

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
 1 + 2 = 3  
 AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
 ID=1 ( 0152): 90.60 1.391 0.833 2.883  
 + ID=2 ( 0153): 91.47 1.212 0.83 2.50  
 ID = 3 ( 0002): 182.07 2.603 0.83 2.69

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c9d32900  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME		RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95		
0.17	4.95	0.50	138.63	0.83	24.76				
0.25	14.85	0.58	74.27	0.92	14.85				
0.33	39.61	0.67	59.41	1.00	4.95				

VO output - Prop Controlled Upstream.txt  
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c9d32900  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME		RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95		
0.17	4.95	0.50	138.63	0.83	24.76				
0.25	14.85	0.58	74.27	0.92	14.85				
0.33	39.61	0.67	59.41	1.00	4.95				

CALIB STANDBYD ( 1421) Area (ha) = 15.00 Total Imp(%) = 30.00 Dir. Conn.(%) = 29.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 4.50 10.50  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 288.20 40.00  
 Mannings n = 0.015 0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c9d32900  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME		RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48		
0.333	27.23	0.667	66.84	1.000	9.90				

Max. Eff. Inten. (mm/hr) = 106.45 24.81  
 over (mi n) = 10.00 20.00  
 Storage Coeff. (mi n) = 4.80 (ii) 14.34 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit Hyd. peak (cms) = 0.15 0.07

PEAK FLOW (cms) = 1.16 0.50 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 0.83 0.50  
 RUNOFF VOLUME (mm) = 40.26 10.90 19.42  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.26 0.47

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 71.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c9d32900  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME		RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95		
0.17	4.95	0.50	138.63	0.83	24.76				
0.25	14.85	0.58	74.27	0.92	14.85				
0.33	39.61	0.67	59.41	1.00	4.95				

Max. Eff. Inten. (mm/hr) = 106.45 30.07  
 over (mi n) = 10.00 30.00  
 Storage Coeff. (mi n) = 9.45 (ii) 20.86 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 30.00  
 Unit Hyd. peak (cms) = 0.11 0.05

PEAK FLOW (cms) = 12.77 2.50 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 1.00 0.61  
 RUNOFF VOLUME (mm) = 39.26 11.24 24.97  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.95 0.27 0.61

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 69.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019) OVERFLOW IS OFF  
 IN = 2 ---> OUT = 1  
 DT = 10.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
INFLOW: ID = 2 ( 0022)	127.590	13.112	24.97
OUTFLOW: ID = 1 ( 0019)	127.590	0.237	2.00

PEAK FLOW REDUCTION [Oout/Oin](%) = 1.80  
 TIME SHIFT OF PEAK FLOW (mi n) = 80.00  
 MAXIMUM STORAGE USED (ha.m.) = 3.0501

ROUTE CHN ( 0026) Routing time step (mi n) = 10.00

DATA FOR SECTION ( 1.1 )  
 Distance Elevation Manning  
 0.00 274.12 0.0500  
 5.20 273.17 0.0300 Main Channel  
 10.58 272.11 0.0300 Main Channel  
 11.50 272.00 0.0300 Main Channel  
 56.67 272.00 0.0300 Main Channel  
 64.61 272.44 0.0300 Main Channel  
 68.47 272.73 0.0300 Main Channel  
 73.75 273.06 0.0300 / 0.0500 Main Channel  
 79.53 273.29 0.0500

TRAVEL TIME TABLE  
 DEPTH (m) ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV. TIME (mi n)  
 0.07 272.07 104E+04 0.9 0.29 19.48  
 0.13 272.13 21E+04 2.8 0.46 12.41  
 0.20 272.20 322E+04 5.6 0.59 9.56  
 0.26 272.26 436E+04 9.1 0.71 7.96  
 0.33 272.33 554E+04 13.3 0.82 6.93

VO output - Prop Controlled Upstream.txt  
 0.33 39.61 | 0.67 59.41 | 1.00 4.95 |

CALIB STANDBYD ( 0142) Area (ha) = 54.15 Total Imp(%) = 67.00 Dir. Conn.(%) = 64.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 36.28 17.87  
 Dep. Storage (mm) = 1.00 16.04  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 600.83 40.00  
 Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME		RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.48	0.500	106.45	0.833	32.18	1.17	2.48		
0.333	27.23	0.667	66.84	1.000	9.90				

Max. Eff. Inten. (mm/hr) = 106.45 17.47  
 over (mi n) = 10.00 30.00  
 Storage Coeff. (mi n) = 7.96 (ii) 30.52 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 30.00  
 Unit Hyd. peak (cms) = 0.12 0.05

PEAK FLOW (cms) = 7.79 0.48 \*TOTALS\*  
 TIME TO PEAK (hrs) = 0.50 1.00 0.50  
 RUNOFF VOLUME (mm) = 40.26 7.05 28.30  
 TOTAL RAINFALL (mm) = 41.26 41.26 41.26  
 RUNOFF COEFFICIENT = 0.98 0.17 0.69

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 76.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\c9d32900  
 Ptotal = 41.26 mm Comments: 10yr 1hr AES

TIME		RAIN		--- TRANSFORMED HYETOGRAPH ---		TIME		RAIN	
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	74.27	0.75	39.61	1.08	4.95		
0.17	4.95	0.50	138.63	0.83	24.76				
0.25	14.85	0.58	74.27	0.92	14.85				
0.33	39.61	0.67	59.41	1.00	4.95				

CALIB STANDBYD ( 0022) Area (ha) = 127.59 Total Imp(%) = 61.00 Dir. Conn.(%) = 49.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 77.83 49.76  
 Dep. Storage (mm) = 2.00 5.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 922.28 40.00  
 Mannings n = 0.013 0.250

VO output - Prop Controlled Upstream.txt							
	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
0.40	272.40	676E+04	18.2	0.91	6.19		
0.46	272.46	801E+04	23.7	1.01	5.62		
0.53	272.53	928E+04	30.0	1.09	5.16		
0.60	272.60	106E+05	36.8	1.18	4.80		
0.66	272.66	119E+05	44.2	1.26	4.49		
0.73	272.73	133E+05	52.2	1.33	4.24		
0.79	272.79	147E+05	60.7	1.40	4.03		
0.86	272.86	161E+05	69.7	1.47	3.84		
0.93	272.93	175E+05	79.3	1.54	3.68		
0.99	272.99	190E+05	89.5	1.60	3.54		
1.06	273.06	205E+05	100.3	1.66	3.41		
1.14	273.14	223E+05	114.9	1.74	3.24		
1.22	273.22	242E+05	130.7	1.83	3.09		
1.29	273.29	262E+05	147.5	1.91	2.96		

<----- hydrograph ----->							
	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 ( 0019)	127.59	0.24	2.00	24.95	0.02	0.29	
OUTFLOW: ID= 1 ( 0026)	127.59	0.23	2.67	24.95	0.02	0.29	

ADD HYD ( 0017)				
1 + 2 = 3	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0142):	54.15	7.818	0.50	28.30
+ ID2= 2 ( 0026):	127.59	0.234	2.67	24.95
ID= 3 ( 0017):	181.74	7.829	0.50	25.95

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN ( 0015)  
IN= 2--> OUT= 1 Routing time step (min)' = 10.00

<----- DATA FOR SECTION ( 1.1) ----->					
Distance	Elevation	Manning			
0.00	252.95	0.0500	/0.0300	Main Channel	
3.65	252.53	0.0500	/0.0300	Main Channel	
12.66	251.31	0.0300		Main Channel	
14.58	251.05	0.0300		Main Channel	
76.79	251.08	0.0300		Main Channel	
87.59	252.17	0.0300		Main Channel	
90.94	252.44	0.0300	/0.0500	Main Channel	
98.65	253.00	0.0500		Main Channel	

<----- TRAVEL TIME TABLE ----->							
DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME		
(m)	(m)	(cu.m.)	(Cms)	(m/s)	(min)		
0.10	251.15	671E+04	4.4	0.82	25.64		
0.20	251.20	148E+05	15.9	1.36	15.43		
0.30	251.35	230E+05	32.9	1.80	11.67		
0.40	251.45	315E+05	54.5	2.18	9.63		
0.49	251.55	402E+05	80.5	2.53	8.32		
0.59	251.64	491E+05	110.6	2.84	7.40		
0.69	251.74	582E+05	144.6	3.13	6.71		
0.79	251.84	676E+05	182.5	3.41	6.17		
0.89	251.94	771E+05	224.1	3.67	5.74		
0.99	252.04	869E+05	269.3	3.91	5.38		
1.09	252.14	969E+05	318.2	4.15	5.07		
1.19	252.24	107E+06	370.2	4.36	4.82		
1.29	252.34	118E+06	425.6	4.57	4.60		
1.39	252.44	128E+06	484.6	4.77	4.41		
1.49	252.54	140E+06	554.9	5.02	4.19		
1.59	252.64	151E+06	632.3	5.28	3.99		
1.69	252.74	162E+06	714.0	5.52	3.81		
1.80	252.85	176E+06	799.8	5.75	3.66		
1.90	252.95	188E+06	889.9	5.97	3.52		

<----- hydrograph -----> <-pi pe / channel ->  
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VO output - Prop Controlled Upstream.txt  
V V I SSSS U U A A A L  
V V I SSSS U U A A L L L L L L  
W I SSSSS UUUUU A A L L L L L L  
000 TTTT TTTT H H Y Y M H 000 TM  
0 0 T T T H H Y Y M M M 0 0  
0 0 T T T H H Y Y M M 0 0  
0 0 T T T H H Y Y M M 0 0  
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHM0 5.2\VO2\voi.n.dat  
Output filename: C:\Users\p001279d\AppData\Local\CVI\civica\WH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\87b1f4-ee-4f70-465e-9b07-9a1339cbf94d\scve  
Summary filename: C:\Users\p001279d\AppData\Local\CVI\civica\WH5\b8ccee0-2cc7-4973-8ab8-dd0de50b3a28\87b1f4-ee-4f70-465e-9b07-9a1339cbf94d\scve

DATE: 11/28/2019 TIME: 10:09:37

USER:

COMMENTS:

\*\*\* SIMULATION : 04 AES 25-Yr \*\*\*

READ STORM							
File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\54806dbf							
Ptotal = 49.17 mm Comments: 25yr 1hr AES							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB NASHYD ( 0153)			
ID= 1 DT=10.0 min	Area	(ha)	Curve Number (CN)= 69.0
	la	(mm)	# of Li near Res. (N)= 3.00
	U.H. Tp(hrs)		
	91.47	22.82	
	0.20		

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.95	0.500	126.86	0.833	38.35	1.17	2.95
0.333	32.45	0.667	79.66	1.000	11.80		

Unit Hyd Opeak (cms) = 17.469

VO output - Prop Controlled Upstream.txt							
	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL	
	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)	
INFLOW: ID= 2 ( 0017)	181.74	7.83	0.50	25.95	0.13	0.93	
OUTFLOW: ID= 1 ( 0015)	181.74	4.33	0.83	25.95	0.10	0.82	

RESERVOIR ( 0020)  
IN= 2--> OUT= 1  
DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
INFLOW: ID= 2 ( 0015)	181.740	4.326	0.83	25.95
OUTFLOW: ID= 1 ( 0020)	181.740	0.717	2.17	25.94

PEAK FLOW REDUCTION [Qout/Oin] (%) = 16.58  
TIME SHIF OF PEAK FLOW (min) = 80.00  
MAXIMUM STORAGE USED (ha.m.) = 1.2215

ADD HYD ( 0003)  
1 + 2 = 3

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 1421):	15.00	1.285	0.50	19.42
+ ID2= 2 ( 0151):	18.20	0.510	0.83	5.62
ID= 3 ( 0003):	33.20	1.662	0.67	11.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
3 + 2 = 1

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 3 ( 0003):	33.20	1.662	0.67	11.85
+ ID2= 2 ( 0002):	182.07	2.603	0.83	2.69
ID= 1 ( 0003):	215.27	4.056	0.83	4.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0003):	215.27	4.056	0.83	4.10
+ ID2= 2 ( 0002):	181.74	0.717	2.17	25.94
ID= 3 ( 0003):	397.01	4.325	0.83	14.10

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

FINISH

V V I SSSS U U A A L (v 5.2.2003)  
V V I SS U U A A L  
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VO output - Prop Controlled Upstream.txt  
PEAK FLOW (cms) = 2.334 (i)  
TIME TO PEAK (hrs) = 0.833  
RUNOFF VOLUME (mm) = 4.815  
TOTAL RAINFALL (mm) = 49.170  
RUNOFF COEFFICIENT = 0.098  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM							
File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\54806dbf							
Ptotal = 49.17 mm Comments: 25yr 1hr AES							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB NASHYD ( 0152)			
ID= 1 DT=10.0 min	Area	(ha)	Curve Number (CN)= 70.0
	la	(mm)	# of Li near Res. (N)= 3.00
	U.H. Tp(hrs)		
	90.60	21.77	
	0.20		

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.95	0.500	126.86	0.833	38.35	1.17	2.95
0.333	32.45	0.667	79.66	1.000	11.80		

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 2.564 (i)  
TIME TO PEAK (hrs) = 0.833  
RUNOFF VOLUME (mm) = 5.367  
TOTAL RAINFALL (mm) = 49.170  
RUNOFF COEFFICIENT = 0.109  
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
1 + 2 = 3

	AREA	OPEAK	TPEAK	R.V.
	(ha)	(cms)	(hrs)	(mm)
ID1= 1 ( 0152):	90.60	2.564	0.83	5.37
+ ID2= 2 ( 0153):	91.47	2.334	0.83	4.81
ID= 3 ( 0002):	182.07	4.898	0.83	5.09

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM

File name: C:\Users\p001279d\AppData\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\54806dbf							
Ptotal = 49.17 mm Comments: 25yr 1hr AES							
TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		



VO output - Prop Controlled Upstream.txt  
0.25 17.70 | 0.58 88.51 | 0.92 17.70  
0.33 47.20 | 0.67 70.80 | 1.00 5.90

CALIB  
STANDHYD ( 0151 ) Area (ha) = 18.20 Curve Number (CN) = 68.0  
ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Linear Res. (N) = 3.00  
Ua H. Tp (hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.167	2.95	0.500	126.86
0.333	32.45	0.667	79.66
		1.000	11.80

Unit Hyd Opeak (cms) = 3.476  
PEAK FLOW (cms) = 0.765 (i)  
TIME TO PEAK (hrs) = 0.833  
RUNOFF VOLUME (mm) = 8.608  
TOTAL RAINFALL (mm) = 49.170  
RUNOFF COEFFICIENT = 0.175

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\54806dbf  
Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB  
STANDHYD ( 0142 ) Area (ha) = 54.15  
ID= 1 DT=10.0 min Total Imp (%) = 67.00 Dir. Conn. (%) = 64.00

VO output - Prop Controlled Upstream.txt  
PEAK FLOW (cms) = 1.40 0.70 1.594 (iii)  
TIME TO PEAK (hrs) = 0.50 0.83 0.50  
RUNOFF VOLUME (mm) = 48.17 14.90 24.55  
TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.98 0.30 0.50

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\54806dbf  
Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	88.51	0.75	47.20	1.08	5.90
0.17	5.90	0.50	165.21	0.83	29.50		
0.25	17.70	0.58	88.51	0.92	17.70		
0.33	47.20	0.67	70.80	1.00	5.90		

CALIB  
STANDHYD ( 0142 ) Area (ha) = 54.15  
ID= 1 DT=10.0 min Total Imp (%) = 67.00 Dir. Conn. (%) = 64.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 36.28 17.87  
Dep. Storage (mm) = 1.00 16.04  
Average Slope (%) = 1.00 3.00  
Length (m) = 600.83 40.00  
Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.167	2.95	0.500	126.86
0.333	32.45	0.667	79.66
		1.000	11.80

Max. Eff. Inten. (mm/hr) = 126.86 33.60  
over (mi n) = 10.00 20.00  
Storage Coeff. (mi n) = 7.42 (ii) 17.92 (ii)  
Unit Hyd. Tpeak (mi n) = 10.00 20.00  
Unit Hyd. peak (cms) = 0.13 0.06  
PEAK FLOW (cms) = 9.58 0.86 \*TOTALS\*  
TIME TO PEAK (hrs) = 0.50 0.83 9.686  
RUNOFF VOLUME (mm) = 48.17 11.00 0.50  
TOTAL RAINFALL (mm) = 49.17 49.17 34.79  
RUNOFF COEFFICIENT = 0.98 0.22 0.71

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 76.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Controlled Upstream.txt

READ STORM File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\54806dbf  
Ptotal = 49.17 mm Comments: 25yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	2.95	0.500	126.86	0.833	38.35	1.17	2.95
0.333	32.45	0.667	79.66	1.000	11.80		

CALIB  
STANDHYD ( 0022 ) Area (ha) = 127.59  
ID= 1 DT=10.0 min Total Imp (%) = 61.00 Dir. Conn. (%) = 49.00

IMPERVIOUS PERVIOUS (i)  
Surface Area (ha) = 77.83 49.76  
Dep. Storage (mm) = 2.00 5.00  
Average Slope (%) = 1.00 2.00  
Length (m) = 922.28 40.00  
Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---			
TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr
0.167	2.95	0.500	126.86
0.333	32.45	0.667	79.66
		1.000	11.80

Max. Eff. Inten. (mm/hr) = 126.86 46.15  
over (mi n) = 10.00 20.00  
Storage Coeff. (mi n) = 8.81 (ii) 18.42 (ii)  
Unit Hyd. Tpeak (mi n) = 10.00 20.00  
Unit Hyd. peak (cms) = 0.12 0.06  
PEAK FLOW (cms) = 15.84 3.90 17.407 (iii)  
TIME TO PEAK (hrs) = 0.50 0.83 0.67  
RUNOFF VOLUME (mm) = 47.17 15.51 31.02  
TOTAL RAINFALL (mm) = 49.17 49.17 49.17  
RUNOFF COEFFICIENT = 0.96 0.32 0.63

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 69.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019 )  
IN= 2 -> OUT= 1  
DT= 10.0 min

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

INFLOW: ID= 2 ( 0022 )	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
	127.590	17.407	0.67	31.02
OUTFLOW: ID= 1 ( 0019 )	127.590	0.312	1.83	31.00

PEAK FLOW REDUCTION [Qout/Qi n] (%) = 1.79  
TIME SHIF T OF PEAK FLOW (mi n) = 70.00

VO output - Prop Controlled Upstream.txt  
MAXIMUM STORAGE USED (ha. m.) = 3.8043

ROUTE CHN( 0026 )  
IN= 2 -> OUT= 1 Routing time step (mi n)' = 10.00

DATA FOR SECTION ( 1.1 )  
Distance Elevation Manning  
0.00 274.12 0.0500  
5.20 273.17 0.0300  
10.58 272.11 0.0300  
11.50 272.00 0.0300  
56.67 272.00 0.0300  
64.61 272.44 0.0300  
68.47 272.73 0.0300  
73.75 273.06 0.0300 / 0.0500  
79.53 273.29 0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.07	272.07	104E+04	0.9	0.29	19.48
0.13	272.13	211E+04	2.8	0.46	12.41
0.20	272.20	322E+04	5.6	0.59	9.56
0.26	272.26	436E+04	9.1	0.71	7.96
0.33	272.33	554E+04	13.3	0.82	6.82
0.40	272.40	676E+04	18.2	0.91	6.19
0.46	272.46	801E+04	23.7	1.01	5.62
0.53	272.53	928E+04	30.0	1.09	5.16
0.60	272.60	106E+05	36.8	1.18	4.80
0.66	272.66	119E+05	44.2	1.26	4.49
0.73	272.73	133E+05	52.2	1.33	4.24
0.79	272.79	147E+05	60.7	1.40	4.03
0.86	272.86	161E+05	69.7	1.47	3.84
0.93	272.93	175E+05	79.3	1.54	3.68
0.99	272.99	190E+05	89.5	1.60	3.54
1.06	273.06	205E+05	100.3	1.66	3.41
1.14	273.14	223E+05	114.9	1.74	3.24
1.22	273.22	242E+05	130.7	1.83	3.09
1.29	273.29	262E+05	147.5	1.91	2.96

INFLOW: ID= 2 ( 0019 ) 127.59 0.31 2.50 31.00  
OUTFLOW: ID= 1 ( 0026 ) 127.59 0.31 2.50 31.00

ADD HYD ( 0017 )  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
54.15	9.686	0.50	34.79
127.59	0.308	2.50	31.00

ID= 3 ( 0017 ) 181.74 9.705 1.50 32.13  
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN( 0015 )  
IN= 2 -> OUT= 1 Routing time step (mi n)' = 10.00

DATA FOR SECTION ( 1.1 )  
Distance Elevation Manning  
0.00 252.95 0.0500  
3.65 252.53 0.0500 / 0.0300  
12.66 251.31 0.0300  
14.58 251.05 0.0300  
76.79 251.08 0.0300  
87.59 252.17 0.0300

VO output - Prop Controlled Upstream.txt  
 90.94 252.44 0.0300 / 0.0500 Main Channel  
 98.65 253.00 0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TRAVEL TIME (min)	TABLE FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.10	251.15	671E+04	4.4	0.82	25.64	
0.20	251.25	148E+05	15.9	1.36	15.43	
0.30	251.35	230E+05	32.9	1.80	11.67	
0.40	251.45	315E+05	54.5	2.18	9.63	
0.49	251.55	402E+05	80.5	2.53	8.32	
0.59	251.64	491E+05	110.6	2.84	7.40	
0.69	251.74	582E+05	144.6	3.13	6.71	
0.79	251.84	676E+05	182.5	3.41	6.17	
0.89	251.94	771E+05	224.1	3.67	5.74	
0.99	252.04	869E+05	269.3	3.91	5.38	
1.09	252.14	969E+05	318.2	4.15	5.07	
1.19	252.24	107E+06	370.2	4.36	4.82	
1.29	252.34	118E+06	425.6	4.57	4.60	
1.39	252.44	128E+06	484.6	4.77	4.41	
1.49	252.54	140E+06	554.9	5.02	4.19	
1.59	252.64	151E+06	632.3	5.28	3.99	
1.69	252.75	163E+06	714.0	5.52	3.81	
1.80	252.85	176E+06	799.8	5.75	3.66	
1.90	252.95	188E+06	889.9	5.97	3.52	

AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL  
 (ha) (cms) (hrs) (mm) (m) (m/s)  
 INFLOW : ID= 2 ( 0017) 181.74 9.70 0.50 32.13 0.14 0.85  
 OUTFLOW : ID= 1 ( 0015) 181.74 5.53 0.83 32.13 0.11 0.85

RESERVOIR ( 0020)  
 IN= 2 --> OUT= 1  
 DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.9900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 INFLOW : ID= 2 ( 0015) 181.740 5.532 0.83 32.13  
 OUTFLOW : ID= 1 ( 0020) 181.740 0.996 2.00 32.13

PEAK FLOW REDUCTION [Qout/Oin] (%) = 18.01  
 TIME SHIFT OF PEAK FLOW (min) = 70.00  
 MAXIMUM STORAGE USED (ha. m.) = 1.4990

ADD HYD ( 0003)  
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 ( 1421):	15.00	1.594	0.50	24.55
+ ID2= 2 ( 0151):	18.20	0.765	0.83	8.61
ID= 3 ( 0003):	33.20	2.250	0.67	15.81

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 3 + 2 = 1

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 3 ( 0003):	33.20	2.250	0.67	15.81
+ ID2= 2 ( 0002):	182.07	4.898	0.83	5.09

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CALIB NASHYD ( 0153)  
 ID= 1 DT=10.0 min

VO output - Prop Controlled Upstream.txt  
 0.25 19.78 0.58 98.91 0.92 19.78  
 0.33 52.75 0.67 79.13 1.00 6.59

Area (ha) = 91.47 Curve Number (CN) = 69.0  
 la (mm) = 22.82 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	--- TRANSFORMED --- hrs	RAIN mm/hr	--- HYETOGRAPH --- TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30
0.333	36.27	0.667	89.02	1.000	13.19		

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 3.288 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 6.875  
 TOTAL RAINFALL (mm) = 54.950  
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\2cfb217d  
 Comments: 50yr 1hr AES

TIME hrs	RAIN mm/hr	--- TRANSFORMED --- TIME hrs	RAIN mm/hr	--- HYETOGRAPH --- TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 3.288 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 6.875  
 TOTAL RAINFALL (mm) = 54.950  
 RUNOFF COEFFICIENT = 0.125

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0152)  
 ID= 1 DT=10.0 min

Area (ha) = 90.60 Curve Number (CN) = 70.0  
 la (mm) = 21.77 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	--- TRANSFORMED --- TIME hrs	RAIN mm/hr	--- HYETOGRAPH --- TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30
0.333	36.27	0.667	89.02	1.000	13.19		

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 3.552 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 7.549  
 TOTAL RAINFALL (mm) = 54.950  
 RUNOFF COEFFICIENT = 0.137

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Controlled Upstream.txt

ID = 1 ( 0003): 215.27 6.888 0.83 6.74

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 1 + 2 = 3

ID	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID= 1 ( 0003):	215.27	6.888	0.83	6.74
+ ID2= 2 ( 0020):	181.74	0.996	2.00	32.13
ID= 3 ( 0003):	397.01	7.231	0.83	18.36

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 5.2.2003)  
 V V I SS U U A A A A L  
 V V I SS U U A A L L L L L  
 V V I SSSS U U U U A M L L L L L

000	TTTT	TTTT	H	Y	M	M	000	TM
0 0 T T H H Y Y M M 0 0 0								
0 0 T T H H Y Y M M 0 0 0								
000 T T H H Y Y M M 000								

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\*\*\*\*\* D E T A I L E D O U T P U T \*\*\*\*\*

Input file name: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO\vo\in.dat

Output file name:  
 C:\Users\p001279d\AppData\Local\Civica\VHS\b\c\cece0-2cc7-4973-8ab8-dd0de50b3a28\5f3870b1-6d35-4f9a-9930-459450a38781\scce

Summary file name:  
 C:\Users\p001279d\AppData\Local\Civica\VHS\b\c\cece0-2cc7-4973-8ab8-dd0de50b3a28\5f3870b1-6d35-4f9a-9930-459450a38781\scce

DATE: 11/28/2019 TIME: 10:09:36

USER:

COMMENTS:

\*\*\* SIMULATION : 05 AES 50-Yr \*\*\*

READ STORM  
 Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\2cfb217d  
 Comments: 50yr 1hr AES

TIME hrs	RAIN mm/hr	--- TRANSFORMED --- TIME hrs	RAIN mm/hr	--- HYETOGRAPH --- TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

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VO output - Prop Controlled Upstream.txt

ID = 1 ( 0002): 182.07 6.841 0.83 7.21

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
 Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\2cfb217d  
 Comments: 50yr 1hr AES

TIME hrs	RAIN mm/hr	--- TRANSFORMED --- TIME hrs	RAIN mm/hr	--- HYETOGRAPH --- TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

Unit Hyd Opeak (cms) = 3.476

PEAK FLOW (cms) = 0.972 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 11.081  
 TOTAL RAINFALL (mm) = 54.950  
 RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB NASHYD ( 0151)  
 ID= 1 DT=10.0 min

Area (ha) = 18.20 Curve Number (CN) = 68.0  
 la (mm) = 11.95 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	--- TRANSFORMED --- TIME hrs	RAIN mm/hr	--- HYETOGRAPH --- TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30
0.333	36.27	0.667	89.02	1.000	13.19		

Unit Hyd Opeak (cms) = 3.476

PEAK FLOW (cms) = 0.972 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 11.081  
 TOTAL RAINFALL (mm) = 54.950  
 RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
 Ptotal = 54.95 mm

File name: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\2cfb217d  
 Comments: 50yr 1hr AES

TIME hrs	RAIN mm/hr	--- TRANSFORMED --- TIME hrs	RAIN mm/hr	--- HYETOGRAPH --- TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

Unit Hyd Opeak (cms) = 3.476

PEAK FLOW (cms) = 0.972 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 11.081  
 TOTAL RAINFALL (mm) = 54.950  
 RUNOFF COEFFICIENT = 0.202

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CALIB STANDHYD ( 1421)  
 ID= 1 DT=10.0 min

Area (ha) = 15.00 Curve Number (CN) = 30.0  
 Total Imp(%) = 30.00 Dir. Conn. (%) = 29.00

VO output - Prop Controlled Upstream.txt

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 4.50 10.50  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 258.20 40.00  
 Mannings n = 0.015 0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30
0.333	36.27	0.667	89.02	1.000	13.19		

Max. Eff. Inten. (mm/hr) = 141.77 40.51  
 over (mi n) = 10.00 20.00  
 Storage Coeff. (mi n) = 4.28 (ii) 12.12 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit Hyd. peak (cms) = 0.15 0.07

PEAK FLOW (cms) = 1.58 0.87 1.851 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.83 0.67  
 RUNOFF VOLUME (mm) = 53.95 18.08 28.48  
 TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
 RUNOFF COEFFICIENT = 0.98 0.33 0.52

\*TOTALS\*

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR Pervious Losses:  
 CN\* = 71.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData  
 ata\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\2cfb217d  
 Ptotal = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB STANDHYD ( 0142) Area (ha) = 54.15  
 ID= 1 DT=10.0 min Total Imp(%) = 67.00 Dir. Conn.(%) = 64.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 36.28 17.87  
 Dep. Storage (mm) = 1.00 16.04  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 600.83 40.00  
 Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30
0.333	36.27	0.667	89.02	1.000	13.19		

VO output - Prop Controlled Upstream.txt

(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019)  
 IN= 2--> OUT= 1  
 DT= 10.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

INFLOW	AREA	OPEAK	TPEAK	R.V.
ID= 2 ( 0022)	(ha)	(cms)	(hrs)	(mm)
127.590	127.590	20.032	0.67	35.58
127.590	127.590	0.340	1.83	35.56

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 1.70  
 TIME SHIFT OF PEAK FLOW (mi n) = 70.00  
 MAXIMUM STORAGE USED (ha.m.) = 4.3713

ROUTE CHN( 0026)  
 IN= 2--> OUT= 1

Routing time step (mi n)' = 10.00

Distance	Elevation	Manning
0.00	274.12	0.0500
5.20	273.17	0.0300
10.58	272.11	0.0300
11.50	272.00	0.0300
56.67	272.00	0.0300
64.61	272.44	0.0300
68.47	272.73	0.0300
73.75	273.06	0.0300 / 0.0500
79.53	273.29	0.0500

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(mi n)
0.07	272.07	1.04E+04	0.9	0.29	19.48
0.13	272.13	2.11E+04	2.8	0.46	12.41
0.20	272.20	3.22E+04	5.6	0.59	9.56
0.26	272.26	4.36E+04	9.1	0.71	7.96
0.33	272.33	5.54E+04	13.3	0.82	6.93
0.40	272.40	6.76E+04	18.2	0.91	6.19
0.46	272.46	8.01E+04	23.7	1.01	5.62
0.53	272.53	9.28E+04	30.0	1.09	5.16
0.60	272.60	1.06E+05	36.8	1.18	4.80
0.66	272.66	1.19E+05	44.2	1.26	4.49
0.73	272.73	1.33E+05	52.2	1.33	4.24
0.79	272.79	1.47E+05	60.7	1.40	4.03
0.86	272.86	1.61E+05	69.7	1.47	3.84
0.93	272.93	1.75E+05	79.3	1.54	3.68
0.99	272.99	1.90E+05	89.5	1.60	3.54
1.06	273.06	2.05E+05	100.3	1.66	3.41
1.14	273.14	2.23E+05	114.9	1.74	3.24
1.22	273.22	2.42E+05	130.7	1.83	3.09
1.29	273.29	2.62E+05	147.5	1.91	2.96

INFLOW	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
ID= 2 ( 0019)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
127.59	127.59	0.34	1.83	35.56	0.03	0.29
127.59	127.59	0.34	2.67	35.56	0.03	0.29

VO output - Prop Controlled Upstream.txt

Max. Eff. Inten. (mm/hr) = 141.77 40.36  
 over (mi n) = 10.00 20.00  
 Storage Coeff. (mi n) = 7.10 (ii) 16.08 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit Hyd. peak (cms) = 0.13 0.06

PEAK FLOW (cms) = 10.91 1.16 11.073 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.83 0.50  
 RUNOFF VOLUME (mm) = 53.95 14.24 39.65  
 TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
 RUNOFF COEFFICIENT = 0.98 0.26 0.72

\*TOTALS\*

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR Pervious Losses:  
 CN\* = 76.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM Filename: C:\Users\p001279d\AppData  
 ata\Local\Temp\FB082F85-bf5b-44d7-8765-aa036ef20a94\2cfb217d  
 Ptotal = 54.95 mm Comments: 50yr 1hr AES

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.08	0.00	0.42	98.91	0.75	52.75	1.08	6.59
0.17	6.59	0.50	184.63	0.83	32.97		
0.25	19.78	0.58	98.91	0.92	19.78		
0.33	52.75	0.67	79.13	1.00	6.59		

CALIB STANDHYD ( 0022) Area (ha) = 127.59  
 ID= 1 DT=10.0 min Total Imp(%) = 61.00 Dir. Conn.(%) = 49.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 77.83 49.76  
 Dep. Storage (mm) = 2.00 5.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 922.28 40.00  
 Mannings n = 0.013 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.167	3.30	0.500	141.77	0.833	42.86	1.17	3.30
0.333	36.27	0.667	89.02	1.000	13.19		

Max. Eff. Inten. (mm/hr) = 141.77 55.72  
 over (mi n) = 10.00 20.00  
 Storage Coeff. (mi n) = 8.43 (ii) 17.34 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit Hyd. peak (cms) = 0.12 0.06

\*TOTALS\*

PEAK FLOW (cms) = 18.12 4.89 20.032 (iii)  
 TIME TO PEAK (hrs) = 0.50 0.83 0.67  
 RUNOFF VOLUME (mm) = 52.95 18.89 35.58  
 TOTAL RAINFALL (mm) = 54.95 54.95 54.95  
 RUNOFF COEFFICIENT = 0.96 0.34 0.65

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR Pervious Losses:  
 CN\* = 69.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Controlled Upstream.txt

ADD HYD ( 0017)	AREA	OPEAK	TPEAK	R.V.
ID= 1 ( 0142)	(ha)	(cms)	(hrs)	(mm)
54.15	11.073	0.50	39.65	
127.59	0.337	2.67	35.56	
ID= 3 ( 0017)	181.74	11.090	5.50	36.78

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN( 0015) Routing time step (mi n)' = 10.00

Distance	Elevation	Manning
0.00	252.95	0.0500
3.65	252.53	0.0500 / 0.0300
12.66	251.31	0.0300
14.58	251.05	0.0300
76.79	251.08	0.0300
87.59	252.17	0.0300
90.94	252.44	0.0300 / 0.0500
98.65	253.00	0.0500

DEPTH	ELEV	VOLUME	FLOW RATE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(m/s)	(mi n)
0.10	251.15	671E+04	4.4	0.82	25.64
0.20	251.25	148E+05	15.9	1.36	15.43
0.30	251.35	230E+05	32.9	1.80	11.67
0.40	251.45	315E+05	54.5	2.18	9.63
0.49	251.55	402E+05	80.5	2.53	8.32
0.59	251.64	491E+05	110.6	2.84	7.40
0.69	251.74	582E+05	144.6	3.13	6.71
0.79	251.84	676E+05	182.5	3.41	6.17
0.89	251.94	771E+05	224.1	3.67	5.74
0.99	252.04	869E+05	269.3	3.91	5.38
1.09	252.14	969E+05	318.2	4.15	5.07
1.19	252.24	107E+06	370.2	4.36	4.82
1.29	252.34	118E+06	425.6	4.57	4.60
1.39	252.44	128E+06	484.6	4.77	4.41
1.49	252.54	140E+06	554.9	5.02	4.19
1.59	252.64	151E+06	632.3	5.28	3.99
1.69	252.75	163E+06	714.0	5.52	3.81
1.80	252.85	176E+06	799.8	5.75	3.66
1.90	252.95	188E+06	889.9	5.97	3.52

INFLOW	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
ID= 2 ( 0017)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
181.74	11.09	0.50	36.78	0.16	1.07	
181.74	6.44	0.83	36.78	0.12	0.88	

RESERVOIR ( 0020)  
 IN= 2--> OUT= 1  
 DT= 10.0 min

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(cms)	(ha.m.)	(cms)	(ha.m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

INFLOW	AREA	OPEAK	TPEAK	R.V.
ID= 2 ( 0015)	(ha)	(cms)	(hrs)	(mm)
181.740	181.740	6.437	0.83	36.78
181.740	181.740	1.418	1.83	36.77

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 22.04  
 TIME SHIFT OF PEAK FLOW (mi n) = 60.00

COMMENTS: \_\_\_\_\_

ADD HYD ( 0003 )  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 1421):	15.00	1.851	0.67	28.48
+ ID2= 2 ( 0151):	18.20	0.972	0.83	11.08
ID = 3 ( 0003):	33.20	2.725	0.67	18.94

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )  
3 + 2 = 1

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 3 ( 0003):	33.20	2.725	0.67	18.94
+ ID2= 2 ( 0002):	182.07	6.841	0.83	7.21
ID = 1 ( 0003):	215.27	9.259	0.83	9.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0003):	215.27	9.259	0.83	9.02
+ ID2= 2 ( 0020):	181.74	1.418	1.83	36.77
ID = 3 ( 0003):	397.01	9.660	0.83	21.73

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

```
V V I SSSSS U U A L (v 5.2.2003)
V V I SS U U A A A L
V V I SS U U A A L
V V I SSSSS UUUU A A LLLLL
000 TTTT TTTT H H Y Y M M O O TM
0 0 T T H H Y Y M M 0 0
0 0 T T H H Y Y M M 0 0
000 T T H H Y Y M M O O
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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voin.dat  
Output filename:  
C:\Users\p001279d\AppData\Local\Ci vi ca\VHS\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\3051a893-0488-4c10-b754-bf2707b99274\scve  
Summary filename:  
C:\Users\p001279d\AppData\Local\Ci vi ca\VHS\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\3051a893-0488-4c10-b754-bf2707b99274\scve

DATE: 11/28/2019 TIME: 10: 09: 35

USER:

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\8e50d755  
Ptotal = 60.87 mm  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB NASHYD ( 0153 )  
ID= 1 DT=10.0 min  
Area (ha)= 91.47 Curve Number (CN)= 69.0  
Ia (mm)= 22.82 # of Li near Res. (N)= 3.00  
U.H. Tp (hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	3.65	0.500	157.04	0.833	47.48	1.17	3.65
0.333	40.17	0.667	98.61	1.000	14.61		

Unit Hyd Opeak (cms)= 17.469

PEAK FLOW (cms)= 4.367 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 9.267  
TOTAL RAINFALL (mm)= 60.870  
RUNOFF COEFFICIENT = 0.152

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\8e50d755  
Ptotal = 60.87 mm  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB NASHYD ( 0152 )  
ID= 1 DT=10.0 min  
Area (ha)= 90.60 Curve Number (CN)= 70.0  
Ia (mm)= 21.77 # of Linear Res. (N)= 3.00  
U.H. Tp (hrs)= 0.20  
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VO output - Prop Controlled Upstream.txt  
NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	3.65	0.500	157.04	0.833	47.48	1.17	3.65
0.333	40.17	0.667	98.61	1.000	14.61		

Unit Hyd Opeak (cms)= 17.302

PEAK FLOW (cms)= 4.664 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 10.064  
TOTAL RAINFALL (mm)= 60.870  
RUNOFF COEFFICIENT = 0.165

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002 )  
1 + 2 = 3

	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
ID1= 1 ( 0152):	90.80	4.664	0.83	10.06
+ ID2= 2 ( 0153):	91.47	4.367	0.83	9.27
ID = 3 ( 0002):	182.07	9.030	0.83	9.66

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\8e50d755  
Ptotal = 60.87 mm  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB NASHYD ( 0151 )  
ID= 1 DT=10.0 min  
Area (ha)= 18.20 Curve Number (CN)= 68.0  
Ia (mm)= 11.95 # of Li near Res. (N)= 3.00  
U.H. Tp (hrs)= 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	3.65	0.500	157.04	0.833	47.48	1.17	3.65
0.333	40.17	0.667	98.61	1.000	14.61		

Unit Hyd Opeak (cms)= 3.476

PEAK FLOW (cms)= 1.201 (i)  
TIME TO PEAK (hrs)= 0.833  
RUNOFF VOLUME (mm)= 13.838  
TOTAL RAINFALL (mm)= 60.870  
RUNOFF COEFFICIENT = 0.227

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Controlled Upstream.txt

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\8e50d755  
Ptotal = 60.87 mm  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		

CALIB STANDHYD ( 1421 )  
ID= 1 DT=10.0 min  
Area (ha)= 15.00 Curve Number (CN)= 29.00  
Ia (mm)= 4.50 # of Linear Res. (N)= 3.00  
U.H. Tp (hrs)= 0.20  
Impervious (%) = 30.00  
Dir. Conn. (%) = 29.00

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	3.65	0.500	157.04	0.833	47.48	1.17	3.65
0.333	40.17	0.667	98.61	1.000	14.61		

Max. Eff. Inten. (mm/hr) over (min) = 157.04 / 10.00 = 15.70  
Storage Coeff. (min) = 4.11 (ii)  
Unit Hyd. Tpeak (min) = 10.00  
Unit Hyd. peak (cms) = 0.16  
PEAK FLOW (cms)= 1.77 1.05 \*TOTALS\*  
TIME TO PEAK (hrs)= 0.50 0.83 2.140 (iii)  
RUNOFF VOLUME (mm)= 59.87 21.52 32.64  
TOTAL RAINFALL (mm)= 60.87 60.87 60.87  
RUNOFF COEFFICIENT = 0.98 0.35 0.54

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 71.0 Ia = Dep. Storage (Above)
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM  
Filename: C:\Users\p001279d\AppData\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\8e50d755  
Ptotal = 60.87 mm  
Comments: 100yr 1hr AES

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	109.57	0.75	58.44	1.08	7.30
0.17	7.30	0.50	204.52	0.83	36.52		
0.25	21.91	0.58	109.57	0.92	21.91		
0.33	58.44	0.67	87.65	1.00	7.30		



\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYM0 5.2\VO2\vojn.dat  
 Output filename:  
 C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1  
 4f-e5e0-497e-8cdf-1b53b78b5c2c\vcie  
 Summary filename:  
 C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1  
 4f-e5e0-497e-8cdf-1b53b78b5c2c\vcie

DATE: 11/28/2019 TIME: 10:09:36  
 USER:

COMMENTS:

\*\*\*\*\* SIMULATION : 07 AES 250-Yr \*\*\*\*\*

READ STORM		File name: C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1	
Ptotal = 70.05 mm		Comments: 250yr 1hr AES	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	126.09
0.17	8.41	0.50	235.37
0.25	25.22	0.58	126.09
0.33	67.25	0.67	100.87

CALIB NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0  
 ID= 1 DT=10.0 min la (mm) = 22.82 # of Li near Res. (N) = 3.00  
 U. H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---		---	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	4.20	0.500	180.73
0.333	46.23	0.667	113.48

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 6.211 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 13.466  
 TOTAL RAINFALL (mm) = 70.050  
 RUNOFF COEFFICIENT = 0.192

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM		File name: C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1	
Ptotal = 70.05 mm		Comments: 250yr 1hr AES	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	126.09
0.17	8.41	0.50	235.37
0.25	25.22	0.58	126.09
0.33	67.25	0.67	100.87

CALIB NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0  
 ID= 1 DT=10.0 min la (mm) = 21.77 # of Li near Res. (N) = 3.00  
 U. H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---		---	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	4.20	0.500	180.73
0.333	46.23	0.667	113.48

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 6.554 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 14.448  
 TOTAL RAINFALL (mm) = 70.050  
 RUNOFF COEFFICIENT = 0.206

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)		AREA (ha)		OPEAK (cms)		TPEAK (hrs)		R. V. (mm)	
1 + 2 = 3									
ID1 = 1 ( 0152):		90.60	6.554	0.83	14.45				
+ ID2 = 2 ( 0153):		91.47	6.211	0.83	13.47				
ID = 3 ( 0002):		182.07	12.764	0.83	13.95				

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

READ STORM		File name: C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1	
Ptotal = 70.05 mm		Comments: 250yr 1hr AES	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	126.09
0.17	8.41	0.50	235.37
0.25	25.22	0.58	126.09
0.33	67.25	0.67	100.87

CALIB NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0  
 ID= 1 DT=10.0 min

VO output - Prop Controlled Upstream.txt  
 ID= 1 DT=10.0 min la (mm) = 11.95 # of Li near Res. (N) = 3.00  
 U. H. Tp(hrs) = 0.20

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---		---	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	4.20	0.500	180.73
0.333	46.23	0.667	113.48

Unit Hyd Opeak (cms) = 3.476

PEAK FLOW (cms) = 1.585 (i)  
 TIME TO PEAK (hrs) = 0.833  
 RUNOFF VOLUME (mm) = 18.510  
 TOTAL RAINFALL (mm) = 70.050  
 RUNOFF COEFFICIENT = 0.264

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM		File name: C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1	
Ptotal = 70.05 mm		Comments: 250yr 1hr AES	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	126.09
0.17	8.41	0.50	235.37
0.25	25.22	0.58	126.09
0.33	67.25	0.67	100.87

CALIB STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00  
 ID= 1 DT=10.0 min Total Imp(%) = 30.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 4.50 10.50  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 258.20 40.00  
 Mannings n = 0.015 0.200

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---		---	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	4.20	0.500	180.73
0.333	46.23	0.667	113.48

Max. Eff. Inten. (mm/hr) = 180.73 60.12  
 over (min) = 10.00 20.00  
 Storage Coeff. (mi n) = 3.88 (ii) 10.58 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit t Hyd. peak (cms) = 0.16 0.08  
 PEAK FLOW (cms) = 2.06 1.35  
 TIME TO PEAK (hrs) = 0.50 0.83  
 RUNOFF VOLUME (mm) = 69.05 27.20  
 TOTAL RAINFALL (mm) = 70.05 70.05  
 RUNOFF COEFFICIENT = 0.99 0.39

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

VO output - Prop Controlled Upstream.txt  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 70.0 la = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM		File name: C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1	
Ptotal = 70.05 mm		Comments: 250yr 1hr AES	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	126.09
0.17	8.41	0.50	235.37
0.25	25.22	0.58	126.09
0.33	67.25	0.67	100.87

CALIB STANDHYD ( 0142) Area (ha) = 54.15 Dir. Conn. (%) = 64.00  
 ID= 1 DT=10.0 min Total Imp(%) = 67.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 36.28 17.87  
 Dep. Storage (mm) = 1.00 16.04  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 600.83 40.00  
 Mannings n = 0.015 0.250

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

--- TRANSFORMED HYETOGRAPH ---		---	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	4.20	0.500	180.73
0.333	46.23	0.667	113.48

Max. Eff. Inten. (mm/hr) = 180.73 64.67  
 over (min) = 10.00 20.00  
 Storage Coeff. (mi n) = 6.44 (ii) 13.88 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit t Hyd. peak (cms) = 0.13 0.07  
 PEAK FLOW (cms) = 14.43 2.07  
 TIME TO PEAK (hrs) = 0.50 0.83  
 RUNOFF VOLUME (mm) = 69.05 27.20  
 TOTAL RAINFALL (mm) = 70.05 70.05  
 RUNOFF COEFFICIENT = 0.99 0.34

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN = 70.0 la = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

READ STORM		File name: C:\Users\p001279d\AppData\Local\Temp\ata\Local\Temp\fb082f85-bf5b-44d7-8765-aa036ef20a94\3a4314b1	
Ptotal = 70.05 mm		Comments: 250yr 1hr AES	
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.08	0.00	0.42	126.09
0.17	8.41	0.50	235.37
0.25	25.22	0.58	126.09
0.33	67.25	0.67	100.87

VO output - Prop Controlled Upstream.txt								
0.08	0.00	0.42	126.09	0.75	67.25	1.08	8.41	
0.17	8.41	0.50	235.37	0.83	42.03			
0.25	25.22	0.58	126.09	0.92	25.22			
0.33	67.25	0.67	100.87	1.00	8.41			

CALIB STANDBYD ( 0022)  
ID= 1 DT=10.0 min

Area (ha) = 127.59  
Total Imp(%) = 61.00  
Dir. Conn. (%) = 49.00

IMPERVIOUS PERVIOUS (I)

Surface Area (ha) = 77.83  
Dep. Storage (mm) = 2.00  
Average Slope (%) = 1.00  
Length (m) = 922.28  
Manning n = 0.013

NOTE: RAINFALL WAS TRANSFORMED TO 10.0 MIN. TIME STEP.

TRANSFORMED HYETOGRAPH					
TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.167	4.20	0.500	180.73	0.833	54.64
0.333	46.23	0.667	113.48	1.000	16.81

Max. Eff. Inten. (mm/hr) = 180.73  
Storage Coeff. (mi n) = 7.65 (ii)  
Unit Hyd. Tpeak (mi n) = 10.00  
Unit Hyd. peak (cms) = 0.12

PEAK FLOW (cms) = 24.22

TIME TO PEAK (hrs) = 0.50

RUNOFF VOLUME (mm) = 68.05

TOTAL RAINFALL (mm) = 70.05

RUNOFF COEFFICIENT = 0.97

RESERVOIR ( 0019)  
IN= 2--> OUT= 1  
DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

INFLOW: ID= 2 ( 0022)  
OUTFLOW: ID= 1 ( 0019)

PEAK FLOW REDUCTION [Qout/Oin] (%) = 1.37  
TIME SHIFT OF PEAK FLOW (mi n) = 70.00  
MAXIMUM STORAGE USED (ha. m.) = 5.9263

ROUTE CHN ( 0026)  
IN= 2--> OUT= 1

Routing time step (mi n) = 10.00

DATA FOR SECTION ( 1.1)  
Page 49

VO output - Prop Controlled Upstream.txt					
Distance	Elevation	Manning			
0.00	274.12	0.0500			
5.20	273.17	0.0300	Main Channel		
11.50	272.11	0.0300	Main Channel		
56.67	272.00	0.0300	Main Channel		
64.61	272.44	0.0300	Main Channel		
68.47	272.73	0.0300	Main Channel		
73.75	273.06	0.0300 / 0.0500	Main Channel		
79.53	273.29	0.0500			

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TIME TABLE FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.07	272.07	104E+04	0.9	0.29	19.48
0.13	272.13	211E+04	2.8	0.46	12.41
0.20	272.20	322E+04	5.6	0.59	9.56
0.26	272.26	436E+04	9.1	0.71	7.96
0.33	272.33	554E+04	13.3	0.82	6.83
0.40	272.40	676E+04	18.2	0.91	6.19
0.46	272.46	801E+04	23.7	1.01	5.62
0.53	272.53	928E+04	30.0	1.09	5.16
0.60	272.60	106E+05	36.8	1.18	4.80
0.66	272.66	119E+05	44.2	1.26	4.49
0.73	272.73	133E+05	52.2	1.33	4.24
0.79	272.79	147E+05	60.7	1.40	4.03
0.86	272.86	161E+05	69.7	1.47	3.84
0.93	272.93	175E+05	79.3	1.54	3.68
0.99	272.99	190E+05	89.5	1.60	3.54
1.06	273.06	205E+05	100.3	1.66	3.41
1.14	273.14	223E+05	114.9	1.74	3.24
1.22	273.22	242E+05	130.7	1.83	3.09
1.29	273.29	262E+05	147.5	1.91	2.96

INFLOW: ID= 2 ( 0019)  
OUTFLOW: ID= 1 ( 0026)

ADD HYD ( 0017)  
1 + 2 = 3

AREA OPEAK TPEAK R.V.  
ID= 1 ( 0142): 54.15 14.815 2.67 52.75  
ID= 2 ( 0026): 127.59 0.371 2.67 47.90  
ID= 3 ( 0017): 181.74 14.839 0.50 49.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN ( 0015)  
IN= 2--> OUT= 1

Routing time step (mi n) = 10.00

DATA FOR SECTION ( 1.1)					
Distance	Elevation	Manning			
0.00	252.95	0.0500			
3.65	252.53	0.0500 / 0.0300	Main Channel		
12.66	251.31	0.0300	Main Channel		
14.58	251.05	0.0300	Main Channel		
76.79	251.08	0.0300	Main Channel		
87.59	252.17	0.0300	Main Channel		
90.94	252.44	0.0300 / 0.0500	Main Channel		
98.65	253.00	0.0500			

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	TIME TABLE FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (mi n)
0.10	251.15	671E+04	4.4	0.82	25.64
0.20	251.25	148E+05	15.9	1.36	15.43

VO output - Prop Controlled Upstream.txt						
0.30	251.35	230E+05	32.9	1.80	11.67	
0.40	251.40	315E+05	54.5	2.18	9.63	
0.49	251.55	402E+05	80.5	2.53	8.32	
0.59	251.64	491E+05	110.6	2.84	7.40	
0.69	251.74	582E+05	144.6	3.13	6.71	
0.79	251.84	676E+05	182.5	3.41	6.17	
0.89	251.94	771E+05	224.1	3.67	5.74	
0.99	252.04	869E+05	269.3	3.91	5.38	
1.09	252.14	969E+05	316.2	4.15	5.07	
1.19	252.24	107E+06	370.2	4.36	4.82	
1.29	252.34	118E+06	425.6	4.57	4.60	
1.39	252.44	128E+06	484.6	4.77	4.41	
1.49	252.54	140E+06	554.9	5.02	4.19	
1.59	252.64	151E+06	632.3	5.28	3.99	
1.69	252.75	163E+06	714.0	5.52	3.81	
1.80	252.85	176E+06	799.8	5.75	3.66	
1.90	252.95	188E+06	889.9	5.97	3.52	

INFLOW: ID= 2 ( 0017)  
OUTFLOW: ID= 1 ( 0015)

PEAK FLOW REDUCTION [Qout/Oin] (%) = 39.24  
TIME SHIFT OF PEAK FLOW (mi n) = 40.00  
MAXIMUM STORAGE USED (ha. m.) = 1.9159

RESERVOIR ( 0020)  
IN= 2--> OUT= 1  
DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

INFLOW: ID= 2 ( 0015)  
OUTFLOW: ID= 1 ( 0020)

PEAK FLOW REDUCTION [Qout/Oin] (%) = 39.24  
TIME SHIFT OF PEAK FLOW (mi n) = 40.00  
MAXIMUM STORAGE USED (ha. m.) = 1.9159

ADD HYD ( 0003)  
1 + 2 = 3

AREA OPEAK TPEAK R.V.  
ID= 1 ( 1421): 15.00 2.614 0.67 39.34  
ID= 2 ( 0151): 18.20 1.585 0.83 18.51  
ID= 3 ( 0003): 33.20 4.121 0.67 27.92

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
3 + 2 = 1

AREA OPEAK TPEAK R.V.  
ID= 3 ( 0003): 33.20 4.121 0.67 27.92  
ID= 2 ( 0002): 182.07 12.764 0.83 13.95  
ID= 1 ( 0003): 215.27 16.420 0.83 16.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)

VO output - Prop Controlled Upstream.txt					
Distance	Elevation	Manning			
0.00	274.12	0.0500			
5.20	273.17	0.0300	Main Channel		
11.50	272.11	0.0300	Main Channel		
56.67	272.00	0.0300	Main Channel		
64.61	272.44	0.0300	Main Channel		
68.47	272.73	0.0300	Main Channel		
73.75	273.06	0.0300 / 0.0500	Main Channel		
79.53	273.29	0.0500			

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi n.dat  
Output filename: C:\Users\p001279d\AppData\Local\Ci va\VH5\b\cccece0-2cc7-4973-8ab8-dd0de50b3a28\0b6d5720-e380-4e97-a3f3-c91af1449dc1\scve  
Summary filename: C:\Users\p001279d\AppData\Local\Ci va\VH5\b\cccece0-2cc7-4973-8ab8-dd0de50b3a28\0b6d5720-e380-4e97-a3f3-c91af1449dc1\scve

DATE: 11/28/2019 TIME: 10:09:34

USER:

COMMENTS:

\*\* SIMULATION: New DesignStormChicago - 3hr \*\*

CHI CAGO STORM IDF curve parameters: A=1499.500 B= 3.298 C= 0.794

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs

Storm time step = 10.00 min

Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	6.35	1.00	192.16	1.83	10.97	2.67	6.07
0.33	7.76	1.17	47.74	2.00	9.37	2.83	5.61
0.50	10.16	1.33	24.88	2.17	8.21	3.00	5.22
0.67	15.64	1.50	17.22	2.33	7.33		
0.83	36.28	1.67	13.33	2.50	6.64		

VO output - Prop Controlled Upstream.txt

CALIB NASHYD ( 0153) Area (ha)= 91.47 Curve Number (CN)= 69.0
Ia (mm)= 22.82 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.20
Unit Hyd Opeak (cms)= 17.469
PEAK FLOW (cms)= 3.540 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 14.306
TOTAL RAINFALL (mm)= 71.757
RUNOFF COEFFICIENT = 0.199
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm IDF curve parameters: A=1499.500 B= 3.298 C= 0.794
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB NASHYD ( 0152) Area (ha)= 90.60 Curve Number (CN)= 70.0
Ia (mm)= 21.77 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.20
Unit Hyd Opeak (cms)= 17.302
PEAK FLOW (cms)= 3.855 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 15.322
TOTAL RAINFALL (mm)= 71.757
RUNOFF COEFFICIENT = 0.214
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002) AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)
ID1= 1 ( 0152): 90.60 3.855 1.17 15.32
+ ID2= 2 ( 0153): 91.47 3.540 1.17 14.31
ID = 3 ( 0002): 182.07 7.395 1.17 14.81
NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm IDF curve parameters: A=1499.500 B= 3.298 C= 0.794
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
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VO output - Prop Controlled Upstream.txt
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB NASHYD ( 0151) Area (ha)= 18.20 Curve Number (CN)= 68.0
Ia (mm)= 11.95 # of Linear Res. (N)= 3.00
U.H. Tp(hrs)= 0.20
Unit Hyd Opeak (cms)= 3.476
PEAK FLOW (cms)= 1.141 (i)
TIME TO PEAK (hrs)= 1.167
RUNOFF VOLUME (mm)= 19.427
TOTAL RAINFALL (mm)= 71.757
RUNOFF COEFFICIENT = 0.271
(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm IDF curve parameters: A=1499.500 B= 3.298 C= 0.794
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB STANDHYD ( 1421) Area Total Imp(%)= 15.00 Dir. Conn. (%) = 29.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 4.50 10.50
Dep. Storage (mm)= 1.00 2.00
Average Slope (%) = 1.00 3.00
Length (m) = 258.20 40.00
Mannings n = 0.015 0.200

Max. Eff. Inten. (mm/hr)= 192.16 70.37
over (mi n) = 10.00 20.00
Storage Coeff. (mi n)= 3.79 (ii) 10.08 (ii)
Unit Hyd. Tpeak (mi n)= 10.00 20.00
Unit Hyd. peak (cms) = 0.16 0.08
PEAK FLOW (cms)= 2.19 1.19 \*TOTALS\*
TIME TO PEAK (hrs)= 1.00 1.17 2.752 (iii)
RUNOFF VOLUME (mm)= 70.76 28.30 40.61
TOTAL RAINFALL (mm)= 71.76 71.76 71.76
RUNOFF COEFFICIENT = 0.99 0.39 0.57

VO output - Prop Controlled Upstream.txt
\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 71.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm IDF curve parameters: A=1499.500 B= 3.298 C= 0.794
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB STANDHYD ( 0142) Area Total Imp(%)= 54.15 Dir. Conn. (%) = 64.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 36.28 17.87
Dep. Storage (mm)= 1.00 16.04
Average Slope (%) = 1.00 3.00
Length (m) = 600.83 40.00
Mannings n = 0.015 0.250

Max. Eff. Inten. (mm/hr)= 192.16 56.77
over (mi n) = 10.00 20.00
Storage Coeff. (mi n)= 6.29 (ii) 14.12 (ii)
Unit Hyd. Tpeak (mi n)= 10.00 20.00
Unit Hyd. peak (cms) = 0.14 0.07
PEAK FLOW (cms)= 15.35 1.42 \*TOTALS\*
TIME TO PEAK (hrs)= 1.00 1.17 15.922 (iii)
RUNOFF VOLUME (mm)= 70.76 24.93 54.26
TOTAL RAINFALL (mm)= 71.76 71.76 71.76
RUNOFF COEFFICIENT = 0.99 0.35 0.76

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 76.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM Ptotal = 71.76 mm IDF curve parameters: A=1499.500 B= 3.298 C= 0.794
used in: INTENSITY = A / (t + B)^C
Duration of storm = 3.00 hrs
Storm time step = 10.00 min
Time to peak ratio = 0.33

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

VO output - Prop Controlled Upstream.txt

Table with 4 columns: TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr), TIME (hrs), RAIN (mm/hr)

CALIB STANDHYD ( 0022) Area Total Imp(%)= 127.59 Dir. Conn. (%) = 49.00

IMPERVIOUS PERVIOUS (i)
Surface Area (ha)= 77.83 49.76
Dep. Storage (mm)= 2.00 5.00
Average Slope (%) = 1.00 2.00
Length (m) = 922.28 40.00
Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr)= 192.16 95.80
over (mi n) = 10.00 20.00
Storage Coeff. (mi n)= 7.46 (ii) 14.64 (ii)
Unit Hyd. Tpeak (mi n)= 10.00 20.00
Unit Hyd. peak (cms) = 0.13 0.07
PEAK FLOW (cms)= 26.01 6.38 \*TOTALS\*
TIME TO PEAK (hrs)= 1.00 1.17 28.969 (iii)
RUNOFF VOLUME (mm)= 69.76 29.74 49.35
TOTAL RAINFALL (mm)= 71.76 71.76 71.76
RUNOFF COEFFICIENT = 0.97 0.41 0.69

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES: CN\* = 69.0 Ia = Dep. Storage (Above)
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019) OVERFLOW IS OFF
IN= 2 ---> OUT= 1
DT= 10.0 min

Table with 4 columns: OUTFLOW (cms), STORAGE (ha.m.), OUTFLOW (cms), STORAGE (ha.m.)

AREA OPEAK TPEAK R.V. (ha) (cms) (hrs) (mm)
INFLOW: ID= 2 ( 0022) 127.590 28.969 1.00 49.35
OUTFLOW: ID= 1 ( 0019) 127.590 0.375 3.33 49.32

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 1.29
TIME SHIFT OF PEAK FLOW (mi n)=140.00
MAXIMUM STORAGE USED (ha. m.) = 5.9751

ROUTE CHN ( 0026) Routing time step (mi n)' = 10.00

Table with 4 columns: Distance, Elevation, Manning, Manning



VO output - Prop Controlled Upstream.txt  
73.75 273.06 0.0300 / 0.0500 Main Channel  
79.53 273.29 0.0500

DEPTH	ELEV	VOLUME	TIME	TABLE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(cms)	(m/s)	(min)
0.07	272.07	104E+04	0.9	0.29	19.48	12.41
0.13	272.13	211E+04	2.8	0.46	12.41	12.41
0.20	272.20	322E+04	5.6	0.59	9.56	3.99
0.26	272.26	436E+04	9.1	0.71	7.96	3.81
0.33	272.33	554E+04	13.3	0.82	6.93	3.66
0.40	272.40	676E+04	18.2	0.91	6.19	3.52
0.46	272.46	801E+04	23.7	1.01	5.62	3.42
0.53	272.53	928E+04	30.0	1.09	5.16	3.33
0.60	272.60	106E+05	36.8	1.18	4.80	3.27
0.66	272.66	119E+05	44.2	1.26	4.49	3.23
0.73	272.73	133E+05	52.2	1.33	4.24	3.20
0.79	272.79	147E+05	60.7	1.40	4.03	3.18
0.86	272.86	161E+05	69.7	1.47	3.84	3.16
0.93	272.93	175E+05	79.3	1.54	3.68	3.15
0.99	272.99	190E+05	89.5	1.60	3.54	3.14
1.06	273.06	205E+05	100.3	1.66	3.41	3.13
1.14	273.14	223E+05	114.9	1.74	3.24	3.12
1.22	273.22	242E+05	130.7	1.83	3.09	3.11
1.29	273.29	262E+05	147.5	1.91	2.96	3.10

<--- hydrograph ---> <- pipe / channel ->  
INFLOW : ID= 2 ( 0019) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)  
OUTFLOW: ID= 1 ( 0026) 127.59 0.37 3.83 49.32 0.03 0.29

ADD HYD ( 0017)  
1 + 2 = 3  
ID1= 1 ( 0142): AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
54.15 15.922 1.00 54.26  
+ ID2= 2 ( 0026): 127.59 0.374 3.83 49.32  
-----  
ID = 3 ( 0017): 181.74 15.963 1.00 50.79

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN( 0015)  
IN= 2--> OUT= 1  
Routing time step (min)' = 10.00

Distance	Elevation	Manning	Main Channel
0.00	252.95	0.0500	
3.65	252.53	0.0300	Main Channel
12.66	251.91	0.0300	Main Channel
14.58	251.05	0.0300	Main Channel
76.79	251.08	0.0300	Main Channel
87.59	252.17	0.0300	Main Channel
90.94	252.44	0.0300 / 0.0500	Main Channel
98.65	253.00	0.0500	

DEPTH	ELEV	VOLUME	TIME	TABLE	VELOCITY	TRAV. TIME
(m)	(m)	(cu.m.)	(cms)	(cms)	(m/s)	(min)
0.10	251.15	671E+04	4.4	0.82	25.64	15.43
0.20	251.20	148E+05	15.9	1.36	15.43	11.67
0.30	251.35	230E+05	32.9	2.18	11.67	9.63
0.40	251.45	315E+05	54.5	3.10	8.32	7.40
0.49	251.55	402E+05	80.5	2.53	6.32	6.71
0.59	251.64	491E+05	110.6	2.84	5.74	6.17
0.69	251.74	582E+05	144.6	3.13	5.17	5.74
0.79	251.84	676E+05	182.5	3.41	4.67	5.38
0.89	251.94	771E+05	224.1	3.67	4.24	5.00
0.99	252.04	869E+05	269.3	3.91	3.81	4.67

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VO output - Prop Controlled Upstream.txt

V V I SSSSS U U A L (v 5. 2. 003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A L  
V V I SSSSS UUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T H H Y Y MM MM 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\voi.n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Civica\VS\5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\05dca9e7-1bf5-4985-8c56-aec15e9fd9d\scce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civica\VS\5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\05dca9e7-1bf5-4985-8c56-aec15e9fd9d\scce

DATE: 11/28/2019 TIME: 10: 09: 33

USER:

COMMENTS:

CHI CAGO STORM IDF curve parameters: A=1498.100  
Ptotal = 78.29 mm B= 2.188  
used in: INTENSITY = A / ( t + B ) ^ C  
C= 0.778

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45
0.50	11.28	1.33	26.15	2.17	9.22
0.67	16.53	1.50	18.53	2.33	8.28
0.83	37.27	1.67	14.57	2.50	7.53

CALIB NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0  
ID= 1 DT=10.0 min Ia (cm) = 22.82 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.469

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VO output - Prop Controlled Upstream.txt

1.09	252.14	969E+05	318.2	4.15	5.07
1.19	252.24	107E+06	370.2	4.36	4.82
1.29	252.34	118E+06	425.6	4.57	4.60
1.39	252.44	128E+06	484.6	4.77	4.41
1.49	252.54	140E+06	554.9	5.02	4.19
1.59	252.64	151E+06	632.3	5.28	3.99
1.69	252.75	163E+06	714.0	5.52	3.81
1.80	252.85	176E+06	799.8	5.75	3.66
1.90	252.95	188E+06	889.9	5.97	3.52

<--- hydrograph ---> <- pipe / channel ->  
INFLOW : ID= 2 ( 0017) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm) MAX DEPTH (m) MAX VEL (m/s)  
OUTFLOW: ID= 1 ( 0015) 181.74 15.96 1.00 50.79 0.20 1.36  
181.74 7.44 1.17 50.79 0.13 0.92

RESERVOIR( 0020) OVERFLOW IS OFF  
IN= 2--> OUT= 1  
DT= 10.0 min

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

INFLOW : ID= 2 ( 0015) AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
OUTFLOW: ID= 1 ( 0020) 181.740 7.444 1.17 50.79  
181.740 3.014 2.17 50.79

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 40.50  
TIME SHIFT OF PEAK FLOW (min) = 60.00  
MAXIMUM STORAGE USED (ha.m.) = 1.8375

ADD HYD ( 0003)  
1 + 2 = 3  
ID1= 1 ( 1421): AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
15.00 2.752 1.00 40.61  
+ ID2= 2 ( 0151): 18.20 1.141 1.17 19.43  
-----  
ID = 3 ( 0003): 33.20 3.684 1.00 29.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
3 + 2 = 1  
ID1= 3 ( 0003): AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
33.20 3.684 1.00 29.00  
+ ID2= 2 ( 0002): 182.07 7.395 1.17 14.81  
-----  
ID = 1 ( 0003): 215.27 10.416 1.17 17.00

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3  
ID1= 1 ( 0003): AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
215.27 10.416 1.17 17.00  
+ ID2= 2 ( 0020): 181.74 3.014 2.17 50.79  
-----  
ID = 3 ( 0003): 397.01 10.757 1.17 32.47

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

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VO output - Prop Controlled Upstream.txt

PEAK FLOW (cms) = 4.517 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 17.674  
TOTAL RAINFALL (mm) = 78.293  
RUNOFF COEFFICIENT = 0.226

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1498.100  
Ptotal = 78.29 mm B= 2.188  
C= 0.778

used in: INTENSITY = A / ( t + B ) ^ C

Duration of storm = 3.00 hrs

Storm time step = 10.00 min

Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0  
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 4.872 (i)

TIME TO PEAK (hrs) = 1.167

RUNOFF VOLUME (mm) = 18.816

TOTAL RAINFALL (mm) = 78.293

RUNOFF COEFFICIENT = 0.240

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
1 + 2 = 3  
ID1= 1 ( 0152): AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
90.60 4.872 1.17 18.82  
+ ID2= 2 ( 0153): 91.47 4.517 1.17 17.67  
-----  
ID = 3 ( 0002): 182.07 9.388 1.17 18.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM IDF curve parameters: A=1498.100  
Ptotal = 78.29 mm B= 2.188  
C= 0.778

used in: INTENSITY = A / ( t + B ) ^ C

Duration of storm = 3.00 hrs

Storm time step = 10.00 min

Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98

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VO output - Prop Controlled Upstream.txt  
 0.67 16.53 | 1.50 18.53 | 2.33 8.28  
 0.83 37.27 | 1.67 14.57 | 2.50 7.53

CALIB  
 WASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0  
 ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Linear Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20 C= 0.778

Unit Hyd Opeak (cms) = 3.476  
 PEAK FLOW (cms) = 1.364 (i)  
 TIME TO PEAK (hrs) = 1.167  
 RUNOFF VOLUME (mm) = 23.064  
 TOTAL RAINFALL (mm) = 78.293  
 RUNOFF COEFFICIENT = 0.295

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1498.100  
 Total = 78.29 mm B = 2.188  
 C = 0.778  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB  
 STANDHYD ( 1421) Area (ha) = 15.00  
 ID= 1 DT=10.0 min Total Imp(%) = 30.00 Dir. Conn.(%) = 29.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 4.50 10.50  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 258.20 40.00  
 Mannings n = 0.015 0.200

Max. Eff. Inten. (mm/hr) = 214.13 83.66  
 over (mi n) = 10.00 10.00  
 Storage Coeff. (mi n) = 3.63 (ii) 9.50 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 10.00  
 Unit Hyd. peak (cms) = 0.16 0.11

PEAK FLOW (cms) = 2.45 1.63 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.00 4.079 (iii)  
 RUNOFF VOLUME (mm) = 77.29 32.61 45.57  
 TOTAL RAINFALL (mm) = 78.29 78.29 78.29  
 RUNOFF COEFFICIENT = 0.99 0.42 0.58

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 71.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

VO output - Prop Controlled Upstream.txt

CHI CAGO STORM  
 Total = 78.29 mm

IDF curve parameters: A=1498.100  
 B = 2.188  
 C = 0.778  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

CALIB  
 STANDHYD ( 0142) Area (ha) = 54.15  
 ID= 1 DT=10.0 min Total Imp(%) = 67.00 Dir. Conn.(%) = 64.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 36.28 17.87  
 Dep. Storage (mm) = 1.00 16.04  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 600.83 40.00  
 Mannings n = 0.015 0.250

Max. Eff. Inten. (mm/hr) = 214.13 72.09  
 over (mi n) = 10.00 20.00  
 Storage Coeff. (mi n) = 6.02 (ii) 13.14 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit Hyd. peak (cms) = 0.14 0.07

PEAK FLOW (cms) = 17.30 1.82 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.17 18.05 (iii)  
 RUNOFF VOLUME (mm) = 77.29 29.49 60.08  
 TOTAL RAINFALL (mm) = 78.29 78.29 78.29  
 RUNOFF COEFFICIENT = 0.99 0.38 0.77

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 76.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
 Total = 78.29 mm

IDF curve parameters: A=1498.100  
 B = 2.188  
 C = 0.778  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	7.21	1.00	214.13	1.83	12.13	2.67	6.92
0.33	8.74	1.17	48.43	2.00	10.45	2.83	6.41
0.50	11.28	1.33	26.15	2.17	9.22	3.00	5.98
0.67	16.53	1.50	18.53	2.33	8.28		
0.83	37.27	1.67	14.57	2.50	7.53		

VO output - Prop Controlled Upstream.txt

CALIB  
 STANDHYD ( 0022) Area (ha) = 127.59  
 ID= 1 DT=10.0 min Total Imp(%) = 61.00 Dir. Conn.(%) = 49.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 77.83 49.76  
 Dep. Storage (mm) = 2.00 5.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 922.28 40.00  
 Mannings n = 0.013 0.250

Max. Eff. Inten. (mm/hr) = 214.13 114.18  
 over (mi n) = 10.00 20.00  
 Storage Coeff. (mi n) = 7.14 (ii) 13.84 (ii)  
 Unit Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit Hyd. peak (cms) = 0.13 0.07

PEAK FLOW (cms) = 29.37 7.71 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.17 33.000 (iii)  
 RUNOFF VOLUME (mm) = 76.29 34.29 54.87  
 TOTAL RAINFALL (mm) = 78.29 78.29 78.29  
 RUNOFF COEFFICIENT = 0.97 0.44 0.70

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 69.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019)  
 IN= 2----> OUT= 1  
 DT= 10.0 min

OVERFLOW IS OFF  
 OUTFLOW (cms) STORAGE (ha.m.)  
 0.0000 0.0000 0.3200 3.8870  
 0.1600 1.7840 0.3400 4.3720  
 0.2000 2.7010 0.3500 4.8700  
 0.2600 3.2740 0.3800 6.2160

INFLOW : ID= 2 ( 0022) 127.590 33.000 1.00 54.87  
 OUTFLOW: ID= 1 ( 0019) 127.590 0.390 3.33 54.85

PEAK FLOW REDUCTION [Qout/Oi n] = 1.18  
 TIME SHIFT OF PEAK FLOW (min)=140.00  
 MAXIMUM STORAGE USED (ha.m.) = 6.6632

ROUTE CHN ( 0026)  
 IN= 2----> OUT= 1

Routing time step (mi n)' = 10.00

----- DATA FOR SECTION ( 1.1) -----  
 Distance Elevation Manning  
 0.00 274.12 0.0500  
 5.20 273.17 0.0300 Main Channel  
 10.58 272.11 0.0300 Main Channel  
 11.50 272.00 0.0300 Main Channel  
 56.67 272.00 0.0300 Main Channel  
 64.61 272.44 0.0300 Main Channel  
 68.47 272.73 0.0300 Main Channel  
 73.75 273.06 0.0300 / 0.0500 Main Channel  
 79.53 273.29 0.0500

TRAVEL TIME TABLE  
 DEPTH (m) ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV TIME (min)  
 0.07 272.07 .104E+04 0.9 0.29 19.48  
 0.13 272.13 .211E+04 2.8 0.46 12.41

VO output - Prop Controlled Upstream.txt

	AREA	OPEAK	TPEAK	R.V.	MAX DEPTH	MAX VEL
INFLOW : ID= 2 ( 0019)	(ha)	(cms)	(hrs)	(mm)	(m)	(m/s)
0.20 272.20	322E+04	5.6	9.1	0.59	9.56	0.71
0.26 272.26	436E+04	9.1	13.3	0.82	6.93	0.82
0.33 272.33	554E+04	13.3	18.5	0.91	6.19	0.91
0.40 272.40	676E+04	18.5	23.7	1.01	5.62	1.01
0.46 272.46	801E+04	23.7	30.0	1.09	5.16	1.09
0.53 272.53	928E+04	30.0	36.8	1.18	4.80	1.18
0.60 272.60	106E+05	36.8	44.2	1.26	4.49	1.26
0.73 272.73	133E+05	44.2	52.2	1.33	4.24	1.33
0.79 272.79	147E+05	60.7	60.7	1.40	4.03	1.40
0.86 272.86	161E+05	69.7	79.3	1.47	3.84	1.47
0.93 272.93	175E+05	79.3	89.5	1.54	3.68	1.54
0.99 272.99	190E+05	89.5	100.3	1.60	3.54	1.60
1.06 273.06	205E+05	100.3	114.7	1.66	3.41	1.66
1.14 273.14	223E+05	114.7	130.9	1.74	3.24	1.74
1.22 273.22	242E+05	130.9	147.5	1.81	3.09	1.81
1.29 273.29	262E+05	147.5				

----- hydrograph -----  
 INFLOW : ID= 2 ( 0019) 127.59 0.39 3.33 54.85 0.03 0.29  
 OUTFLOW: ID= 1 ( 0026) 127.59 0.39 3.83 54.85 0.03 0.29

ADD HYD ( 0017)  
 1 + 2 = 3

AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0142): 54.15 18.057 1.00 60.08  
 + ID2= 2 ( 0026): 127.59 0.389 3.83 54.85  
 ID = 3 ( 0017): 181.74 18.104 1.00 56.41

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN ( 0015)  
 IN= 2----> OUT= 1

Routing time step (mi n)' = 10.00

----- DATA FOR SECTION ( 1.1) -----  
 Distance Elevation Manning  
 0.00 252.95 0.0500  
 3.65 251.31 0.0500 / 0.0300 Main Channel  
 12.66 251.31 0.0300 Main Channel  
 14.58 251.05 0.0300 Main Channel  
 0.76 251.79 0.0300 Main Channel  
 87.59 252.17 0.0300 Main Channel  
 90.94 252.44 0.0300 / 0.0500 Main Channel  
 98.65 253.00 0.0500

TRAVEL TIME TABLE  
 DEPTH (m) ELEV (m) VOLUME (cu.m.) FLOW RATE (cms) VELOCITY (m/s) TRAV TIME (min)  
 0.10 251.15 4.4 15.9 1.36 15.43  
 0.20 251.25 148E+05 15.9 2.18 11.67  
 0.30 251.35 230E+05 32.9 2.18 9.63  
 0.40 251.45 315E+05 54.5 2.53 8.32  
 0.49 251.55 402E+05 80.5 2.84 7.40  
 0.59 251.64 491E+05 110.6 3.13 6.71  
 0.69 251.74 582E+05 144.6 3.41 6.17  
 0.79 251.84 676E+05 182.5 3.67 5.74  
 0.89 251.94 771E+05 224.1 3.91 5.38  
 0.99 252.04 869E+05 269.3 4.15 5.07  
 1.09 252.14 969E+05 318.2 4.36 4.82  
 1.19 252.24 107E+06 370.2 4.57 4.60  
 1.29 252.34 118E+06 425.6 4.77 4.41  
 1.39 252.44 128E+06 484.6 5.02 4.19  
 1.49 252.54 140E+06 554.9 5.28 3.99  
 1.59 252.64 151E+06 632.3 5.52 3.81  
 1.69 252.75 163E+06 714.0 5.75 3.66  
 1.80 252.85 176E+06 799.8

VO output - Prop Controlled Upstream.txt  
1.90 252.95 .188E+06 889.9 5.97 3.52

<--- hydrograph ---> <- pipe / channel ->  
AREA OPEAK TPEAK R.V. MAX DEPTH MAX VEL  
(ha) (cms) (hrs) (mm) (m) (m/s)  
INFLOW: ID= 2 ( 0017) 181.74 18.10 1.00 56.41 0.21 1.41  
OUTFLOW: ID= 1 ( 0015) 181.74 8.47 1.17 56.41 0.13 0.95

RESERVOIR ( 0020)  
IN= 2---> OUT= 1  
DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
181.740	8.473	1.17	56.41
181.740	3.263	2.00	56.41

INFLOW: ID= 2 ( 0015)  
OUTFLOW: ID= 1 ( 0020)

PEAK FLOW REDUCTION [Out/Oi n] (%) = 38.51  
TIME SHIFT OF PEAK FLOW (min) = 50.00  
MAXIMUM STORAGE USED (ha.m.) = 1.8618

ADD HYD ( 0003)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
15.00	4.079	1.00	45.57
18.20	1.364	1.17	23.06
33.20	5.252	1.00	33.23

ID1= 1 ( 1421):  
+ ID2= 2 ( 0151):  
ID = 3 ( 0003):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
3 + 2 = 1

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
33.20	5.252	1.00	33.23
182.07	9.388	1.17	18.24
215.27	12.543	1.17	20.55

ID1= 3 ( 0003):  
+ ID2= 2 ( 0002):  
ID = 1 ( 0003):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
215.27	12.543	1.17	20.55
181.74	3.263	2.00	56.41
397.01	12.928	1.17	36.97

ID1= 1 ( 0003):  
+ ID2= 2 ( 0020):  
ID = 3 ( 0003):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)  
V V I SS U U A A L  
V V I SS U U A A A A L  
V V I SS U U A A A L

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VO output - Prop Controlled Upstream.txt  
W I SSSSS UUUUU A A LLLLL

000 TTTT TTTT H H Y Y M M 000 TM  
0 0 T T T H H Y Y M M 0 0  
0 0 T T H H Y Y M M 0 0  
000 T T H H Y Y M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Vialoft\OTHYMO 5.2\VO2\voi.n.dat

Output filename:  
C:\Users\p001279d\AppData\Local\Civica\WH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\269e5f  
e1-864b-4ce5-9da5-e3ae993287ec\vsce  
Summary filename:  
C:\Users\p001279d\AppData\Local\Civica\WH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\269e5f  
e1-864b-4ce5-9da5-e3ae993287ec\vsce

DATE: 11/28/2019 TIME: 10:09:34

USER:

COMMENTS:

\*\*\*\*\*  
\*\* SIMULATION : New DesignStormChicago - 3hr \*\*  
\*\*\*\*\*

CHICAGO STORM IDF curve parameters: A=1496.300  
Ptotal = 60.39 mm B = 5.250  
C = 0.825

used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB

NASHYD ( 0153) Area (ha) = 91.47 Curve Number (CN) = 69.0  
ID= 1 DT=10.0 min Ia (mm) = 22.82 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 2.068 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 9.065  
TOTAL RAINFALL (mm) = 60.393  
RUNOFF COEFFICIENT = 0.150

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.  
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VO output - Prop Controlled Upstream.txt

CHICAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B = 5.250  
C = 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB

NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0  
ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.302

PEAK FLOW (cms) = 2.307 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 9.852  
TOTAL RAINFALL (mm) = 60.393  
RUNOFF COEFFICIENT = 0.163

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
90.60	2.307	1.17	9.85
91.47	2.068	1.17	9.07
182.07	4.375	1.17	9.46

ID1= 1 ( 0152):  
+ ID2= 2 ( 0153):  
ID = 3 ( 0002):

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHICAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B = 5.250  
C = 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB

NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0  
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ID= 1 DT=10.0 min | VO output - Prop Controlled Upstream.txt  
Ia (mm) = 11.95 # of Linear Res. (N) = 3.00  
U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 3.476

PEAK FLOW (cms) = 0.787 (i)  
TIME TO PEAK (hrs) = 1.167  
RUNOFF VOLUME (mm) = 13.608  
TOTAL RAINFALL (mm) = 60.393  
RUNOFF COEFFICIENT = 0.225

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B = 5.250  
C = 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
Storm time step = 10.00 min  
Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB

STANDHYD ( 01421) Area (ha) = 15.00 Curve Number (CN) = 70.0  
ID= 1 DT=10.0 min Total Imp(%) = 30.00 Dir. Conn. (%) = 29.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha)	4.50
Dep. Storage (mm)	1.00
Average Slope (%)	1.00
Length (m)	258.20
Mannings n	0.015
Max. Eff. Inten. (mm/hr)	158.06
over (min)	10.00
Storage Coeff. (min)	4.10 (ii)
Unit Hyd. Tpeak (min)	10.00
Unit Hyd. peak (cms)	0.16

PEAK FLOW (cms) = 1.78  
TIME TO PEAK (hrs) = 1.00  
RUNOFF VOLUME (mm) = 59.39  
TOTAL RAINFALL (mm) = 60.39  
RUNOFF COEFFICIENT = 0.98

\*TOTALS\*  
2.165 (iii)  
0.825  
60.39  
0.53

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
CN = 71.0 Ia = Dep. Storage (Above)  
(ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
THAN THE STORAGE COEFFICIENT  
(iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
B = 5.250  
C = 0.825  
used in: INTENSITY = A / (t + B)^C  
Duration of storm = 3.00 hrs  
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VO output - Prop Controlled Upstream.txt  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB STANDHYD ( 0142)  
 ID= 1 DT=10.0 min

Area (ha) = 54.15  
 Total Imp(%) = 67.00  
 Dir. Conn. (%) = 64.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 36.28	17.87
Dep. Storage (mm) = 1.00	16.04
Average Slope (%) = 1.00	3.00
Length (m) = 600.83	40.00
Mannings n = 0.015	0.250
Max. Eff. Inten. (mm/hr) over (min) = 158.06 / 10.00	34.58 / 20.00
Storage Coeff. (min) = 6.80 (ii)	16.35 (iii)
Unit Hyd. Tpeak (min) = 10.00	20.00
Unit Hyd. peak (cms) = 0.13	0.06

PEAK FLOW (cms)	12.35	0.87	*TOTALS*	
TIME TO PEAK (hrs) =	1.00	1.33	12.665	(iii)
RUNOFF VOLUME (mm) =	59.39	17.51	44.32	
TOTAL RAINFALL (mm) =	60.39	60.39	60.39	
RUNOFF COEFFICIENT =	0.98	0.29	0.73	

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 76.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM Ptotal = 60.39 mm

IDF curve parameters: A=1496.300  
 B= 5.250  
 C= 0.825

used in: INTENSITY = A / (T + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.17	4.89	1.00	158.06	1.83	8.94	2.67	4.66
0.33	6.10	1.17	44.35	2.00	7.51	2.83	4.27
0.50	8.21	1.33	22.12	2.17	6.49	3.00	3.95
0.67	12.90	1.50	14.74	2.33	5.73		
0.83	33.21	1.67	11.10	2.50	5.14		

CALIB STANDHYD ( 0022)  
 ID= 1 DT=10.0 min

Area (ha) = 127.59  
 Total Imp(%) = 61.00  
 Dir. Conn. (%) = 49.00

IMPERVIOUS	PERVIOUS (i)
Surface Area (ha) = 49.76	77.83
Dep. Storage (mm) = 2.00	5.00

VO output - Prop Controlled Upstream.txt  
 Average Slope (%) = 1.00  
 Length (m) = 922.28  
 Mannings n = 0.013

Max. Eff. Inten. (mm/hr) over (min)	158.06	68.41	*TOTALS*	
Storage Coeff. (min) =	8.07 (ii)	16.28 (ii)	22.818	(iii)
Unit Hyd. Tpeak (min) =	10.00	20.00	1.00	
Unit Hyd. peak (cms) =	0.12	0.06	39.96	
PEAK FLOW (cms) =	20.84	4.40	60.39	
TIME TO PEAK (hrs) =	1.00	1.17	0.97	
RUNOFF VOLUME (mm) =	58.39	22.25	60.39	
TOTAL RAINFALL (mm) =	60.39	60.39	0.66	
RUNOFF COEFFICIENT =	0.97	0.37		

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 69.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019)  
 IN= 2--> OUT= 1  
 DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

INFLOW: ID= 2 ( 0022)	127.590	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 ( 0019)	127.590	22.818	1.00	39.96
		0.349	3.33	39.94

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 1.53  
 TIME SHIFT OF PEAK FLOW (min) = 140.00  
 MAXIMUM STORAGE USED (ha. m.) = 4.8101

ROUTE CHN ( 0026)  
 IN= 2--> OUT= 1

Routing time step (min) = 10.00

Distance	Elevation	Manning
0.00	274.12	0.0500
5.20	273.17	0.0300
10.58	272.11	0.0300
11.50	272.00	0.0300
56.67	272.00	0.0300
64.61	272.44	0.0300
68.47	272.73	0.0300
73.75	273.96	0.0300 / 0.0500
79.53	273.29	0.0300

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.07	272.07	104E+04	0.9	0.29	19.48
0.13	272.13	211E+04	2.8	0.46	12.41
0.20	272.20	322E+04	5.6	0.59	9.56
0.26	272.26	436E+04	9.1	0.71	7.96
0.33	272.33	554E+04	13.3	0.82	6.93
0.40	272.40	676E+04	18.2	0.91	6.19
0.46	272.46	801E+04	23.7	1.01	5.62
0.53	272.53	928E+04	30.0	1.09	5.16
0.60	272.60	106E+05	36.8	1.18	4.80
0.66	272.66	119E+05	44.2	1.26	4.49

TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr	TIME hrs	RAIN mm/hr
0.73	272.73	1.33E+05	52.2	1.33	3.24		
0.79	272.79	147E+05	60.7	1.40	4.03		
0.86	272.86	161E+05	69.7	1.47	3.84		
0.93	272.93	175E+05	79.3	1.54	3.68		
0.99	272.99	190E+05	89.5	1.60	3.54		
1.06	273.06	205E+05	100.3	1.66	3.41		
1.14	273.14	223E+05	114.9	1.74	3.24		
1.22	273.22	242E+05	130.7	1.83	3.09		
1.29	273.29	262E+05	147.5	1.91	2.96		

<--- hydrograph ---> <-pi pe / channel->  
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL  
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW: ID= 2 ( 0019)	127.59	0.35	3.33	39.94	0.03	0.29
OUTFLOW: ID= 1 ( 0026)	127.59	0.35	3.83	39.94	0.03	0.29

ADD HYD ( 0017)  
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 ( 0142):	54.15	12.665	1.00	44.32
+ ID2= 2 ( 0026):	127.59	0.348	3.83	39.94
ID = 3 ( 0017):	181.74	12.697	1.00	41.24

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN ( 0015)  
 IN= 2--> OUT= 1

Routing time step (min) = 10.00

Distance	Elevation	Manning
0.00	252.95	0.0500
3.65	252.53	0.0500 / 0.0300
12.66	251.31	0.0300
14.58	251.05	0.0300
76.79	251.08	0.0300
87.59	252.17	0.0300
90.94	252.44	0.0300 / 0.0500
98.65	253.00	0.0500

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.10	251.15	671E+04	4.4	0.82	25.64
0.20	251.25	148E+05	15.9	1.36	15.43
0.30	251.35	230E+05	32.9	1.80	11.67
0.40	251.45	315E+05	54.5	2.18	9.63
0.49	251.55	402E+05	80.5	2.53	8.32
0.59	251.64	491E+05	110.6	2.84	7.40
0.69	251.74	582E+05	144.6	3.13	6.71
0.79	251.84	676E+05	182.5	3.41	6.17
0.89	251.94	771E+05	224.1	3.67	5.74
0.99	252.04	869E+05	269.3	3.91	5.38
1.09	252.14	969E+05	318.2	4.15	5.07
1.19	252.24	107E+06	370.2	4.36	4.82
1.29	252.34	118E+06	425.6	4.57	4.60
1.39	252.44	128E+06	484.6	4.77	4.41
1.49	252.54	140E+06	554.9	5.02	4.19
1.59	252.64	151E+06	632.3	5.28	3.99
1.69	252.75	163E+06	714.0	5.52	3.81
1.80	252.85	176E+06	799.8	5.75	3.66
1.90	252.95	188E+06	889.9	5.97	3.52

<--- hydrograph ---> <-pi pe / channel->  
 AREA OPEAK TPEAK R. V. MAX DEPTH MAX VEL  
 (ha) (cms) (hrs) (mm) (m) (m/s)

INFLOW: ID= 2 ( 0017)	181.74	12.70	1.00	41.24	0.17	1.15
OUTFLOW: ID= 1 ( 0015)	181.74	5.79	1.17	41.24	0.11	0.86

VO output - Prop Controlled Upstream.txt

RESERVOIR ( 0020)  
 IN= 2--> OUT= 1  
 DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

INFLOW: ID= 2 ( 0015)	181.740	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW: ID= 1 ( 0020)	181.40	5.794	1.17	41.24
	181.40	1.469	2.67	41.24

PEAK FLOW REDUCTION [Oout/Oi n] (%) = 25.35  
 TIME SHIFT OF PEAK FLOW (min) = 90.00  
 MAXIMUM STORAGE USED (ha. m.) = 1.6694

ADD HYD ( 0003)  
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 ( 1421):	15.00	2.165	1.00	32.30
+ ID2= 2 ( 0151):	182.07	4.375	1.17	13.61
ID = 3 ( 0003):	33.20	2.750	1.00	22.05

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 3 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 3 ( 0003):	33.20	2.750	1.00	22.05
+ ID2= 2 ( 0020):	182.07	4.375	1.17	9.46
ID = 1 ( 0003):	215.27	6.640	1.17	11.40

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003)  
 1 + 2 = 3

AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	
ID1= 1 ( 0003):	215.27	6.640	1.17	11.40
+ ID2= 2 ( 0020):	181.74	1.469	2.67	41.24
ID = 3 ( 0003):	397.01	6.908	1.17	25.06

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSSS U U A L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U A A A A L  
 V V I SS U U A A L  
 V V I SSSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M 000 TM  
 0 0 T T H H Y Y M M 0 0  
 0 0 T T H H Y Y M M 0 0  
 0 0 T T H H Y Y M M 0 0

\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vojn.dat  
 Output filename:  
 C:\Users\p001279d\AppData\Local\CI\vi\ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\09129c  
 e5-5ece-4bb5-8fd8-ed67bf91cb5a\scse  
 Summary filename:  
 C:\Users\p001279d\AppData\Local\CI\vi\ca\VH5\b8ccece0-2cc7-4973-8ab8-dd0de50b3a28\09129c  
 e5-5ece-4bb5-8fd8-ed67bf91cb5a\scse

DATE: 11/28/2019 TIME: 10:09:34  
 USER:

COMMENTS:

CHICAGO STORM  
 Ptotal = 33.36 mm  
 IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 NASHYD ( 0153)  
 ID= 1 DT=10.0 min  
 U.H. Tp(hrs)= 0.20  
 Area (ha) = 91.47  
 Curve Number (CN) = 69.0  
 # of Linear Res. (N) = 3.00

Unit Hyd Opeak (cms) = 17.469  
 PEAK FLOW (cms) = 0.134 (i)  
 TIME TO PEAK (hrs) = 1.833  
 RUNOFF VOLUME (mm) = 0.869  
 TOTAL RAINFALL (mm) = 33.364  
 RUNOFF COEFFICIENT = 0.026  
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
 Ptotal = 33.36 mm  
 IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 NASHYD ( 0152)  
 ID= 1 DT=10.0 min  
 U.H. Tp(hrs)= 0.20  
 Area (ha) = 90.60  
 Curve Number (CN) = 70.0  
 # of Linear Res. (N) = 3.00

Unit Hyd Opeak (cms) = 17.302  
 PEAK FLOW (cms) = 0.168 (i)  
 TIME TO PEAK (hrs) = 1.667  
 RUNOFF VOLUME (mm) = 1.087  
 TOTAL RAINFALL (mm) = 33.364  
 RUNOFF COEFFICIENT = 0.033

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
 1 + 2 = 3  
 ID1= 1 ( 0152): 90.60 0.168 1.67 1.09  
 + ID2= 2 ( 0153): 91.47 0.134 1.83 0.87  
 ID = 3 ( 0002): 182.07 0.299 1.83 0.98

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHICAGO STORM  
 Ptotal = 33.36 mm  
 IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 NASHYD ( 0151)  
 ID= 1 DT=10.0 min  
 U.H. Tp(hrs)= 0.20  
 Area (ha) = 18.20  
 Curve Number (CN) = 68.0  
 # of Linear Res. (N) = 3.00

Unit Hyd Opeak (cms) = 3.476  
 PEAK FLOW (cms) = 0.134 (i)  
 TIME TO PEAK (hrs) = 1.335  
 RUNOFF VOLUME (mm) = 3.169

VO output - Prop Controlled Upstream.txt  
 TOTAL RAINFALL (mm) = 33.364  
 RUNOFF COEFFICIENT = 0.095  
 (i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
 Ptotal = 33.36 mm  
 IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 1421)  
 ID= 1 DT=10.0 min  
 Area (ha) = 15.00  
 Dir. Conn. (%) = 29.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 4.50 10.50  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 258.20 40.00  
 Mannings n = 0.015 0.200  
 Max. Eff. Inten. (mm/hr) = 83.04 11.44  
 over (mi n) = 10.00 20.00  
 Storage Coeff. (mi n) = 5.30 (ii) 18.31 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 20.00  
 Unit t Hyd. peak (cms) = 0.14 0.06  
 PEAK FLOW (cms) = 0.88 0.20 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.17 0.971 (iii)  
 RUNOFF VOLUME (mm) = 32.36 7.37 14.62  
 TOTAL RAINFALL (mm) = 33.36 33.36 33.36  
 RUNOFF COEFFICIENT = 0.97 0.22 0.44

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 71.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
 Ptotal = 33.36 mm  
 IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44

CALIB  
 STANDHYD ( 0142)  
 ID= 1 DT=10.0 min  
 Area (ha) = 54.15  
 Dir. Conn. (%) = 64.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 36.28 17.87  
 Dep. Storage (mm) = 1.00 16.04  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 600.83 40.00  
 Mannings n = 0.015 0.250  
 Max. Eff. Inten. (mm/hr) = 83.04 3.39  
 over (mi n) = 10.00 40.00  
 Storage Coeff. (mi n) = 8.79 32.99 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 40.00  
 Unit t Hyd. peak (cms) = 0.12 0.03  
 PEAK FLOW (cms) = 5.88 0.10 \*TOTALS\*  
 TIME TO PEAK (hrs) = 1.00 1.83 1.00  
 RUNOFF VOLUME (mm) = 32.36 3.78 22.07  
 TOTAL RAINFALL (mm) = 33.36 33.36 33.36  
 RUNOFF COEFFICIENT = 0.97 0.11 0.66

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!  
 (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN\* = 76.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL  
 THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHICAGO STORM  
 Ptotal = 33.36 mm  
 IDF curve parameters: A= 724.700  
 B= 5.000  
 C= 0.800  
 used in: INTENSITY = A / (t + B)^C

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	2.99	1.00	83.04	1.83	5.29	2.67	2.86
0.33	3.69	1.17	24.13	2.00	4.49	2.83	2.63
0.50	4.88	1.33	12.47	2.17	3.91	3.00	2.44
0.67	7.49	1.50	8.49	2.33	3.47		
0.83	18.29	1.67	6.50	2.50	3.13		

CALIB  
 STANDHYD ( 0022)  
 ID= 1 DT=10.0 min  
 Area (ha) = 127.59  
 Dir. Conn. (%) = 49.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 77.83 49.76  
 Dep. Storage (mm) = 2.00 5.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 922.28 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 83.04 14.73  
 over (mi n) = 10.00 30.00  
 Storage Coeff. (mi n) = 10.44 (ii) 25.62 (ii)  
 Unit t Hyd. Tpeak (mi n) = 10.00 30.00

VO output - Prop Controlled Upstream.txt  
 Unit Hyd. peak (cms) = 0.10 0.04  
 \*TOTALS\*  
 PEAK FLOW (cms) = 9.75 0.97 9.982 (iii)  
 TIME TO PEAK (hrs) = 1.00 1.50 1.00  
 RUNOFF VOLUME (mm) = 31.36 7.47 19.18  
 TOTAL RAINFALL (mm) = 33.36 33.36 33.36  
 RUNOFF COEFFICIENT = 0.94 0.22 0.57

- (i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES:  
 CN1 = 69.0 (a = Dep. Storage (Above))
- (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.
- (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019 )  
 IN= 2 ---> OUT= 1  
 DT= 10.0 min  
 OVERFLOW IS OFF  

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	0.3200	3.8970
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

AREA (ha) OPEAK (cms) TPEAK (hrs) R.V. (mm)  
 INFLOW: ID= 2 ( 0022) 127.590 9.982 1.00 19.18  
 OUTFLOW: ID= 1 ( 0019) 127.590 0.182 3.50 19.16

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 1.82  
 TIME SHIFT OF PEAK FLOW (min) = 150.00  
 MAXIMUM STORAGE USED (ha. m.) = 2.2831

ROUTE CHN( 0026 )  
 IN= 2 ---> OUT= 1  
 Routing time step (min)' = 10.00  
 DATA FOR SECTION ( 1.1 ) ----->  

Distance	Elevation	Manning	
0.00	274.12	0.0500	
5.20	273.17	0.0300	Main Channel
10.58	272.11	0.0300	Main Channel
11.50	272.00	0.0300	Main Channel
56.67	272.00	0.0300	Main Channel
64.61	272.44	0.0300	Main Channel
68.47	272.73	0.0300	Main Channel
73.75	273.06	0.0300 / 0.0500	Main Channel
79.53	273.29	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.07	272.07	104E+04	0.9	0.29	19.48
0.13	272.13	211E+04	2.8	0.46	12.41
0.20	272.20	322E+04	5.6	0.59	9.56
0.26	272.26	436E+04	9.1	0.71	7.96
0.33	272.33	554E+04	13.3	0.82	6.93
0.40	272.40	676E+04	18.2	0.91	6.19
0.46	272.46	801E+04	23.7	1.01	5.62
0.53	272.53	928E+04	30.0	1.09	5.16
0.60	272.60	106E+05	36.8	1.18	4.80
0.66	272.66	119E+05	44.2	1.26	4.49
0.73	272.73	133E+05	52.2	1.33	4.24
0.79	272.79	147E+05	60.7	1.40	4.03
0.86	272.86	161E+05	69.7	1.47	3.84
0.93	272.93	175E+05	79.3	1.54	3.68
0.99	272.99	190E+05	89.5	1.60	3.54
1.06	273.06	205E+05	100.3	1.66	3.41
1.14	273.14	223E+05	114.9	1.74	3.24
1.22	273.22	242E+05	130.7	1.83	3.09
1.29	273.29	262E+05	147.5	1.91	2.96

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VO output - Prop Controlled Upstream.txt  
 AREA OPEAK TPEAK R.V.  
 INFLOW: ID= 2 ( 0019) 127.59 0.18 3.50 19.16  
 OUTFLOW: ID= 1 ( 0026) 127.59 0.18 4.00 19.16

ADD HYD ( 0017 )  
 1 + 2 = 3  
 ID1= 1 ( 0142): 54.15 5.881 1.00 22.07  
 + ID2= 2 ( 0026): 127.59 0.181 4.00 19.16  
 ID = 3 ( 0017): 181.74 5.893 1.00 20.02

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN( 0015 )  
 IN= 2 ---> OUT= 1  
 Routing time step (min)' = 10.00

DATA FOR SECTION ( 1.1 ) ----->  

Distance	Elevation	Manning	
0.00	252.95	0.0500	
3.65	252.53	0.0500 / 0.0300	Main Channel
12.66	251.31	0.0300	Main Channel
14.58	251.05	0.0300	Main Channel
76.79	251.08	0.0300	Main Channel
87.59	252.17	0.0300	Main Channel
90.94	252.44	0.0300 / 0.0500	Main Channel
98.65	253.00	0.0500	

DEPTH (m)	ELEV (m)	VOLUME (cu. m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.10	251.15	671E+04	4.4	0.82	25.64
0.20	251.25	148E+05	15.9	1.36	15.43
0.30	251.35	230E+05	32.9	1.80	11.67
0.40	251.45	315E+05	54.5	2.18	9.63
0.49	251.55	402E+05	80.5	2.53	8.32
0.59	251.64	491E+05	110.6	2.84	7.40
0.69	251.74	582E+05	144.6	3.13	6.71
0.79	251.84	676E+05	182.5	3.41	6.17
0.89	251.94	771E+05	224.1	3.67	5.74
0.99	252.04	869E+05	269.3	3.91	5.38
1.09	252.14	969E+05	318.2	4.15	5.07
1.19	252.24	1076E+05	370.2	4.36	4.82
1.29	252.34	118E+06	425.6	4.57	4.60
1.39	252.44	128E+06	484.6	4.77	4.41
1.49	252.54	140E+06	554.9	5.02	4.19
1.59	252.64	151E+06	632.3	5.28	3.99
1.69	252.75	163E+06	714.0	5.52	3.81
1.80	252.85	176E+06	799.8	5.75	3.66
1.90	252.95	188E+06	889.9	5.97	3.52

AREA OPEAK TPEAK R.V.  
 INFLOW: ID= 2 ( 0017) 181.74 5.89 1.00 20.02  
 OUTFLOW: ID= 1 ( 0015) 181.74 2.61 1.33 20.02

RESERVOIR ( 0020 )  
 IN= 2 ---> OUT= 1  
 DT= 10.0 min  
 OVERFLOW IS OFF  

OUTFLOW (cms)	STORAGE (ha. m.)	OUTFLOW (cms)	STORAGE (ha. m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200

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VO output - Prop Controlled Upstream.txt  
 1.8300 1.7990 3.5000 1.8910  
 2.0100 1.8050 4.0000 1.9520  
 2.0200 1.8070 0.0000 0.0000

AREA OPEAK TPEAK R.V.  
 INFLOW: ID= 2 ( 0015) 181.740 2.605 1.33 20.02  
 OUTFLOW: ID= 1 ( 0020) 181.740 0.521 3.17 20.02

PEAK FLOW REDUCTION [Qout/Oi n] (%) = 20.01  
 TIME SHIFT OF PEAK FLOW (min) = 110.00  
 MAXIMUM STORAGE USED (ha. m.) = 0.8870

ADD HYD ( 0003 )  
 1 + 2 = 3  
 ID1= 1 ( 1421): 15.00 0.971 1.00 14.62  
 + ID2= 2 ( 0151): 18.20 0.134 1.33 3.17  
 ID = 3 ( 0003): 33.20 1.039 1.00 8.34

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )  
 3 + 2 = 1  
 ID1= 3 ( 0003): 33.20 1.039 1.00 8.34  
 + ID2= 2 ( 0002): 182.07 0.299 1.83 0.98  
 ID = 1 ( 0003): 215.27 1.039 1.00 2.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )  
 1 + 2 = 3  
 ID1= 1 ( 0003): 215.27 1.039 1.00 2.11  
 + ID2= 2 ( 0020): 181.74 0.521 3.17 20.02  
 ID = 3 ( 0003): 397.01 1.090 1.00 10.31

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

V V I SSSS U U A L (v 5.2.2003)  
 V V I SS U U A A L  
 V V I SS U U A A L  
 V V I SS U U A A L  
 V V I SSSS UUUU A A LLLL

000 TTTT TTTT H H Y Y M M M 000 TM  
 0 0 T T T H H Y Y M M M 0 0  
 0 0 T T T H H Y Y M M M 0 0  
 000 T T H Y M M M 000

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\*\*\*\*\* DETAILED OUTPUT \*\*\*\*\*

Input filename: C:\Program Files (x86)\Visual OTTHYMO 5.2\VO2\vo1n.dat

Output filename: Page 79

VO output - Prop Controlled Upstream.txt  
 C:\Users\p001279d\AppData\Local\Clvi\ca\VH5\B8cccece0-2cc7-4973-8ab8-dd0de50b3a28\F87cb50d-1808-40dc-b610-e2d776fab5be5cse  
 Summary filename:  
 C:\Users\p001279d\AppData\Local\Clvi\ca\VH5\B8cccece0-2cc7-4973-8ab8-dd0de50b3a28\F87cb50d-1808-40dc-b610-e2d776fab5be5cse

DATE: 11/28/2019 TIME: 10:09:35  
 USER:

COMMENTS:

\*\*\* SIMULATION: DesignStormChicago 3hr \*\*\*

CHI CAGO STORM  
 Ptotal = 45.35 mm  
 IDF curve parameters: A=1330.300  
 B= 7.938  
 C= 0.855  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB NASHVD ( 0153 )  
 ID= 1 DT=10.0 min  
 Area (ha) = 91.47  
 la (mm) = 22.82  
 U.H. Tp(hrs) = 0.20  
 Curve Number (CN) = 69.0  
 # of Li near Res. (N) = 3.00

Unit Hyd Opeak (cms) = 17.469

PEAK FLOW (cms) = 0.768 (i)  
 TIME TO PEAK (hrs) = 1.333  
 RUNOFF VOLUME (mm) = 3.617  
 TOTAL RAINFALL (mm) = 45.346  
 RUNOFF COEFFICIENT = 0.080

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM  
 Ptotal = 45.35 mm  
 IDF curve parameters: A=1330.300  
 B= 7.938  
 C= 0.855  
 used in: INTENSITY = A / (t + B)^C  
 Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)	TIME (hrs)	RAIN (mm/hr)
0.17	3.43	1.00	112.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70

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VO output - Prop Controlled Upstream.txt  
 0.67 10.04 1.50 11.61 2.33 4.08  
 0.83 27.27 1.67 8.50 2.50 3.61

CALIB NASHYD ( 0152) Area (ha) = 90.60 Curve Number (CN) = 70.0  
 ID= 1 DT=10.0 min Ia (mm) = 21.77 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 17.302  
 PEAK FLOW (cms) = 0.887 (i)  
 TIME TO PEAK (hrs) = 1.333  
 RUNOFF VOLUME (mm) = 4.088  
 TOTAL RAINFALL (mm) = 45.346  
 RUNOFF COEFFICIENT = 0.090

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

ADD HYD ( 0002)  
 1 + 2 = 3  
 AREA OPEAK TPEAK R.V.  
 (ha) (cms) (hrs) (mm)  
 ID1= 1 ( 0152): 90.60 0.887 1.33 4.09  
 + ID2= 2 ( 0153): 91.47 0.768 1.33 3.62  
 ID = 3 ( 0002): 182.07 1.656 1.33 3.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300  
 Ptotal = 45.35 mm B= 7.938  
 C= 0.855  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	12.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB NASHYD ( 0151) Area (ha) = 18.20 Curve Number (CN) = 68.0  
 ID= 1 DT=10.0 min Ia (mm) = 11.95 # of Li near Res. (N) = 3.00  
 U.H. Tp(hrs) = 0.20

Unit Hyd Opeak (cms) = 3.476  
 PEAK FLOW (cms) = 0.377 (i)  
 TIME TO PEAK (hrs) = 1.167  
 RUNOFF VOLUME (mm) = 7.103  
 TOTAL RAINFALL (mm) = 45.346  
 RUNOFF COEFFICIENT = 0.157

(i) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300  
 Ptotal = 45.35 mm B= 7.938  
 C= 0.855  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	12.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

VO output - Prop Controlled Upstream.txt

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 36.28 17.87  
 Dep. Storage (mm) = 1.00 16.04  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 600.83 40.00  
 Mannings n = 0.015 0.250  
 Max. Eff. Inten. (mm/hr) = 112.71 12.43  
 over (min) = 10.00 30.00  
 Storage Coeff. (min) = 7.78 (ii) 22.17 (ii)  
 Unit Hyd. Tpeak (min) = 10.00 30.00  
 Unit Hyd. peak (cms) = 0.12 0.05  
 \*TOTALS\*  
 PEAK FLOW (cms) = 8.44 0.36 8.491 (iii)  
 TIME TO PEAK (hrs) = 1.00 1.50 1.00  
 RUNOFF VOLUME (mm) = 44.35 9.01 31.63  
 TOTAL RAINFALL (mm) = 45.35 45.35 45.35  
 RUNOFF COEFFICIENT = 0.98 0.20 0.70

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES.  
 CN\* = 76.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300  
 Ptotal = 45.35 mm B= 7.938  
 C= 0.855  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	12.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 0022) Area (ha) = 127.59 Dir. Conn. (%) = 49.00  
 ID= 1 DT=10.0 min Total Imp(%) = 61.00

Surface Area (ha) = 77.83 49.76  
 Dep. Storage (mm) = 2.00 5.00  
 Average Slope (%) = 1.00 2.00  
 Length (m) = 922.28 40.00  
 Mannings n = 0.013 0.250  
 Max. Eff. Inten. (mm/hr) = 112.71 28.46  
 over (min) = 10.00 30.00  
 Storage Coeff. (min) = 9.24 (ii) 20.90 (ii)  
 Unit Hyd. Tpeak (min) = 10.00 30.00  
 Unit Hyd. peak (cms) = 0.11 0.05  
 \*TOTALS\*  
 PEAK FLOW (cms) = 14.15 2.10 14.667 (iii)  
 TIME TO PEAK (hrs) = 1.00 1.33 1.00  
 RUNOFF VOLUME (mm) = 43.35 13.39 28.07  
 TOTAL RAINFALL (mm) = 45.35 45.35 45.35  
 RUNOFF COEFFICIENT = 0.96 0.30 0.62

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

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VO output - Prop Controlled Upstream.txt  
 C= 0.855  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	12.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 1421) Area (ha) = 15.00 Dir. Conn. (%) = 29.00  
 ID= 1 DT=10.0 min Total Imp(%) = 30.00

IMPERVIOUS PERVIOUS (i)  
 Surface Area (ha) = 4.50 10.50  
 Dep. Storage (mm) = 1.00 2.00  
 Average Slope (%) = 1.00 3.00  
 Length (m) = 258.20 40.00  
 Mannings n = 0.015 0.200  
 Max. Eff. Inten. (mm/hr) = 112.71 28.23  
 over (min) = 10.00 20.00  
 Storage Coeff. (min) = 4.69 (ii) 13.75 (ii)  
 Unit Hyd. Tpeak (min) = 10.00 20.00  
 Unit Hyd. peak (cms) = 0.15 0.07

\*TOTALS\*  
 PEAK FLOW (cms) = 1.24 0.44 1.433 (iii)  
 TIME TO PEAK (hrs) = 1.00 1.17 1.00  
 RUNOFF VOLUME (mm) = 44.35 12.92 22.03  
 TOTAL RAINFALL (mm) = 45.35 45.35 45.35  
 RUNOFF COEFFICIENT = 0.98 0.28 0.49

\*\*\*\*\* WARNING: STORAGE COEFF. IS SMALLER THAN TIME STEP!

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES.  
 CN\* = 71.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

CHI CAGO STORM IDF curve parameters: A=1330.300  
 Ptotal = 45.35 mm B= 7.938  
 C= 0.855  
 used in: INTENSITY = A / (t + B)^C

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	12.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

CALIB STANDHYD ( 0142) Area (ha) = 54.15 Dir. Conn. (%) = 64.00  
 ID= 1 DT=10.0 min Total Imp(%) = 67.00

Duration of storm = 3.00 hrs  
 Storm time step = 10.00 min  
 Time to peak ratio = 0.33

TIME	RAIN	TIME	RAIN	TIME	RAIN	TIME	RAIN
hrs	mm/hr	hrs	mm/hr	hrs	mm/hr	hrs	mm/hr
0.17	3.43	1.00	12.71	1.83	6.69	2.67	3.25
0.33	4.38	1.17	36.59	2.00	5.51	2.83	2.95
0.50	6.09	1.33	17.98	2.17	4.69	3.00	2.70
0.67	10.04	1.50	11.61	2.33	4.08		
0.83	27.27	1.67	8.50	2.50	3.61		

VO output - Prop Controlled Upstream.txt

(i) CN PROCEDURE SELECTED FOR PERVIOUS LOSSES.  
 CN\* = 69.0 Ia = Dep. Storage (Above)  
 (ii) TIME STEP (DT) SHOULD BE SMALLER OR EQUAL THAN THE STORAGE COEFFICIENT.  
 (iii) PEAK FLOW DOES NOT INCLUDE BASEFLOW IF ANY.

RESERVOIR ( 0019)  
 IN= 2---> OUT= 1  
 DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW	STORAGE	OUTFLOW	STORAGE
(ha.m.)	(ha.m.)	(ha.m.)	(ha.m.)
0.0000	0.0000	0.3200	3.8870
0.1600	1.7840	0.3400	4.3720
0.2000	2.7010	0.3500	4.8700
0.2600	3.2740	0.3800	6.2160

INFLOW: ID= 2 ( 0022)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)
127.590	14.667	1.00	28.07	
OUTFLOW: ID= 1 ( 0019)	127.590	0.268	3.33	28.04

PEAK FLOW REDUCTION [Oout/Oin](%) = 1.83  
 TIME SHIFT OF PEAK FLOW (min) = 140.00  
 MAXIMUM STORAGE USED (ha.m.) = 3.3609

ROUTE CHN ( 0026)

Routing time step (min) = 10.00

----- DATA FOR SECTION ( 1.1) -----

Distance	Elevation	Manning	
0.00	274.12	0.0500	
5.20	273.17	0.0300	Main Channel
10.58	272.11	0.0300	Main Channel
11.50	272.00	0.0300	Main Channel
56.67	272.00	0.0300	Main Channel
64.61	272.44	0.0300	Main Channel
68.47	272.73	0.0300	Main Channel
73.75	273.06	0.0300 / 0.0500	Main Channel
79.53	273.29	0.0500	

----- TRAVEL TIME TABLE -----

DEPTH (m)	ELEV (m)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.07	272.07	104E+04	0.9	0.29	19.48
0.13	272.13	211E+04	2.8	0.46	12.41
0.20	272.20	322E+04	5.6	0.59	9.56
0.26	272.26	436E+04	9.1	0.71	7.96
0.33	272.33	554E+04	13.3	0.82	6.93
0.40	272.40	676E+04	18.2	0.91	6.19
0.46	272.46	801E+04	23.7	1.01	5.62
0.53	272.53	928E+04	30.0	1.09	5.16
0.60	272.60	106E+05	36.8	1.18	4.80
0.66	272.66	119E+05	44.2	1.26	4.49
0.73	272.73	133E+05	52.2	1.33	4.24
0.79	272.79	147E+05	60.7	1.40	4.03
0.86	272.86	161E+05	69.7	1.47	3.84
0.93	272.93	175E+05	79.3	1.54	3.68
0.99	272.99	190E+05	89.5	1.60	3.54
1.06	273.06	205E+05	100.3	1.66	3.41
1.14	273.14	223E+05	114.9	1.74	3.24
1.22	273.22	242E+05	130.7	1.83	3.09
1.29	273.29	262E+05	147.5	1.91	2.96

INFLOW: ID= 2 ( 0019)	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R.V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
127.59	14.667	1.00	28.07	0.27	3.28	0.29
OUTFLOW: ID= 1 ( 0026)	127.59	0.27	3.33	28.04	0.02	0.29

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VO output - Prop Controlled Upstream.txt

ADD HYD ( 0017 )	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 0142 ):	54.15	8.491	1.00	31.63
+ ID2= 2 ( 0026 ):	127.59	0.266	3.83	28.04
ID = 3 ( 0017 ):	181.74	8.511	1.00	29.11

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ROUTE CHN( 0015 ) IN= 2--> OUT= 1 Routing time step (min)' = 10.00

Distance	Elevation	Manning	Channel
0.00	252.95	0.0500	Main Channel
3.65	252.53	0.0300	Main Channel
12.66	251.31	0.0300	Main Channel
14.58	251.05	0.0300	Main Channel
76.79	251.08	0.0300	Main Channel
87.59	252.17	0.0300	Main Channel
90.94	252.44	0.0300 / 0.0500	Main Channel
98.65	253.00	0.0500	Main Channel

DEPTH (m)	ELEV (m)	TRAVEL TIME (min)	VOLUME (cu.m.)	FLOW RATE (cms)	VELOCITY (m/s)	TRAV. TIME (min)
0.10	251.15	671E+04	4.4	0.82	25.64	
0.20	251.25	148E+05	15.9	1.36	15.43	
0.30	251.35	230E+05	32.9	1.80	11.67	
0.40	251.45	315E+05	54.5	2.18	9.63	
0.49	251.55	402E+05	80.5	2.53	8.32	
0.59	251.64	491E+05	110.6	2.84	7.40	
0.69	251.74	582E+05	144.6	3.13	6.71	
0.79	251.84	676E+05	182.5	3.41	6.17	
0.89	251.94	771E+05	224.1	3.67	5.74	
0.99	252.04	869E+05	269.3	3.91	5.38	
1.09	252.14	969E+05	318.2	4.15	5.07	
1.19	252.24	107E+06	370.2	4.36	4.82	
1.29	252.34	118E+06	425.6	4.57	4.60	
1.39	252.44	128E+06	484.6	4.77	4.41	
1.49	252.54	140E+06	554.9	5.02	4.19	
1.59	252.64	151E+06	632.3	5.28	3.99	
1.69	252.75	163E+06	714.0	5.52	3.81	
1.80	252.85	176E+06	799.8	5.75	3.66	
1.90	252.95	188E+06	889.9	5.97	3.52	

INFLOW : ID= 2 ( 0017 )	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)	MAX DEPTH (m)	MAX VEL (m/s)
OUTFLOW : ID= 1 ( 0015 )	181.74	3.81	1.17	29.11	0.13	0.96
	181.74	3.81	1.17	29.11	0.09	0.82

RESERVOIR( 0020 ) IN= 2--> OUT= 1 DT= 10.0 min

OVERFLOW IS OFF

OUTFLOW (cms)	STORAGE (ha.m.)	OUTFLOW (cms)	STORAGE (ha.m.)
0.0000	0.0000	2.3900	1.8110
0.8500	1.4450	2.9700	1.8200
1.8300	1.7990	3.5000	1.8910
2.0100	1.8050	4.0000	1.9620
2.0200	1.8070	0.0000	0.0000

INFLOW : ID= 2 ( 0015 )	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
OUTFLOW : ID= 1 ( 0020 )	181.740	3.811	1.17	29.11
	181.740	0.751	3.17	29.11

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VO output - Prop Controlled Upstream.txt

PEAK FLOW REDUCTION [Qout/Qin] (%) = 19.70  
 TIME SHIFT OF PEAK FLOW (min) = 120.00  
 MAXIMUM STORAGE USED (ha.m.) = 1.2772

ADD HYD ( 0003 )	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 1421 ):	15.00	1.433	1.00	22.03
+ ID2= 2 ( 0151 ):	18.20	0.377	1.17	7.10
ID = 3 ( 0003 ):	33.20	1.665	1.00	13.85

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
3 + 2 = 1				
ID1= 3 ( 0003 ):	33.20	1.665	1.00	13.85
+ ID2= 2 ( 0002 ):	182.07	1.656	1.33	3.85
ID = 1 ( 0003 ):	215.27	2.644	1.33	5.39

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.

ADD HYD ( 0003 )	AREA (ha)	OPEAK (cms)	TPEAK (hrs)	R. V. (mm)
1 + 2 = 3				
ID1= 1 ( 0003 ):	215.27	2.644	1.33	5.39
+ ID2= 2 ( 0020 ):	181.74	0.751	3.17	29.11
ID = 3 ( 0003 ):	397.01	2.947	1.33	16.25

NOTE: PEAK FLOWS DO NOT INCLUDE BASEFLOWS IF ANY.



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# APPENDIX C

## Hydraulic Analysis and Floodplain Mapping Updates

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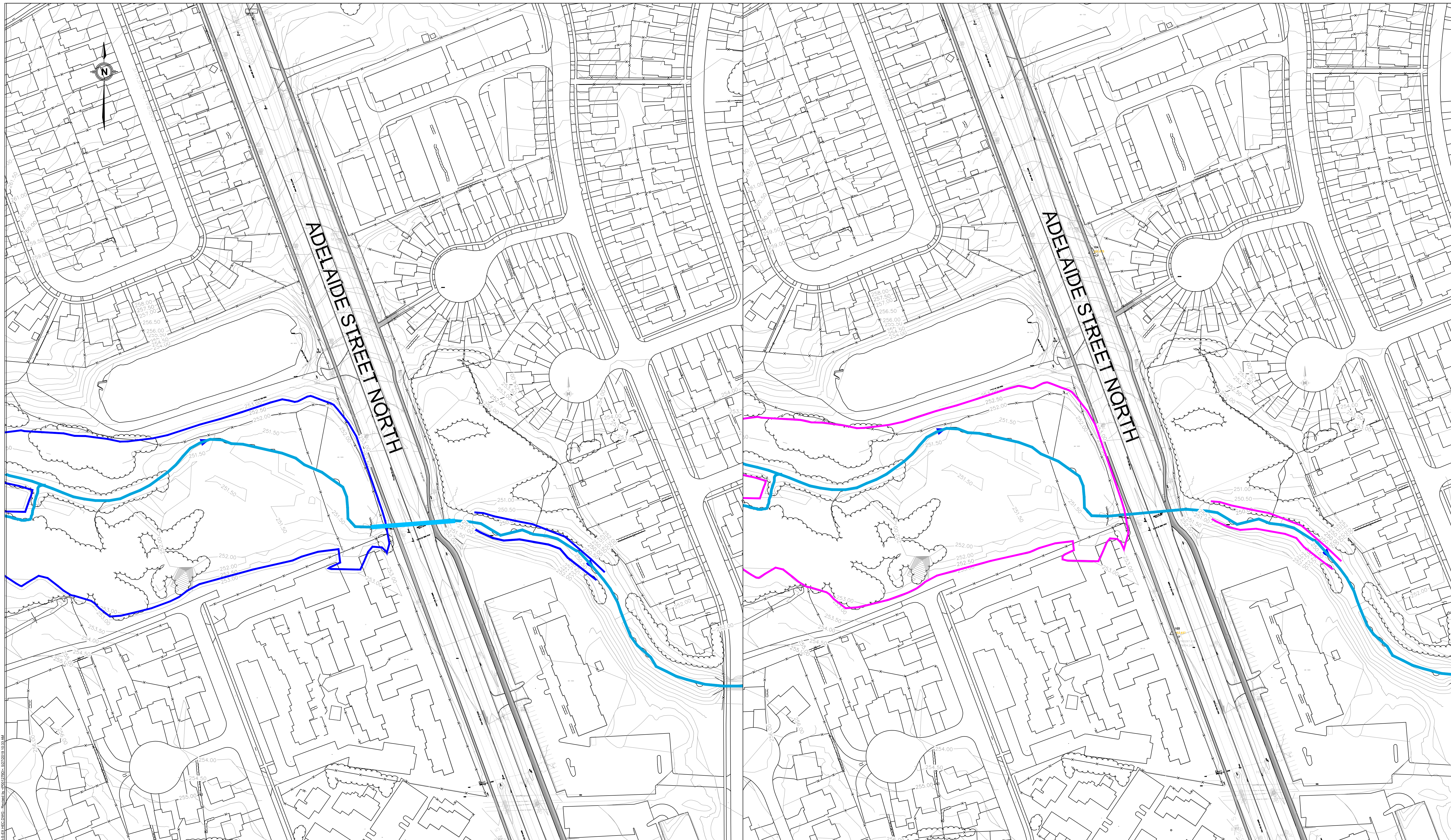
## **APPENDIX C: Hydraulic Analysis**

Figure C.1 : Floodplain Map






HEC-RAS Result: Water Profile

Cross Sections Upstream and Downstream of the Adelaide Culvert


HEC-RAS Result Table



**LEGEND**

-  PROPERTY LINE
-  100 YEAR STORM EVENT FLOODLINE
-  250 YEAR REGULATORY FLOODLINE
-  CREEK FLOW DIRECTION
-  EXISTING CULVERT

SCALE HORIZONTAL  
1:1000



DATE: MAY 2019  
SCALE: 1 : 1000

**PARSONS**

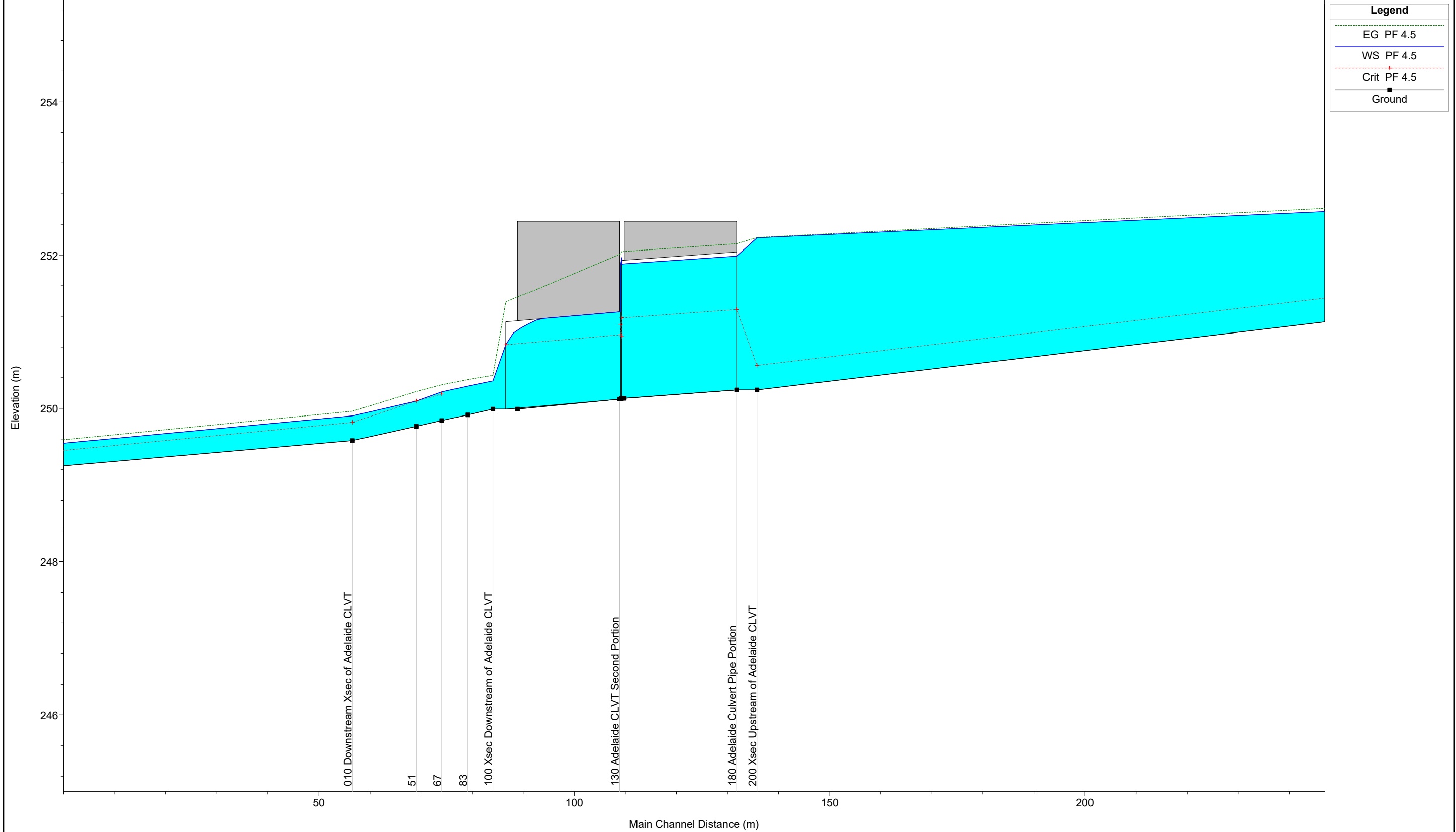
625 COCHRANE DRIVE, SUITE 500  
MARKHAM, ONTARIO L3R 9R9  
TEL: 905-943-0500  
FAX: 905-943-0400

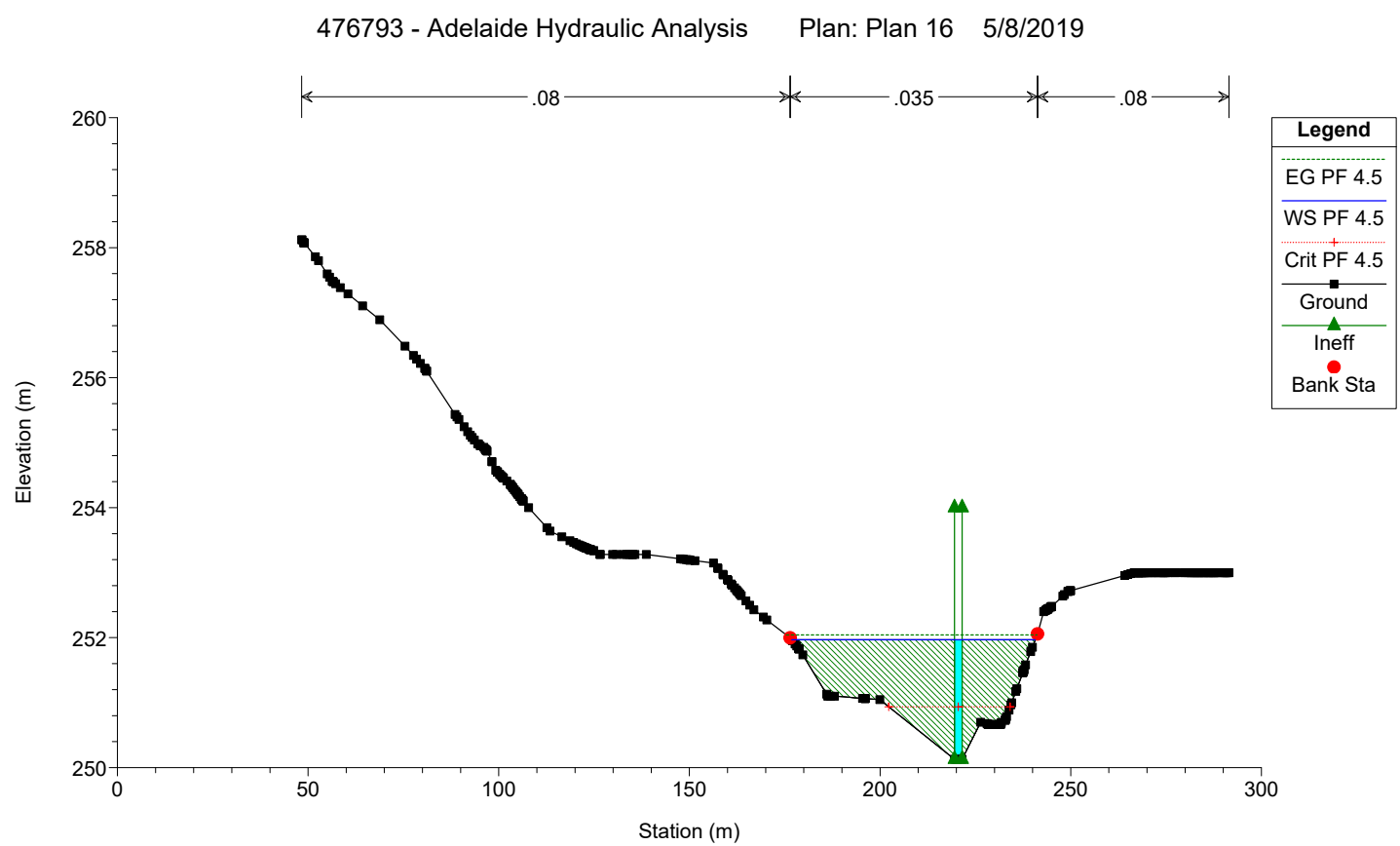
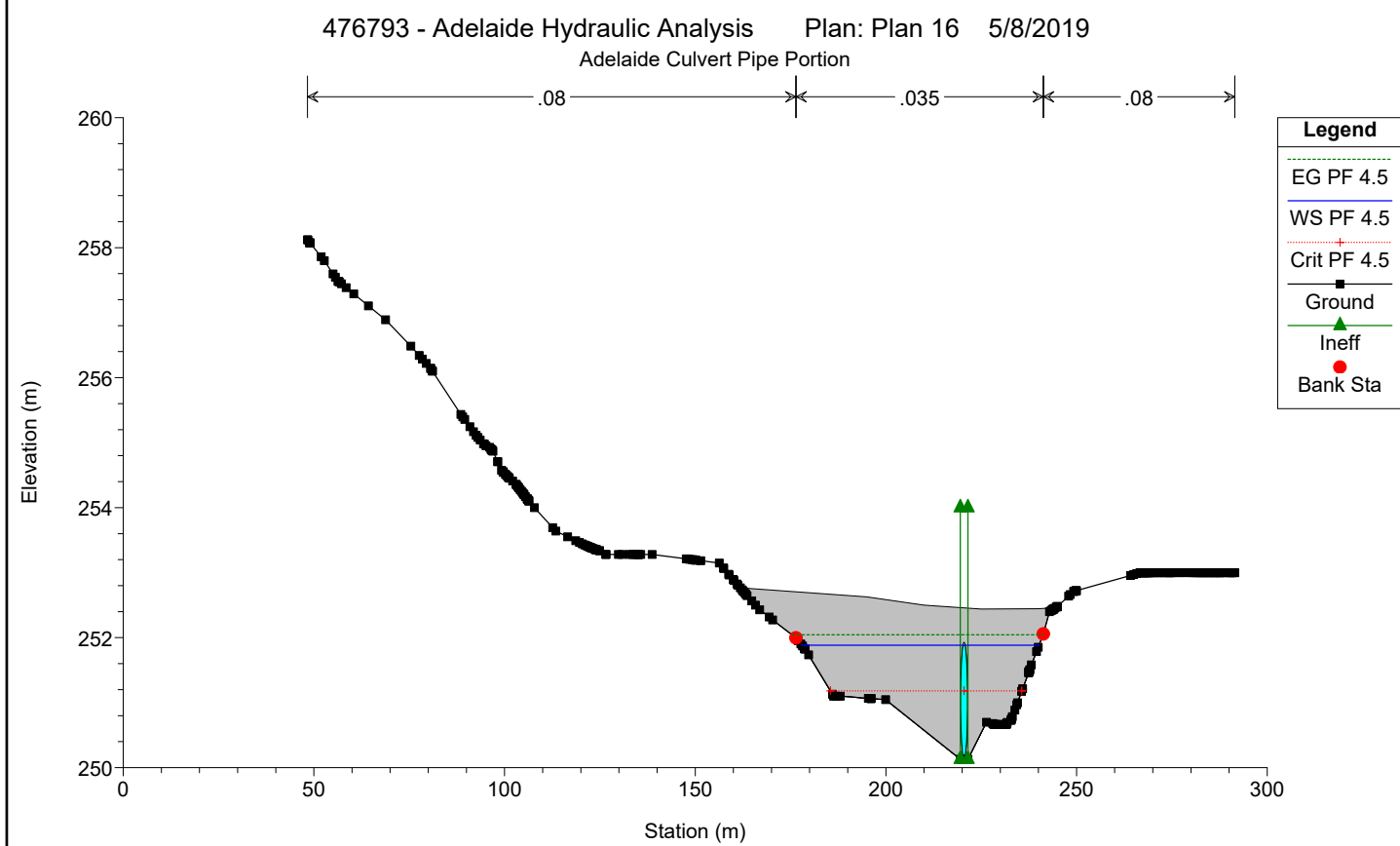
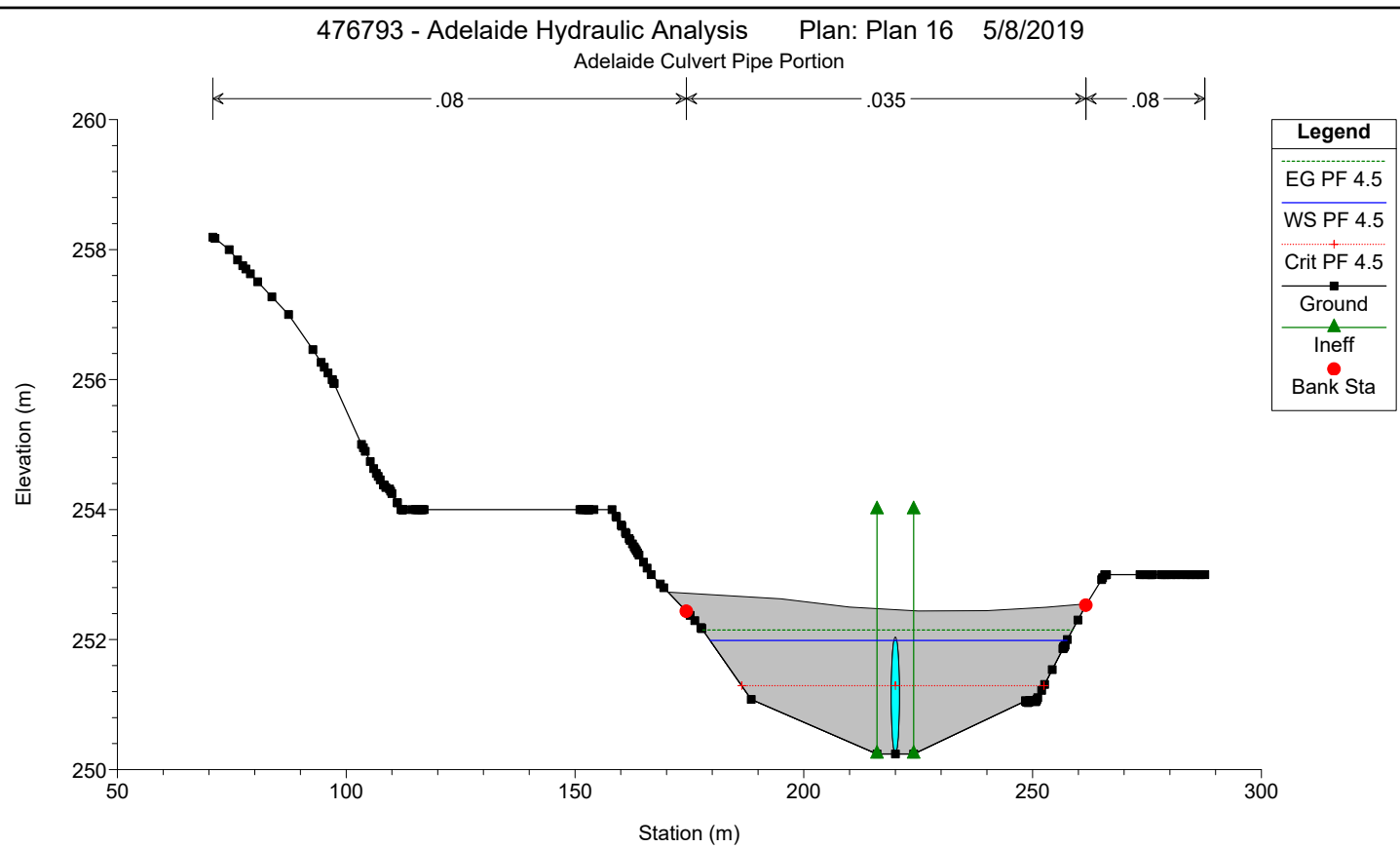
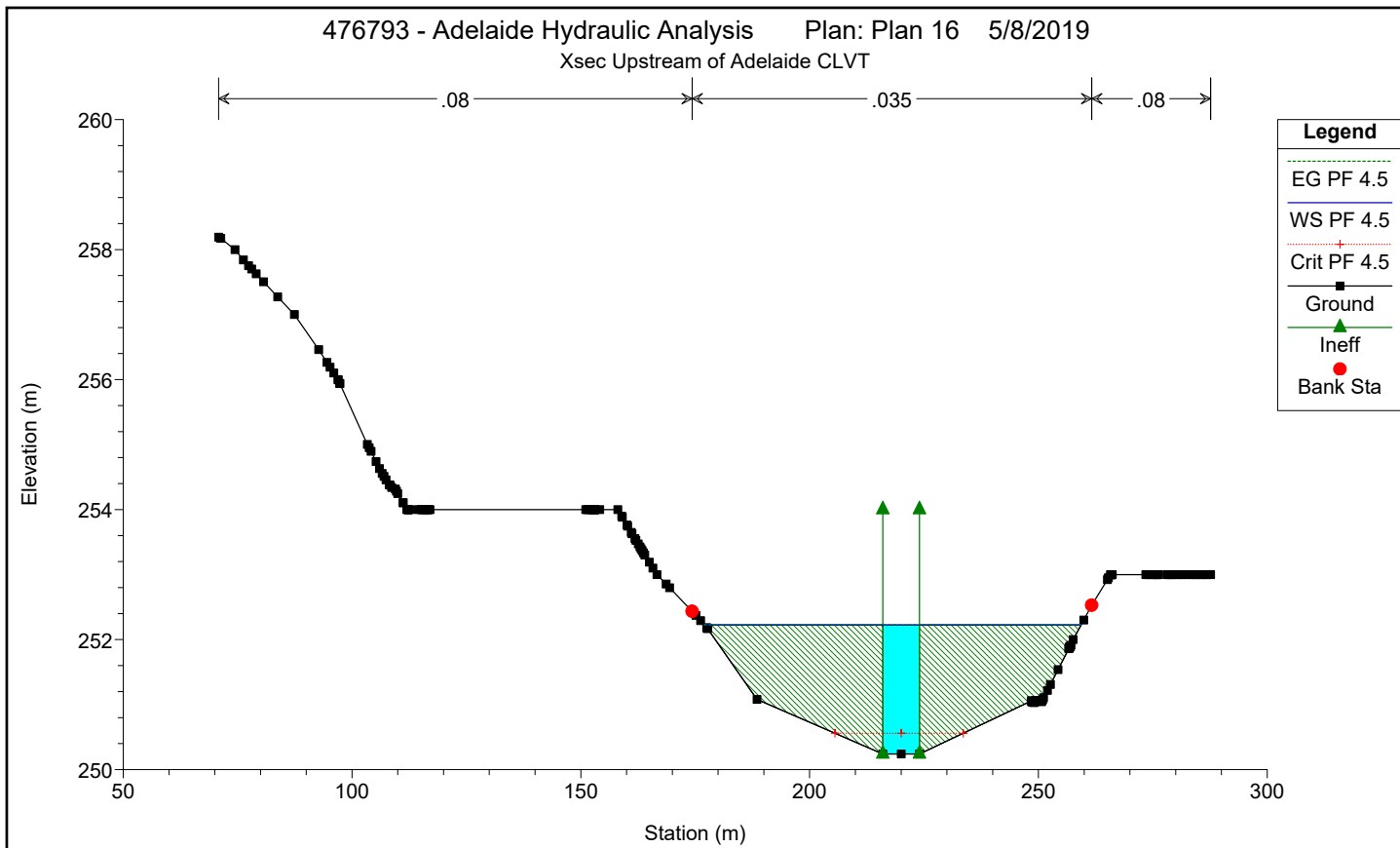
ADELAIDE ST N FROM SUNNINGDALE ST TO FANSHAWE PARK RD

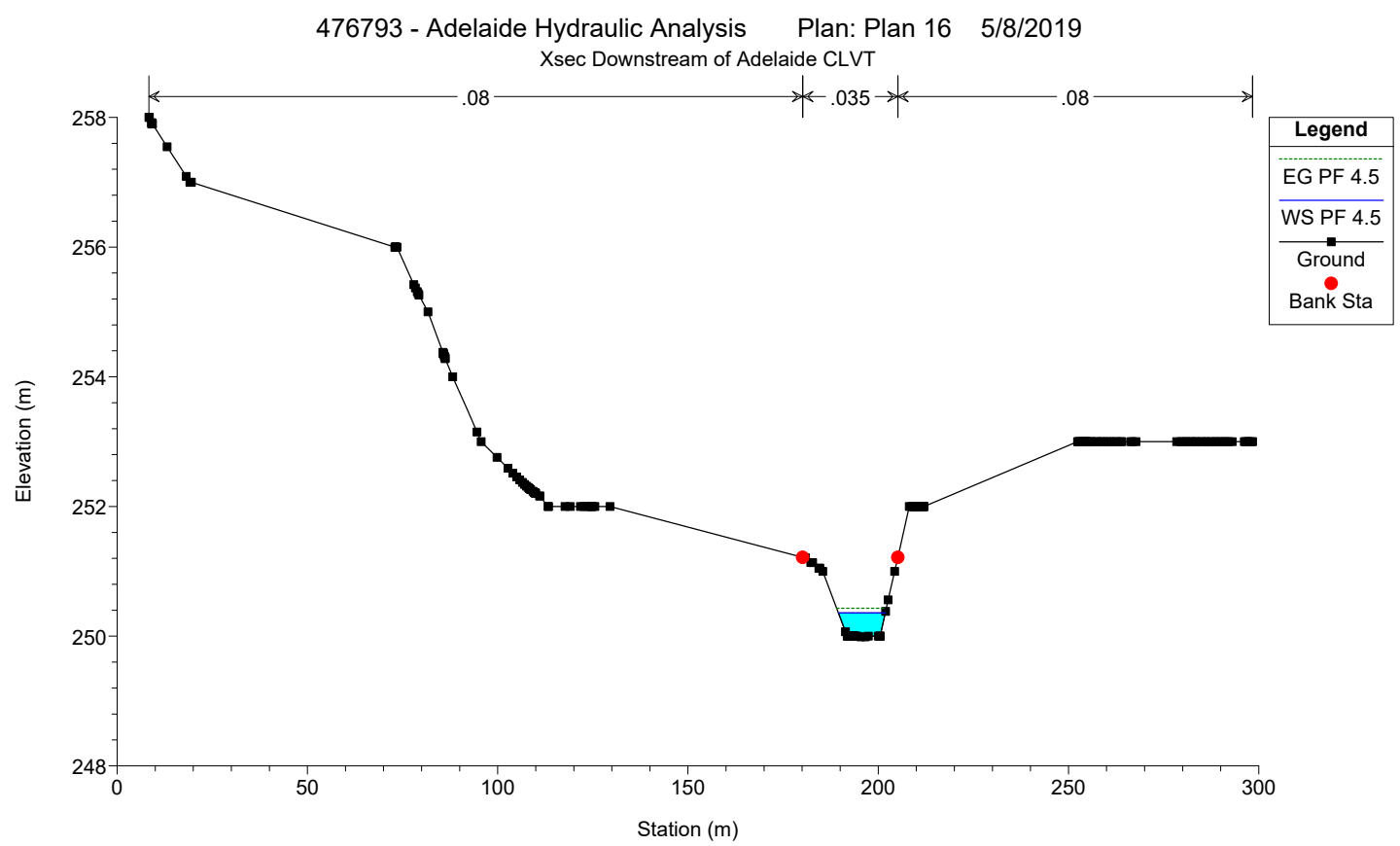
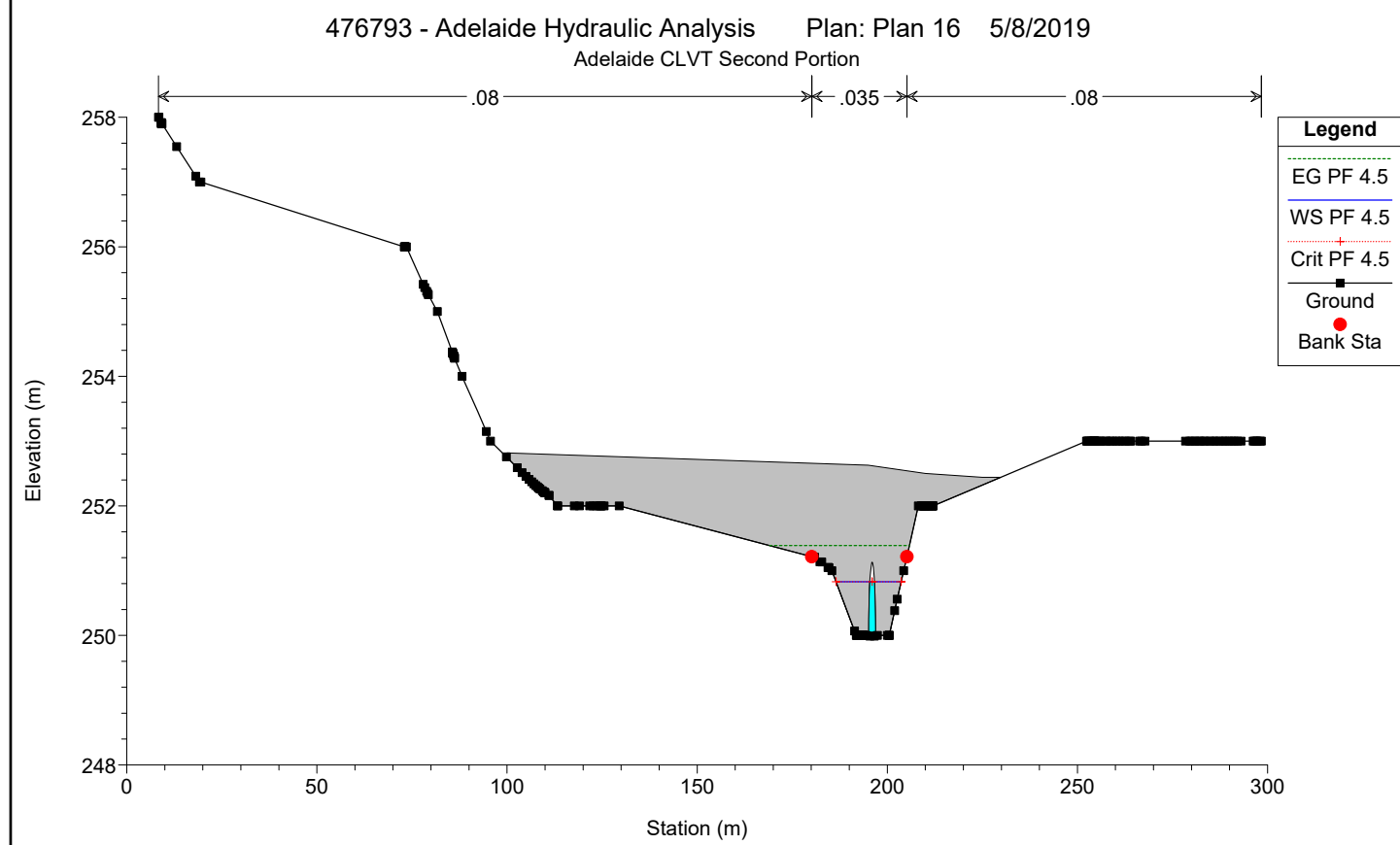
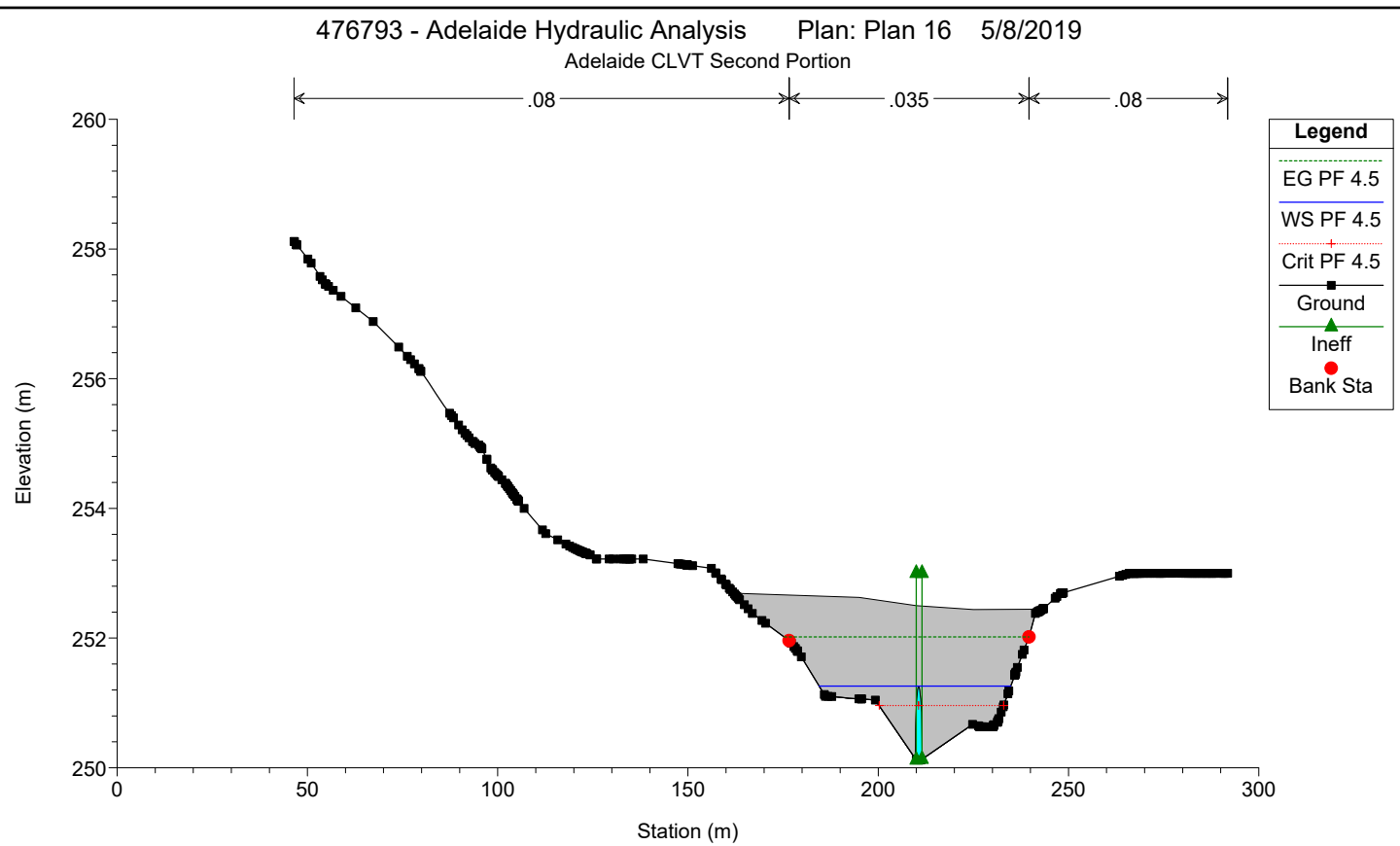
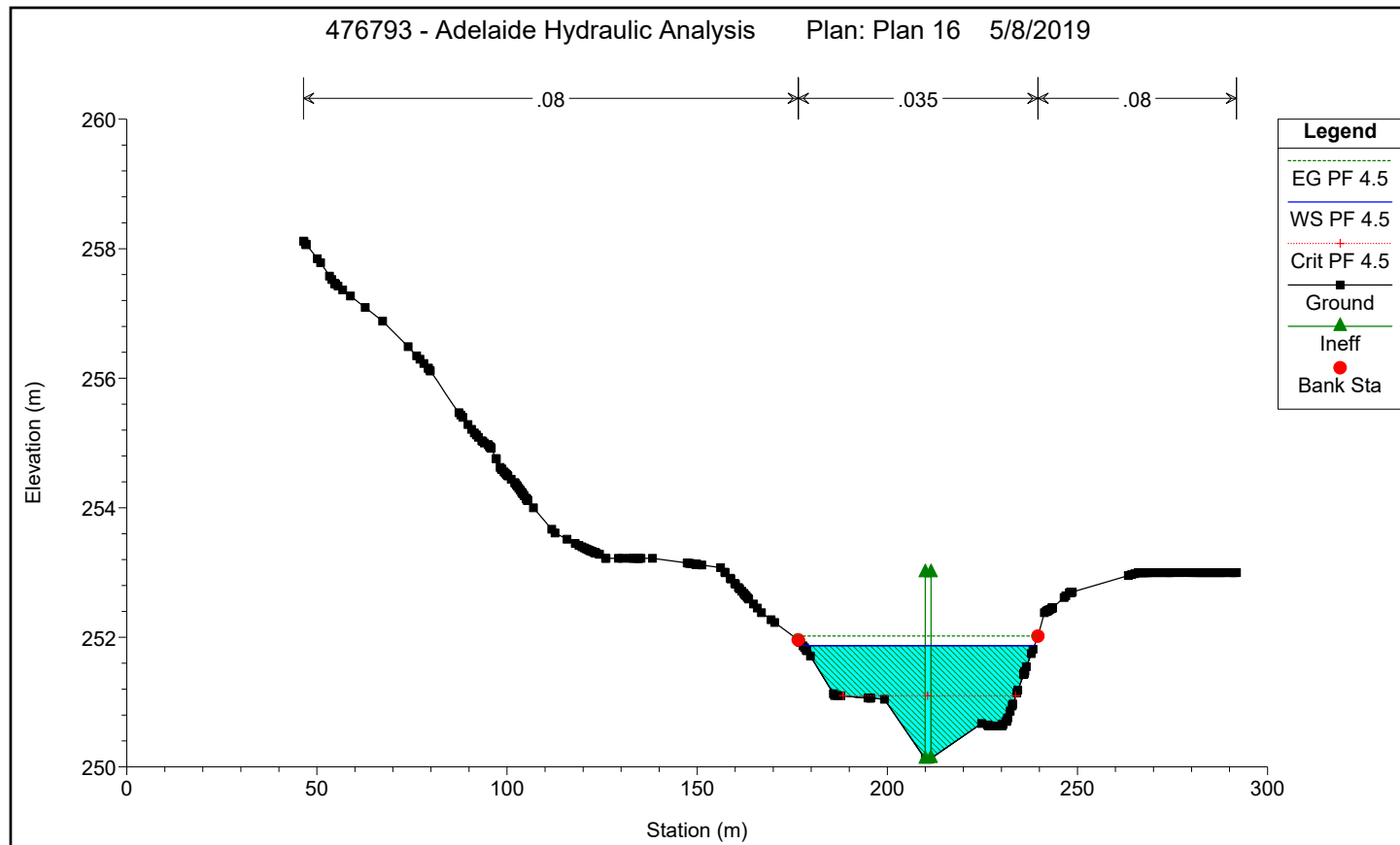
FIGURE C.1 : FLOODPLAIN MAP - POWELL DRAIN

## **HEC-RAS Results**

Powell Drain Powell Reach







HEC-RAS Plan: Plan 16 Locations: User Defined Profile: PF 4.5

River	Reach	River Sta	Profile	Q Total (m3/s)	Min Ch El (m)	W.S. Elev (m)	Crit W.S. (m)	E.G. Elev (m)	E.G. Slope (m/m)	Vel Chnl (m/s)	Flow Area (m2)	Top Width (m)	Froude # Chl
Powell Drain	Powell Reach	200	PF 4.5	4.50	250.24	252.23	250.56	252.23	0.000039	0.28	15.88	82.34	0.06
Powell Drain	Powell Reach	180		Culvert									
Powell Drain	Powell Reach	160	PF 4.5	4.50	250.13	251.97	250.94	252.04	0.000817	1.23	3.67	63.86	0.29
Powell Drain	Powell Reach	150	PF 4.5	4.50	250.12	251.87	251.10	252.02	0.001709	1.71	2.62	60.85	0.41
Powell Drain	Powell Reach	130		Culvert									
Powell Drain	Powell Reach	100	PF 4.5	4.50	249.99	250.36		250.43	0.008473	1.19	3.78	12.31	0.69



HEC-RAS Plan: Plan 19 Locations: User Defined Profile: PF 4.5

River	Reach	River Sta	Profile	Q Total (m3/s)	Min Ch El (m)	W.S. Elev (m)	Crit W.S. (m)	E.G. Elev (m)	E.G. Slope (m/m)	Vel Chnl (m/s)	Flow Area (m2)	Top Width (m)	Froude # Chl
Powell Drain	Powell Reach	100	PF 4.5	4.50	249.99	250.36		250.43	0.008473	1.19	3.78	12.31	0.69
Powell Drain	Powell Reach	83	PF 4.5	4.50	249.92	250.29		250.38	0.011482	1.29	3.48	12.64	0.79
Powell Drain	Powell Reach	67	PF 4.5	4.50	249.84	250.22	250.19	250.31	0.013572	1.35	3.34	12.88	0.85
Powell Drain	Powell Reach	51	PF 4.5	4.50	249.77	250.10	250.10	250.22	0.019612	1.56	2.88	11.73	1.00
Powell Drain	Powell Reach	010	PF 4.5	4.50	249.58	249.90	249.82	249.96	0.007089	1.09	4.13	13.28	0.62
Powell Drain	Powell Reach	001	PF 4.5	4.50	249.25	249.54	249.45	249.59	0.005901	0.95	4.76	16.62	0.56

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# APPENDIX D

## Storm Sewer Analysis

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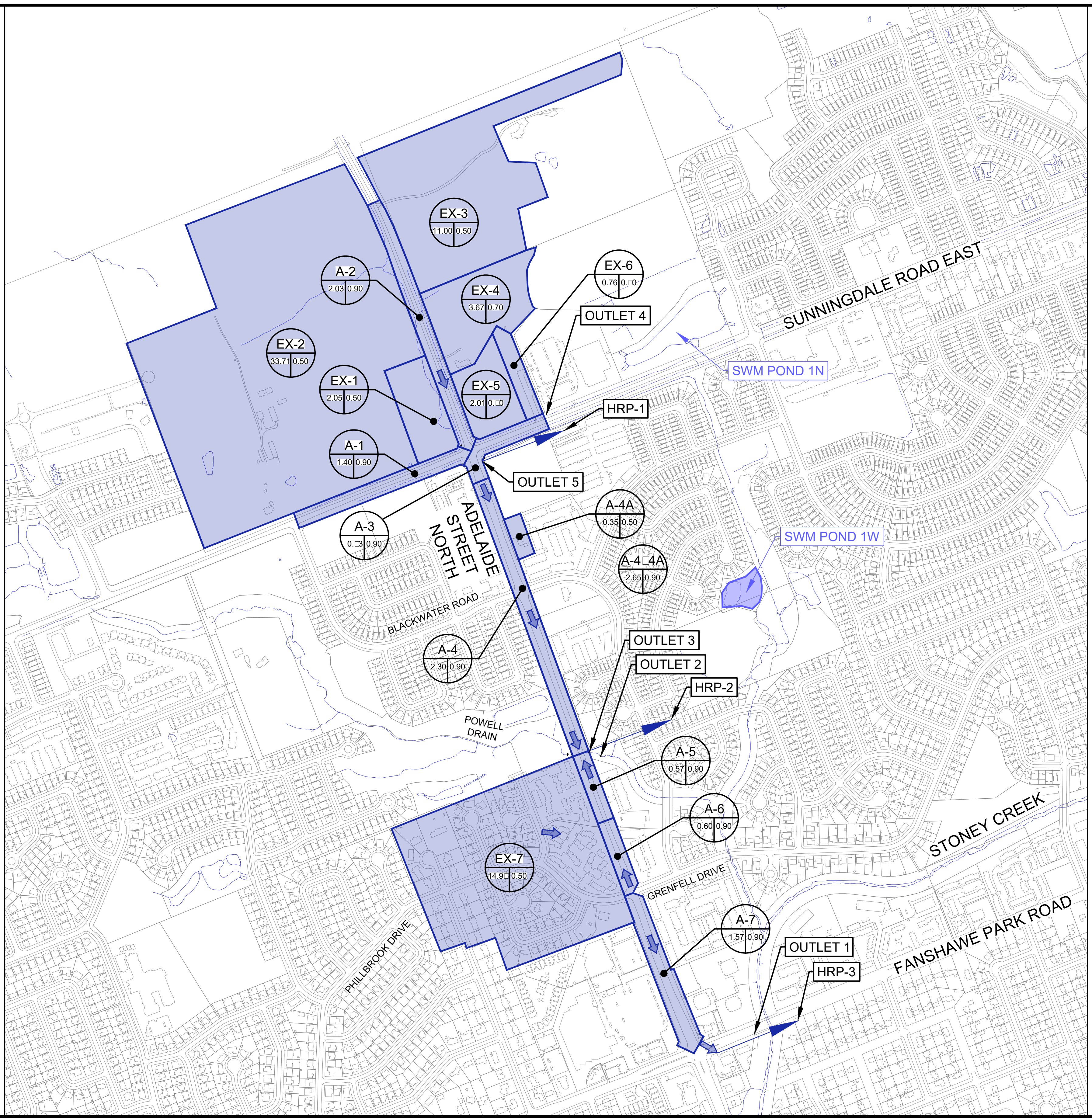
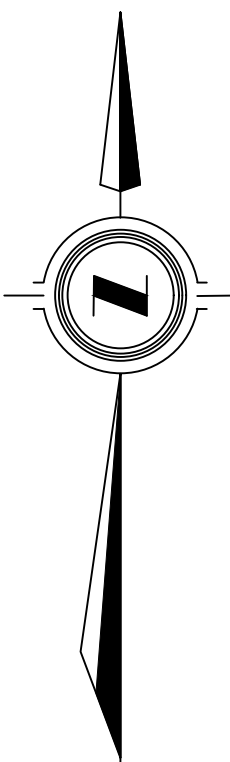
## APPENDIX D: Storm Sewer Analysis

### Figures:

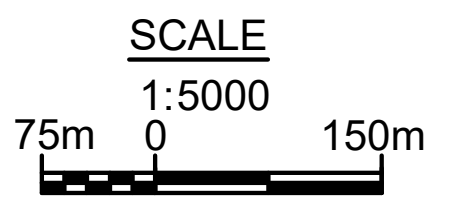
- Figure D1: PROPOSED STORM SEWER DRAINAGE AREAS
- Figure D2: EXSITING STORM SEWER
- Figure D3: PROPOSED STORM SEWER

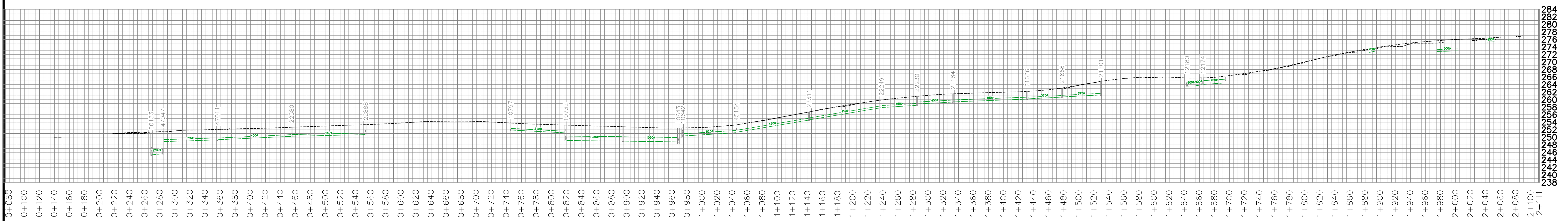
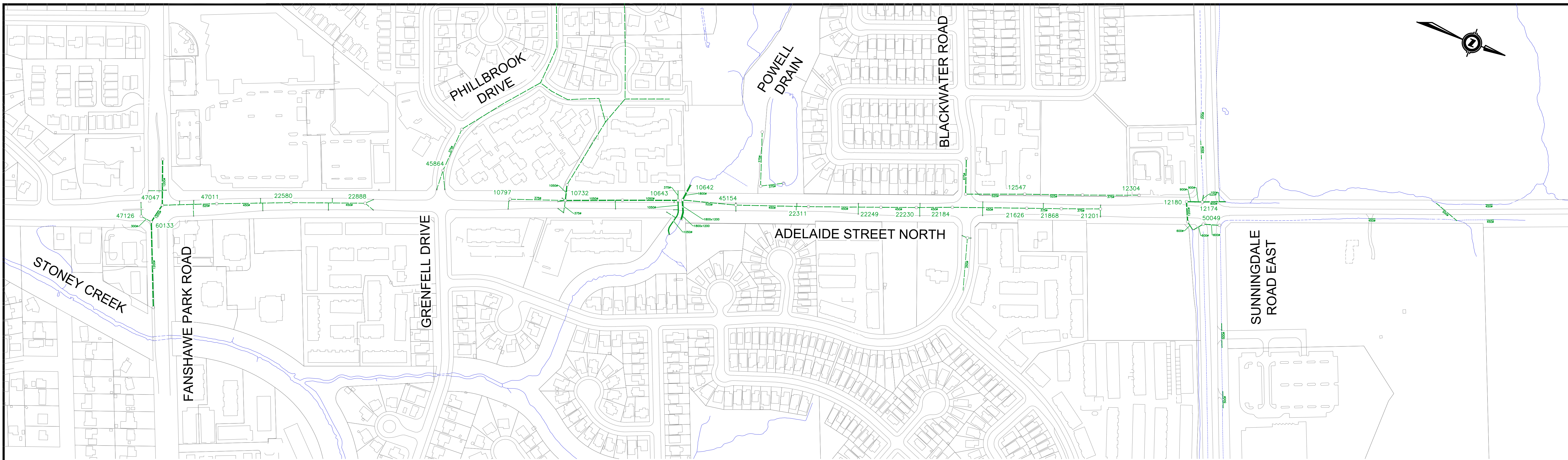
### Tables:

- Capacity of Existing Strom Sewer Outlets
- Time of Concentration (Tc) Calculations for Existing Storm Sewer Outlets
- Conceptual Design of proposed Strom Sewers

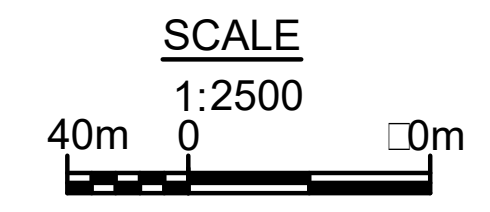


ID
A C

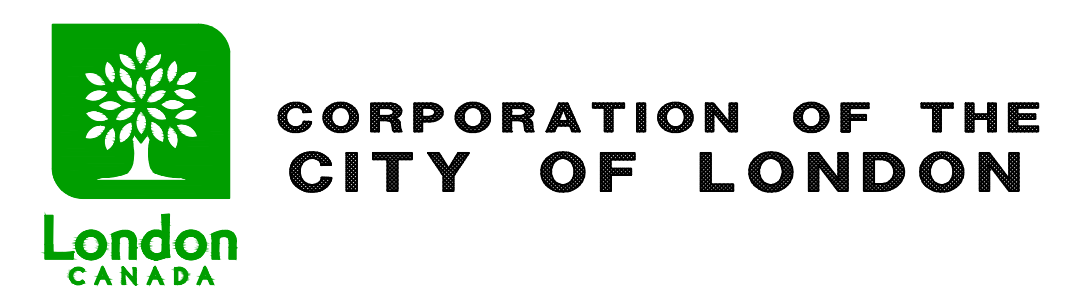




ADELAIDE STREET NORTH PROFILE

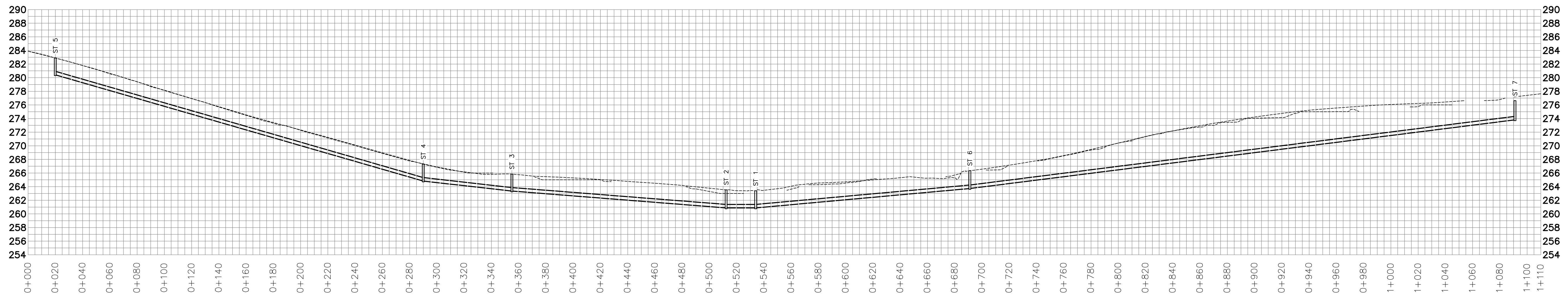
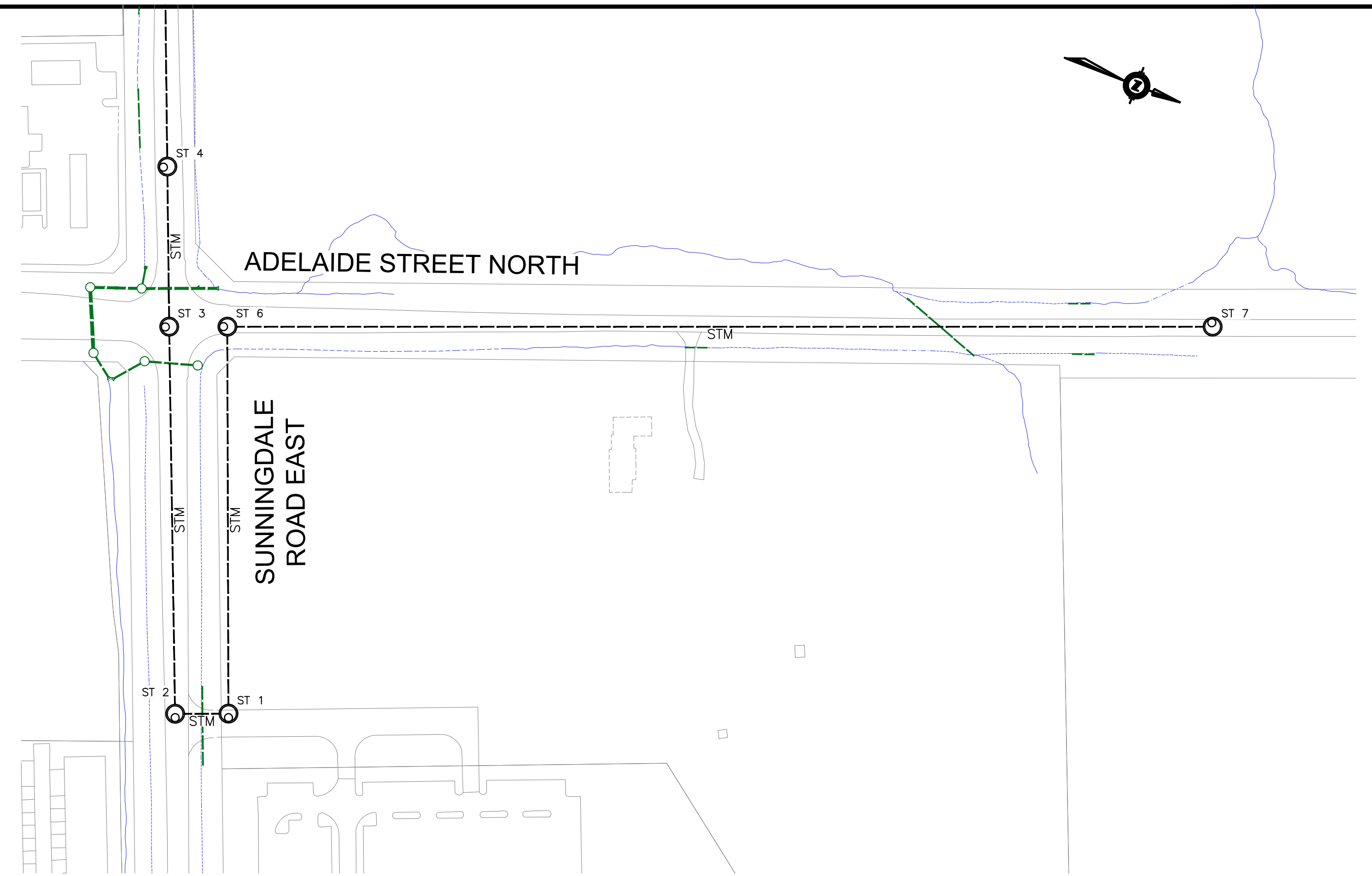
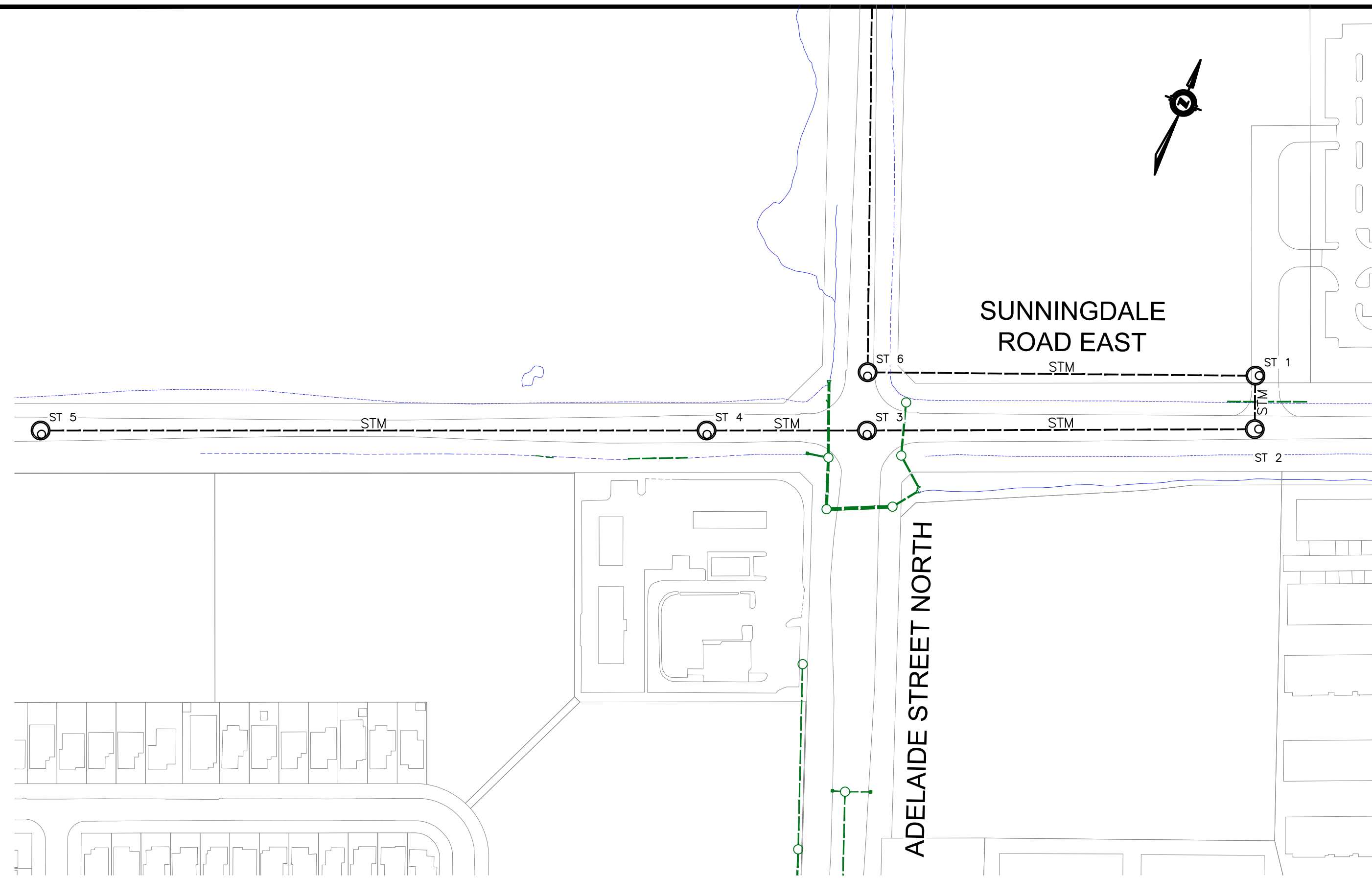


**ADELAIDE STREET NORTH - SWM REPORT**  
(FANSHAWE ROAD TO SUNNINGDALE ROAD)  
**EXISTING STORM SEWER**

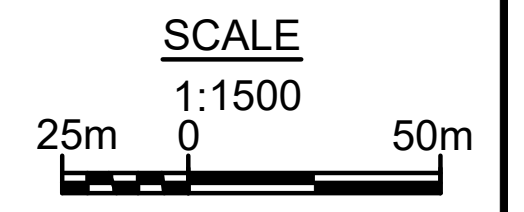


**FIGURE D-2**

17, 2019



PROFILE ALONG PROPOSED STORM PROFILE



**Capacity of Existing Storm Sewer Outlets**

**Table #1**

AREA NO.	LOCATION			ACCUMULATED STORMWATER FLOWS											SEWER DESIGN									
	STREET	NODE		INCR. AREA (ha)	ACCUM. AREA (ha)	C	INCR. AxC	TOT. SECT. AxC	TOTAL LATERAL AxC	TOT. SWR AxC	TOTAL AxCx2.78	TIME OF SECT. (min)	ACCUM. TIME (min)	INTENSITY I	PEAK FLOWS (l/s)	PIPE DIA. (mm)	SLOPE %	n	CAPACITY (l/s)	VELOCITY (m/s)	LENGTH (m)	TIME (min)	LOSSES (m)	FALL IN SEWER (m)
		FROM	TO																					
<b>OUTLET 1 - PHILBROOK DR. TO FANSHAWE PARK RD. (INFO FROM CITY AS-BUILT #15806)</b>																								
A7	ADELAIDE STREET	Ex MH 2U-167	Ex MH 2U-163	1.580	1.580	0.83	1.311	1.311	0.000	1.311	3.646	n/a	12.85	93	339.554	525	0.62	0.013	338.632	1.564	72.80	0.78	0.000	0.451
<b>OUTLET 2 - PHILBROOK DR. TO POWELL DRAIN (INFO FROM CITY AS-BUILTS #9893, 9901, 20027, 20028 &amp; 20029)</b>																								
A6a	ADELAIDE STREET	Ex MH 3V-240	Ex MH 3V-42	0.530	0.530	0.90	0.477	0.477	0.000	0.477	1.326	n/a	13.50	91	120.298	375	1.05	0.013	179.660	1.627	73.00	0.75	0.000	0.767
Ex 2	EASEMENT	Ex MH 3V-41	Ex MH 3V-42	14.260	14.260	0.50	7.130	7.607	0.000	7.607	21.147	0.75	27.17	59	1244.536	1050	0.21	0.013	1251.378	1.445	23.00	0.27	0.000	0.048
A6b	ADELAIDE STREET	Ex MH 3V-42	Ex MH 3V-44	0.570	15.360	0.90	0.513	8.120	0.000	8.120	22.574	0.27	27.43	58	1317.377	1050	0.24	0.013	1323.771	1.529	146.90	1.60	0.000	0.345
	EASEMENT	Ex MH 3V-44	Ex 3V-5004	0.000	15.360	0.90	0.000	8.120	0.000	8.120	22.574	1.60	29.03	56	1260.293	1050	0.33	0.013	1568.685	1.812	47.20	0.43	0.000	0.156
<b>OUTLET 3 - SUNNINGDALE RD. TO POWELL DRAIN (INFO FROM CITY AS-BUILTS #20027, 20028 &amp; 20029)</b>																								
A5	ADELAIDE STREET	Ex MH 3V-242	Ex 3V-241	2.700	2.700	0.76	2.052	2.052	0.000	2.052	5.705	n/a	15.61	84	477.387	525	1.24	0.013	478.898	2.212	71.10	0.54	0.000	0.882

**Time of Concentration (T<sub>c</sub>) Calculations for Existing Storm Sewer Outlets**

**Table #2**

LOCATION			SEWER DESIGN								
STREET	NODE		TIME OF SECT. (min)	ACCUM. TIME (min)	PIPE DIA. (mm)	SLOPE %	n	CAPACITY (l/s)	VELOCITY (m/s)	LENGTH (m)	TIME (min)
	FROM	TO									
<b>OUTLET 1 - PHILBROOK DR. TO FANSHAWE PARK RD. (INFO FROM CITY AS-BUILT #15806)</b>											
ADELAIDE STREET	Ex MH 2U-165	Ex MH 2U-166	n/a	10.50	450	0.44	0.013	189.118	1.189	98.70	1.38
ADELAIDE STREET	Ex MH 2U-166	Ex MH 2U-167	1.38	11.88	450	0.92	0.013	273.465	1.719	99.30	0.96
			0.96	12.85							
<b>OUTLET 2 - PHILBROOK DR. TO POWELL DRAIN (INFO FROM CITY AS-BUILTS #9893, 9901, 20027, 20028 &amp; 20029)</b>											
ADELAIDE STREET	Ex MH 3V-240	Ex MH 3V-42	n/a	11.00	375	1.05	0.013	179.660	1.627	90.40	0.93
STONEBROOK CR./EASEMENT	Ex MH	Ex MH 3V-29	0.93	19.00	250	0.50	0.013	42.050	0.857	67.40	1.31
EASEMENT	Ex MH 3V-29	Ex MH 3V-30	1.31	20.31	300	0.50	0.013	68.378	0.967	69.50	1.20
EASEMENT	Ex MH 3V-30	Ex MH 3V-32	1.20	21.51	375	0.50	0.013	123.977	1.123	138.80	2.06
EASEMENT	Ex MH 3V-32	Ex MH 3V-35	2.06	23.57	450	0.50	0.013	201.601	1.268	37.00	0.49
BYBROOK CR./EASEMENT	Ex MH 3V-35	Ex MH 3V-39	0.49	24.06	525	0.50	0.013	304.100	1.405	130.30	1.55
EASEMENT	Ex MH 3V-39	Ex MH 3V-40	1.55	25.60	600	0.50	0.013	434.172	1.536	41.70	0.45
EASEMENT	Ex MH 3V-40	Ex MH 3V-41	0.45	26.05	1050	0.21	0.013	1251.378	1.445	96.50	1.11
EASEMENT	Ex MH 3V-41	Ex MH 3V-42	1.11	27.17	1050	0.25	0.013	1365.365	1.577	23.00	0.24
<b>OUTLET 3 - SUNNINGDALE RD. TO POWELL DRAIN (INFO FROM CITY AS-BUILTS #20027, 20028 &amp; 20029)</b>											
ADELAIDE STREET	Ex MH 3V-151	Ex MH 3V-150	n/a	11.74	375	1.02	0.013	177.075	1.603	52.20	0.54
ADELAIDE STREET	Ex MH 3V-150	Ex MH 3V-149	0.54	12.28	375	1.02	0.013	177.075	1.603	46.00	0.48
ADELAIDE STREET	Ex MH 3V-149	Ex MH 3V-246	0.48	12.76	450	0.76	0.013	248.550	1.563	98.90	1.05
ADELAIDE STREET	Ex MH 3V-246	Ex MH 3V-245	1.05	13.82	450	1.13	0.013	303.072	1.906	48.50	0.42
ADELAIDE STREET	Ex MH 3V-245	Ex MH 3V-244	0.42	14.24	450	1.25	0.013	318.759	2.004	46.30	0.39
ADELAIDE STREET	Ex MH 3V-244	Ex MH 3V-243	0.39	14.62	450	3.35	0.013	521.831	3.281	96.60	0.49
ADELAIDE STREET	Ex MH 3V-243	Ex MH 3V-242	0.49	15.12	450	3.35	0.013	521.831	3.281	96.40	0.49
			0.49	15.61							



**Conceptual Design of Proposed Storm Sewers**

**Table #3**

AREA NO.	LOCATION			ACCUMULATED STORMWATER FLOWS												SEWER DESIGN								PROFILE			
	STREET	NODE		INCR. AREA (ha)	ACCUM. AREA (ha)	C	INCR. AxC	TOT. SECT. AxC	TOTAL LATERAL AxC	TOT. SWR AxC	TOTAL AxCx2.78	TIME OF SECT. (min)	ACCUM. TIME (min)	INTENSITY I	PEAK FLOWS (l/s)	PIPE DIA. (mm)	SLOPE %	n	CAPACITY (l/s)	VELOCITY (m/s)	LENGTH (m)	TIME (min)	LOSSES (m)	DROP IN NODE (m)	FALL IN SEWER (m)	INVERT ELEV. U/S	INVERT ELEV. D/S
		FROM	TO																								
A1	SUNNINGDALE ROAD	ST-5	ST-4	1.39	1.39	0.90	1.25	1.25	0.00	1.25	3.48	n/a	10.50	102	355.47	375	6.00	0.013	429.47	3.89	270.00	1.16	0.00	n/a	16.20	280.20	264.00
	SUNNINGDALE ROAD	ST-4	ST-3	0.00	1.39	0.90	0.00	1.25	1.00	2.25	6.26	n/a	11.50	98	613.78	450	1.25	0.013	318.76	2.00	65.00	0.54	0.00	0.08	0.81	263.92	263.11
A2	SUNNINGDALE ROAD	ST-3	ST-2	2.03	3.42	0.90	1.83	3.08	0.00	3.08	8.56	0.54	12.04	96	821.86	600	1.80	0.013	823.78	2.91	157.00	0.90	0.00	0.15	2.83	262.96	260.13
	SUNNINGDALE ROAD	ST-2	ST-1	0.00	3.42	0.50	0.00	3.08	0.00	3.08	8.56	0.90	12.94	93	794.01	675	1.00	0.013	840.59	2.35	21.50	0.15	0.00	0.08	0.22	260.13	259.92
A2	ADELAIDE STREET	ST-7	ST-6	2.03	2.03	0.90	1.83	1.83	0.00	1.83	5.08	n/a	23.12	66	335.51	1200	2.25	0.013	5848.11	5.17	0.00	0.00	0.00	0.00	0.00	272.13	272.13
Ex2	ADELAIDE STREET	ST-7	ST-6	33.38	35.41	0.50	16.69	18.52	0.00	18.52	51.48	0.00	23.12	66	3400.50	1200	2.25	0.013	5848.11	5.17	0.00	0.00	0.00	0.00	0.00	272.13	272.13
Ex3	ADELAIDE STREET	ST-7	ST-6	11.06	46.47	0.50	5.53	24.05	0.00	24.05	66.85	0.00	23.12	66	4416.03	1200	2.25	0.013	5848.11	5.17	400.00	1.29	0.00	0.10	9.00	272.13	263.13
Ex4	SUNNINGDALE ROAD	ST-6	ST-1	3.80	50.27	0.70	2.66	26.71	0.00	26.71	74.25	1.29	24.41	64	4722.87	1200	2.25	0.013	5848.11	5.17	0.00	0.00	0.00	0.00	0.00	263.03	263.03
Ex5	SUNNINGDALE ROAD	ST-6	ST-1	2.05	52.32	0.80	1.64	28.35	0.00	28.35	78.80	0.00	24.41	64	5012.89	1200	2.25	0.013	5848.11	5.17	157.00	0.51	0.00	0.10	3.53	263.03	259.49
Ex6	SUNNINGDALE ROAD	ST-1	Ex MH 4U-18S	0.84	56.58	0.80	0.67	32.10	0.00	32.10	89.23	0.51	24.92	63	5593.18	1200	2.18	0.013	5756.42	5.09	47.90	0.16	0.00	0.53	1.04	259.39	258.35

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# **APPENDIX E**

## **Stormwater Management Quantity and Quality Control**

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## **APPENDIX E: Quantity and Quality Control**

### **Bio-Swales**

### **Stormceptor**

Detailed Stormceptor Sizing Report - Outlet 1

Detailed Stormceptor Sizing Report - Outlet 2

Detailed Stormceptor Sizing Report - Outlet 3

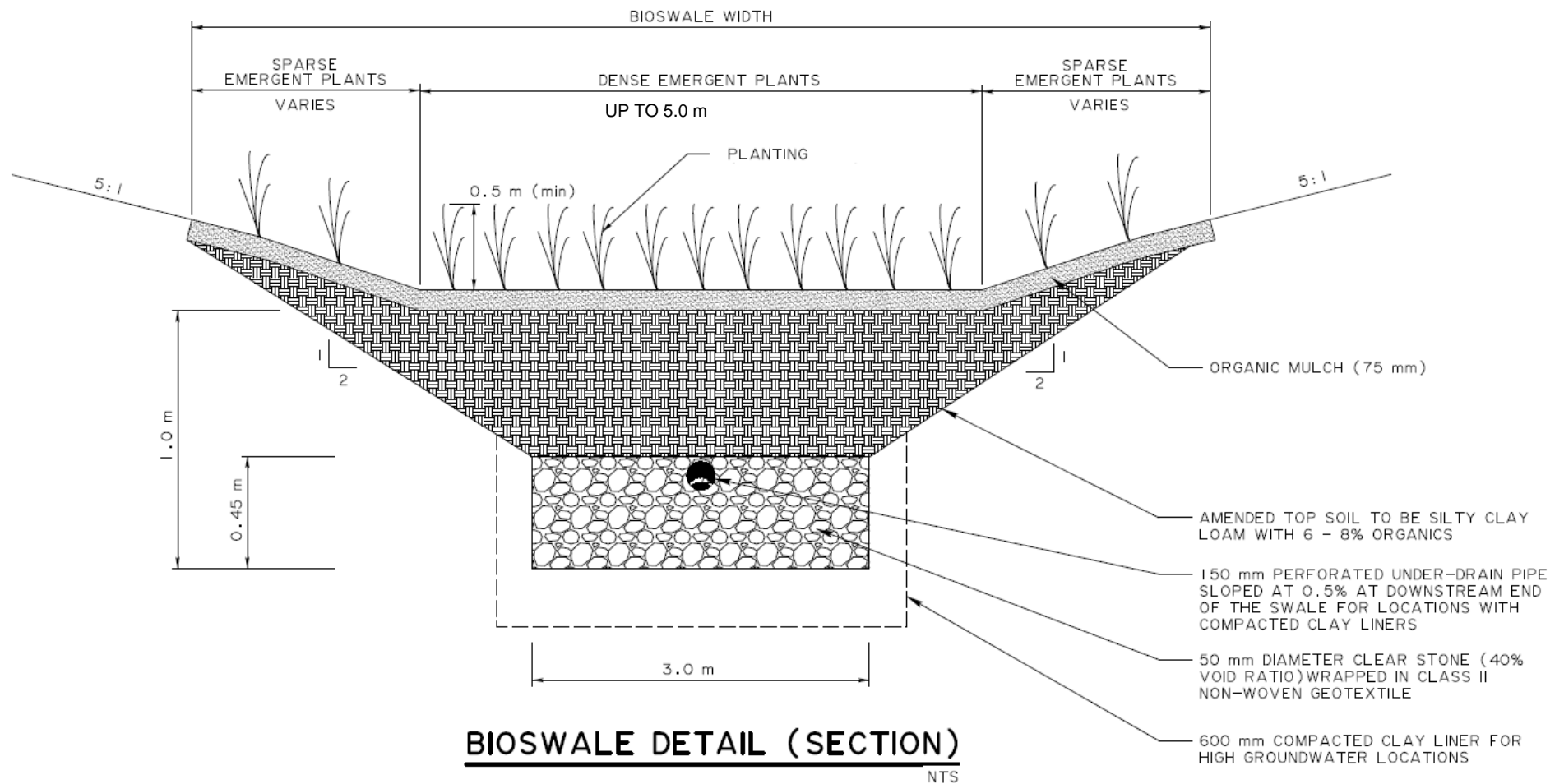
Detailed Stormceptor Sizing Report - Outlet 4

Stormceptor OSR Product Sheet

OSR 300 Drawing and Specification

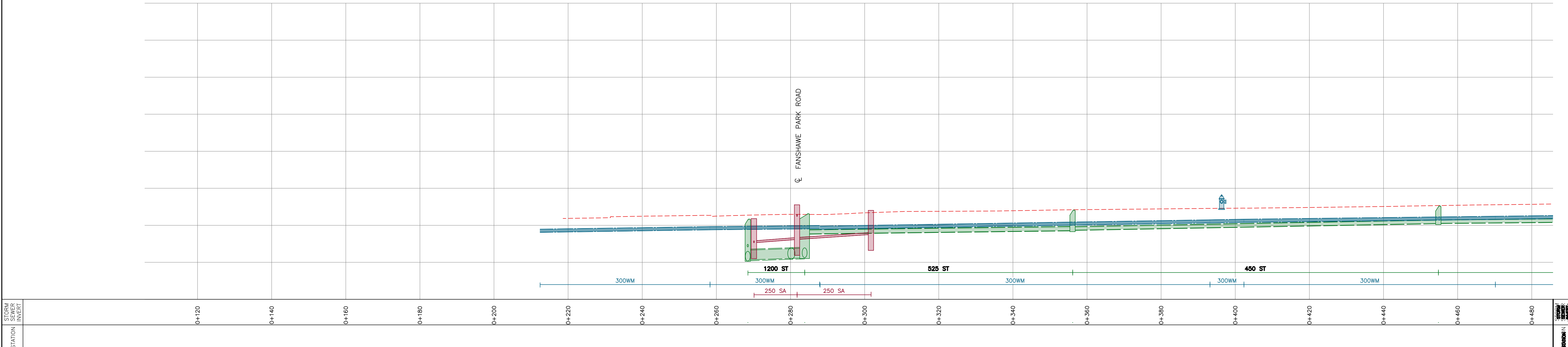
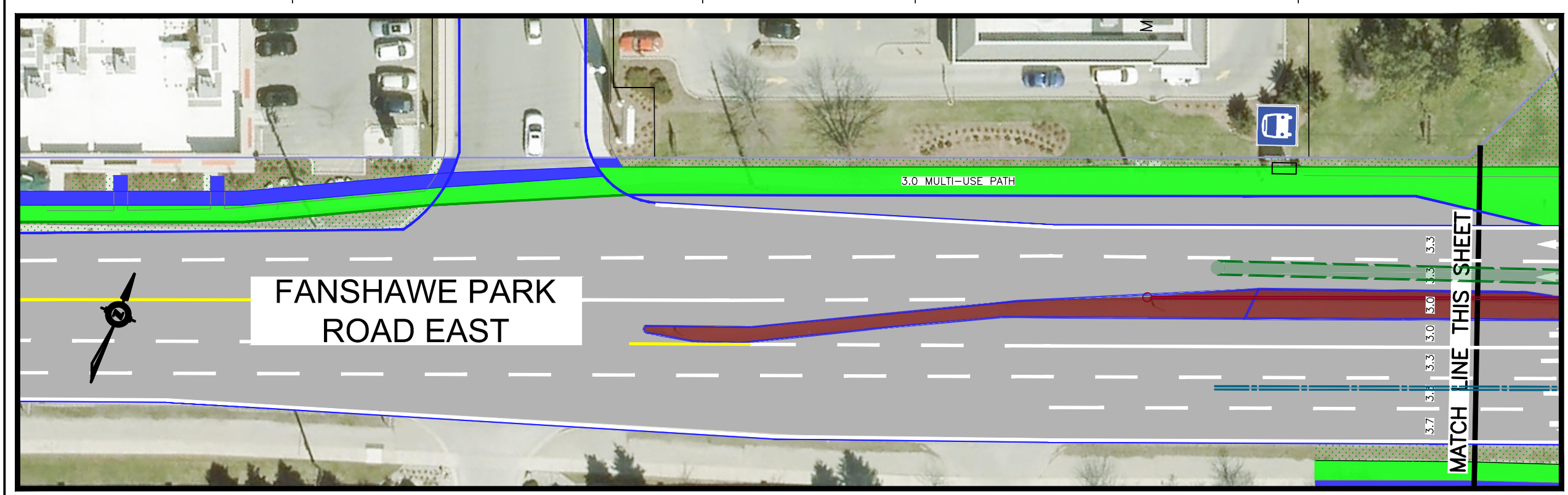
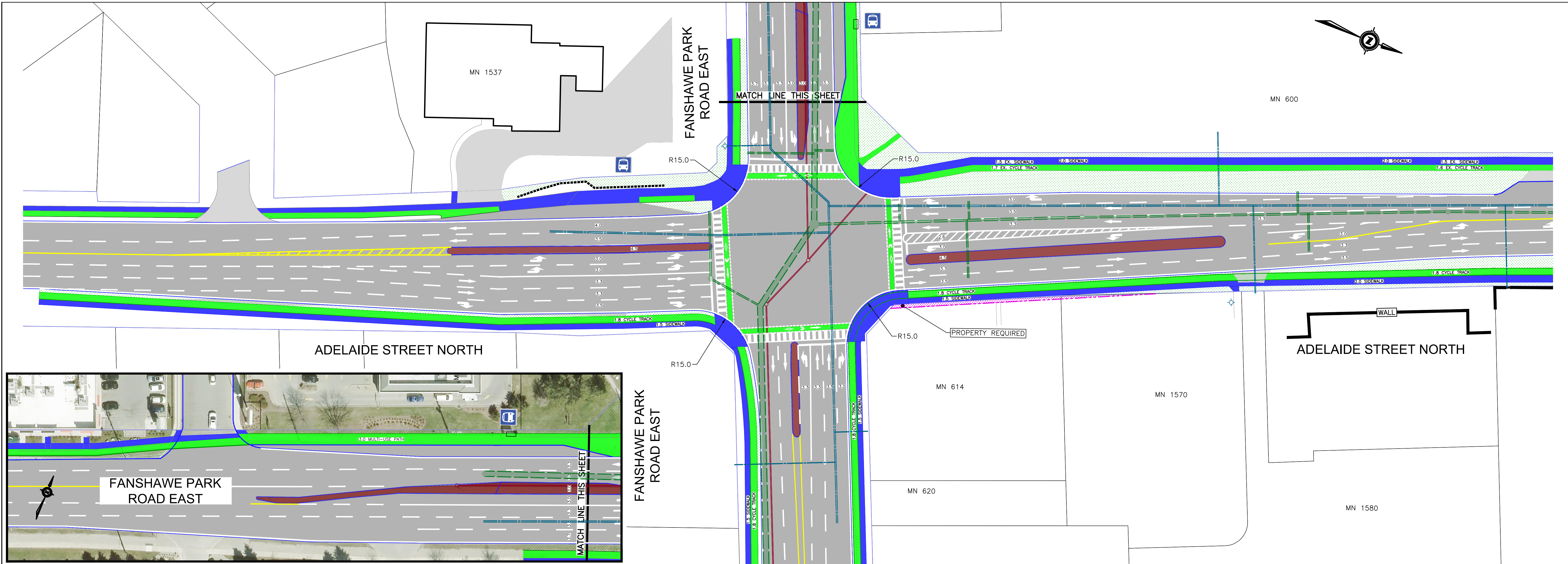
OSR 750 Drawing and Specification

## **BIO-SWALES**



TYPICAL BIO-SWALE CROSS SECTION

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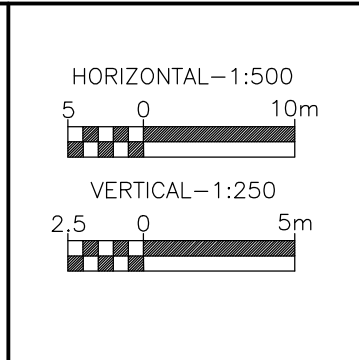


**LEGEND**

	PROPERTY LINE		EXISTING SIDEWALK
	PROPOSED RIGHT-OF-WAY		EXISTING CYCLE TRACK
	EXISTING GROUND PROFILE		PROPOSED ROAD
	FUTURE ROAD PROFILE		PROPOSED BICYCLE LANE
	PROPERTY REQUIRED FOR ADDITIONAL RIGHT OF WAY		PROPOSED SIDEWALK
			PROPOSED CYCLE TRACK

**LEGEND**

	EXISTING SIDEWALK
	EXISTING CYCLE TRACK
	PROPOSED ROAD
	PROPOSED BICYCLE LANE
	PROPOSED SIDEWALK
	PROPOSED CYCLE TRACK

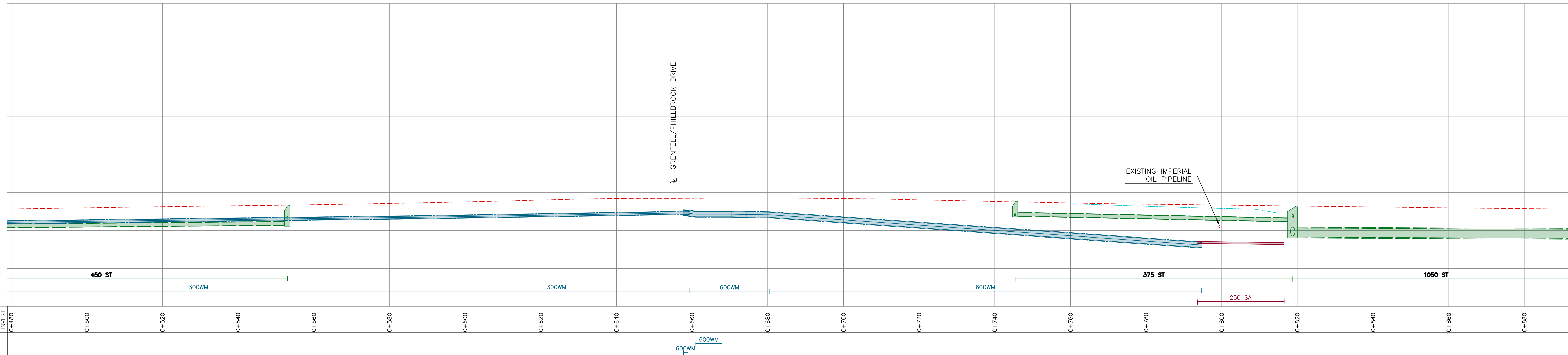
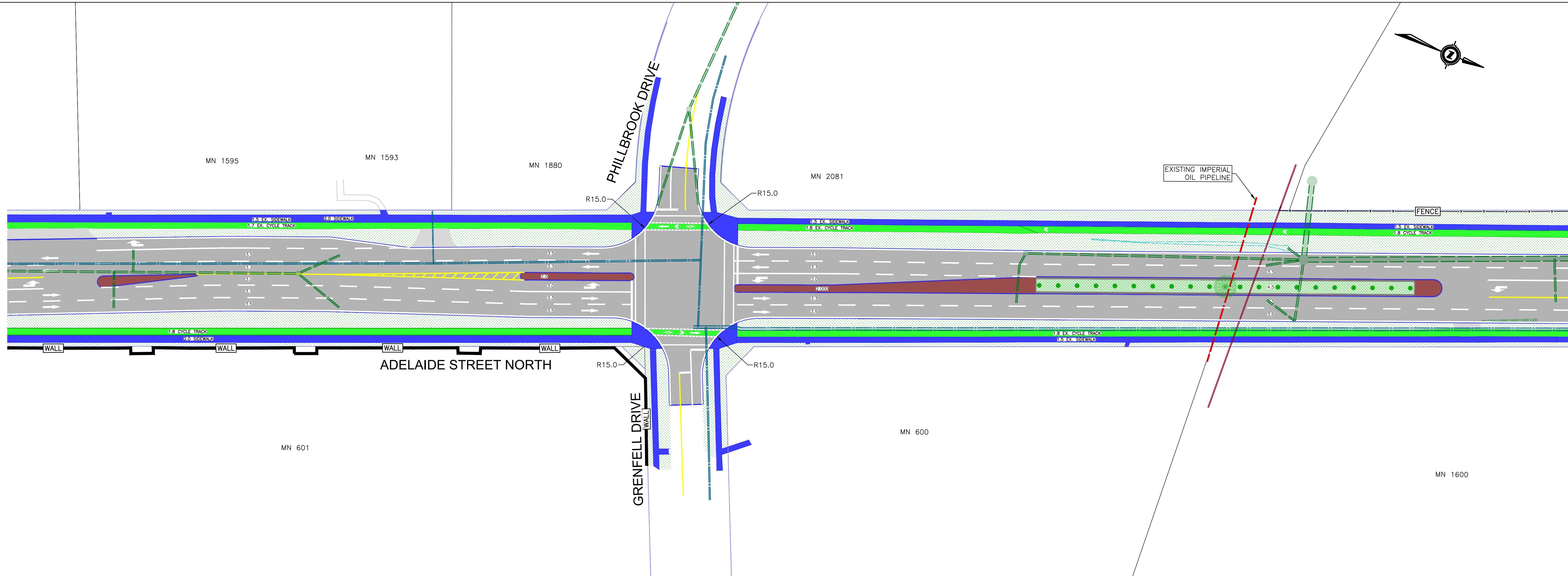
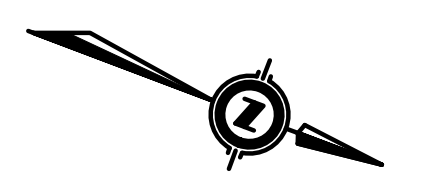


**ADELAIDE STREET NORTH**  
 FANSHAWE PARK ROAD TO SUNNINGDALE

**RECOMMENDED ALTERNATIVE**

ADELAIDE STREET NORTH STA 0+000 TO STA 0+480

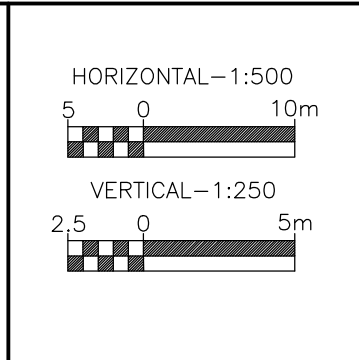
PROJECT No.	-----
PLATE No.	1



Job No. ----- DWG No.2 J:\DATA\476793 - Adelaide St. North\6E\_Dwg\NewConst - EA\476793-Plan and Profile.dwg



LEGEND	
	PROPERTY LINE
	PROPOSED RIGHT-OF-WAY
	EXISTING GROUND PROFILE
	FUTURE ROAD PROFILE
	PROPERTY REQUIRED FOR ADDITIONAL RIGHT OF WAY
	EXISTING SIDEWALK
	EXISTING CYCLE TRACK
	PROPOSED ROAD
	PROPOSED BICYCLE LANE
	PROPOSED SIDEWALK
	PROPOSED CYCLE TRACK

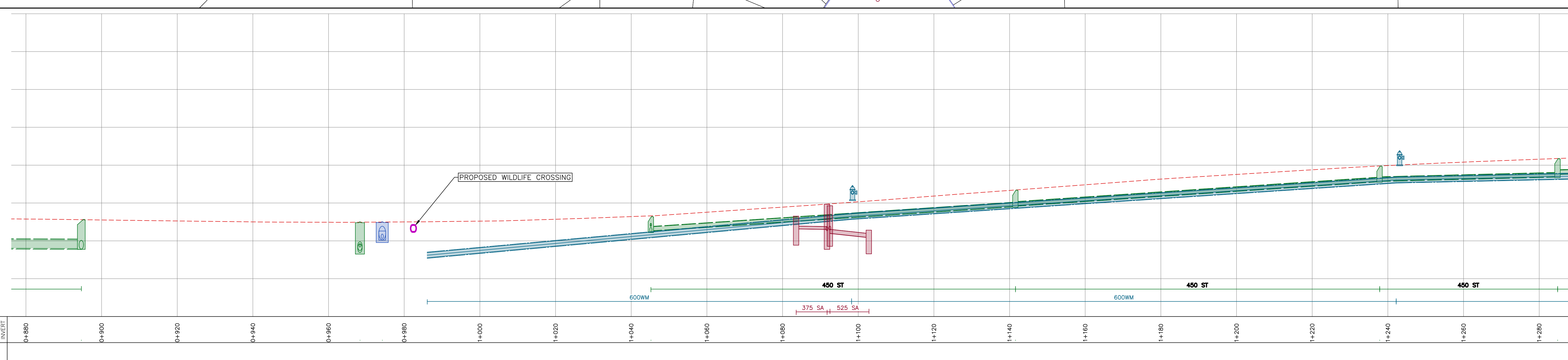
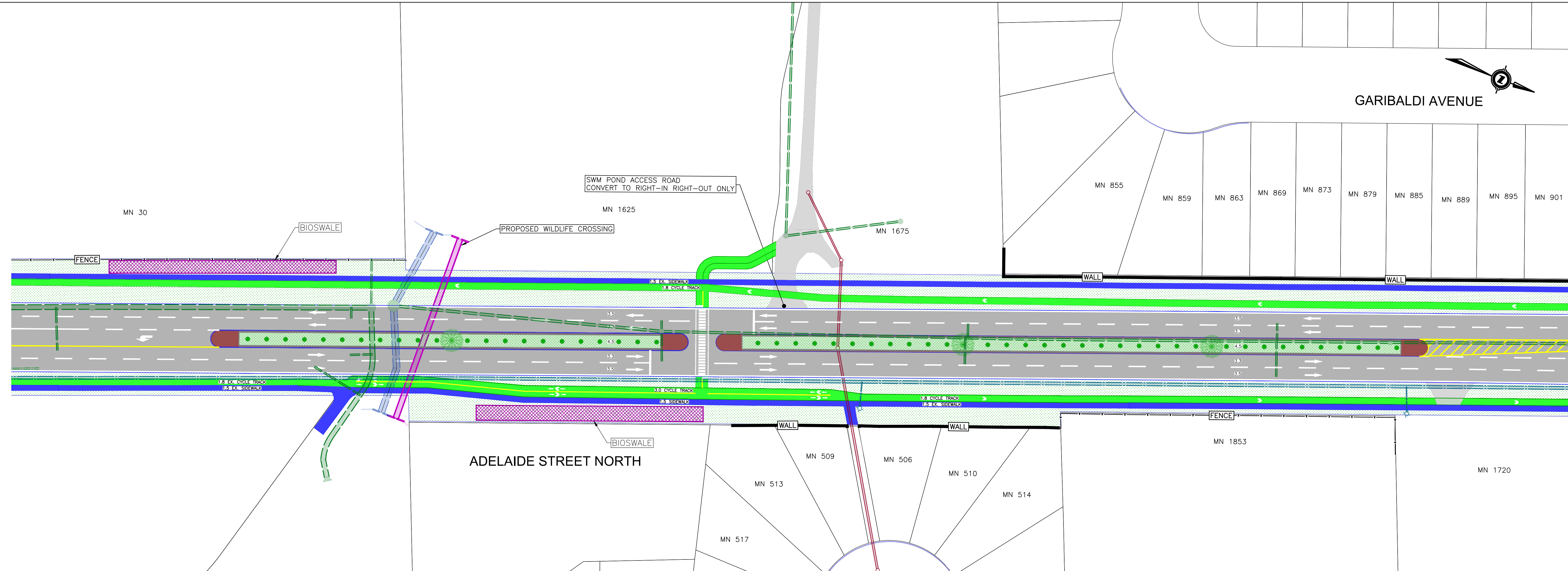
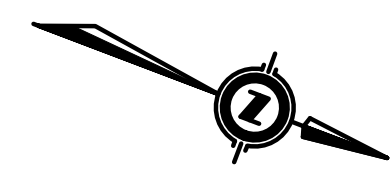


**ADELAIDE STREET NORTH**  
FANSHAWE PARK ROAD TO SUNNINGDALE

**RECOMMENDED ALTERNATIVE**

ADELAIDE STREET NORTH STA 0+480 TO STA 0+880

PROJECT No.	-----
PLATE No.	<b>2</b>



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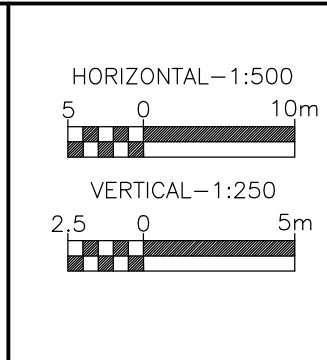
STATION  
STORM SEWER  
INVERT

STATION  
STORM SEWER  
INVERT



**LEGEND**

	PROPERTY LINE		EXISTING SIDEWALK
	PROPOSED RIGHT-OF-WAY		EXISTING CYCLE TRACK
	EXISTING GROUND PROFILE		PROPOSED ROAD
	FUTURE ROAD PROFILE		PROPOSED BICYCLE LANE
	PROPERTY REQUIRED FOR ADDITIONAL RIGHT OF WAY		PROPOSED SIDEWALK
			PROPOSED CYCLE TRACK



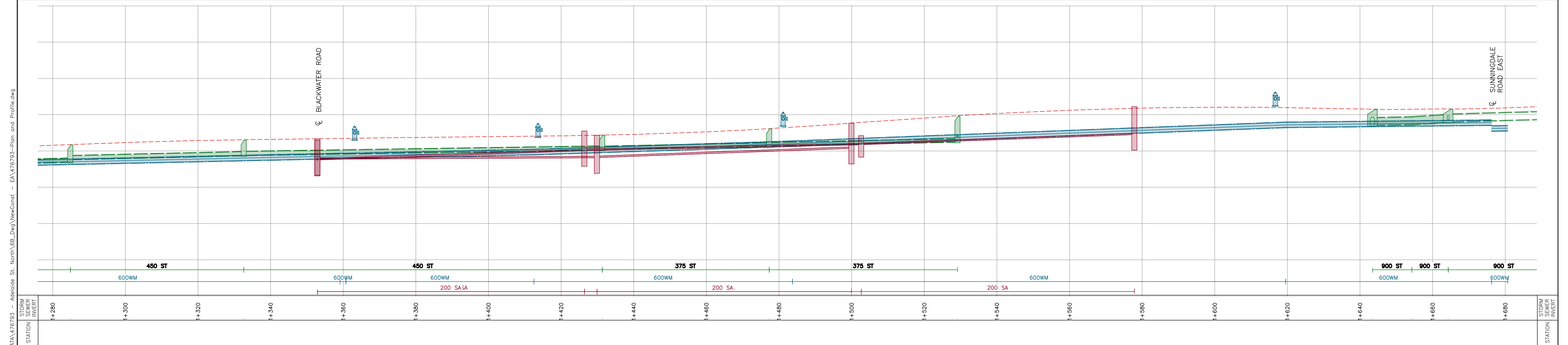
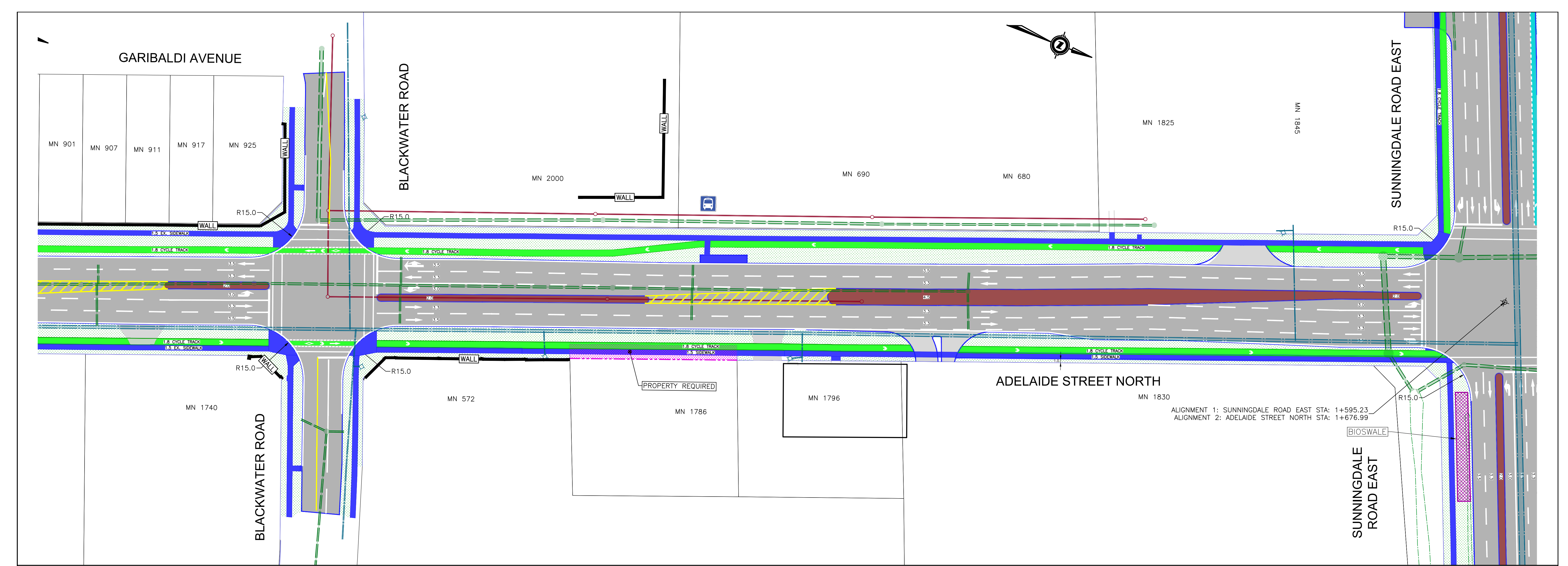
**ADELAIDE STREET NORTH**  
 FANSHAW PARK ROAD TO SUNNINGDALE

**RECOMMENDED ALTERNATIVE**

ADELAIDE STREET NORTH STA 0+880 TO STA 1+280

PROJECT No. ---  
 PLATE No. **3**





**LEGEND**

	PROPERTY LINE		EXISTING SIDEWALK
	PROPOSED RIGHT-OF-WAY		EXISTING CYCLE TRACK
	EXISTING GROUND PROFILE		PROPOSED ROAD
	FUTURE ROAD PROFILE		PROPOSED BICYCLE LANE
	PROPERTY REQUIRED FOR ADDITIONAL RIGHT OF WAY		PROPOSED SIDEWALK
			PROPOSED CYCLE TRACK

HORIZONTAL - 1:500

VERTICAL - 1:250

**ADELAIDE STREET NORTH**  
FANSHAW PARK ROAD TO SUNNINGDALE

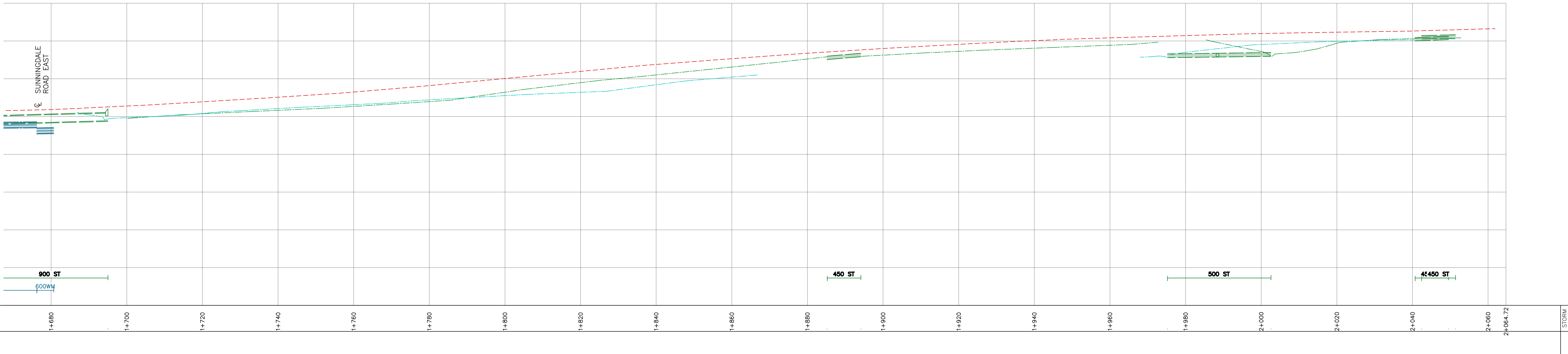
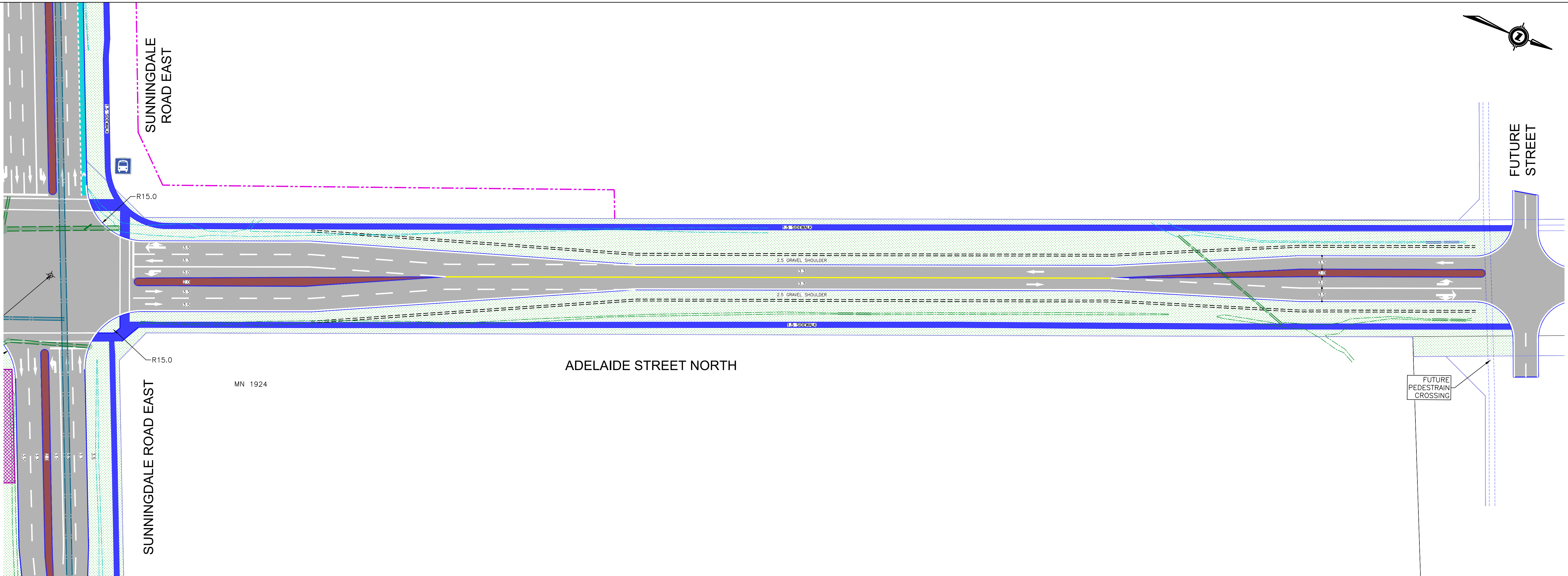
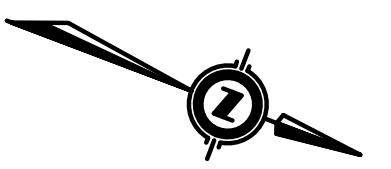
**RECOMMENDED ALTERNATIVE**

ADELAIDE STREET NORTH STA 1+280 TO STA 1+680

PROJECT No. \_\_\_\_\_

PLATE No. **4**

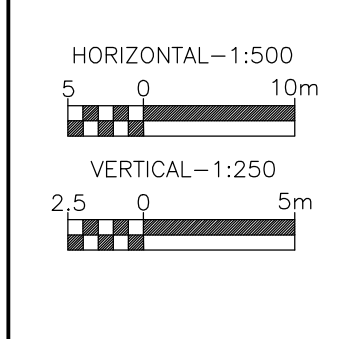
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**LEGEND**

	PROPERTY LINE		EXISTING SIDEWALK
	PROPOSED RIGHT-OF-WAY		EXISTING CYCLE TRACK
	EXISTING GROUND PROFILE		PROPOSED ROAD
	FUTURE ROAD PROFILE		PROPOSED BICYCLE LANE
	PROPERTY REQUIRED FOR ADDITIONAL RIGHT OF WAY		PROPOSED SIDEWALK
			PROPOSED CYCLE TRACK

	EXISTING SIDEWALK
	EXISTING CYCLE TRACK
	PROPOSED ROAD
	PROPOSED BICYCLE LANE
	PROPOSED SIDEWALK
	PROPOSED CYCLE TRACK



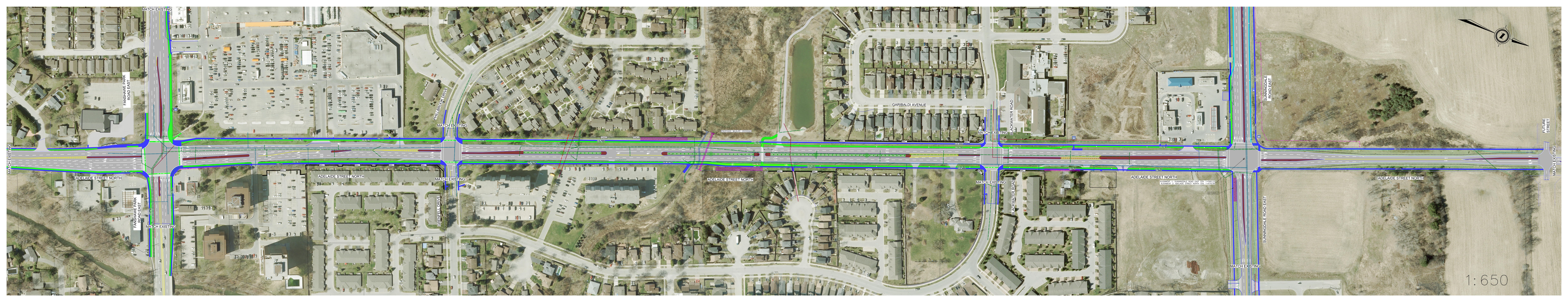
**ADELAIDE STREET NORTH**  
 FANSHAWE PARK ROAD TO SUNNINGDALE

**RECOMMENDED ALTERNATIVE**

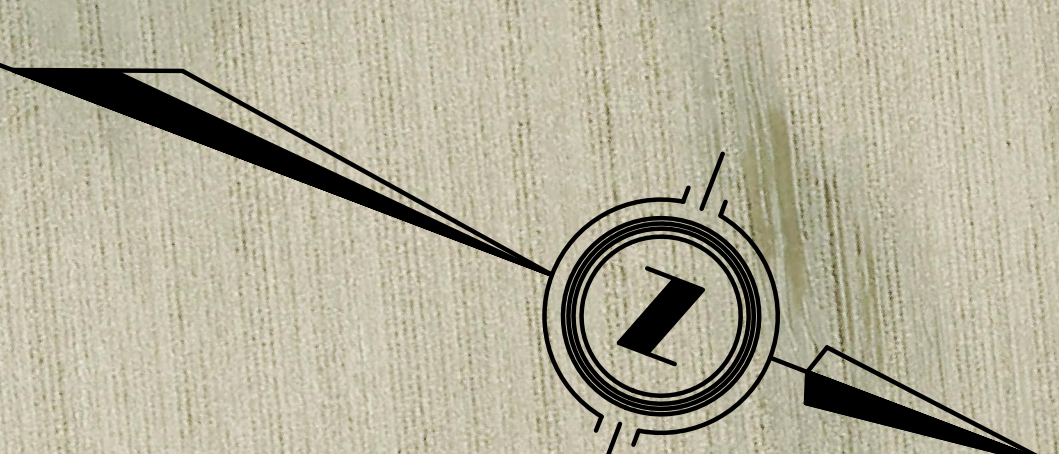
ADELAIDE STREET NORTH STA 1+680 TO STA 2+065

PROJECT No.	-----
PLATE No.	<b>5</b>

Job No. ----- DWG No.5 J:\DATA\476793 - Adelaide St. North\6E\_Dwg\NewConst - EA\476793-Plan and Profile.dwg



1:650



ALIGNMENT 1: SUNNINGDALE ROAD EAST STA: 14+565.23  
ALIGNMENT 2: ADELAIDE STREET NORTH STA: 14+676.99

SUNNINGDALE ROAD EAST

ADELAIDE STREET NORTH

ADELAIDE STREET NORTH

MATCH EXISTING

BLACKWATER ROAD

MATCH EXISTING

BLACKWATER ROAD

GARIBALDI AVENUE

PROPOSED WALKWAY CROSSING

MATCH EXISTING

PHILLBROOK DRIVE

MATCH EXISTING

FANSHAWE PARK ROAD EAST

MATCH EXISTING

MATCH EXISTING

FUTURE STREET

MATCH EXISTING

**STORMCEPTOR**

## Detailed Stormceptor Sizing Report – Outlet #1

Project Information & Location			
<b>Project Name</b>	Adelaide Street Widening	<b>Project Number</b>	476793
<b>City</b>	London	<b>State/ Province</b>	Ontario
<b>Country</b>	Canada	<b>Date</b>	5/21/2019
Designer Information		EOR Information (optional)	
<b>Name</b>	Nicanor Juan	<b>Name</b>	
<b>Company</b>	Parsons	<b>Company</b>	
<b>Phone #</b>	905-917-3290	<b>Phone #</b>	
<b>Email</b>	Mike.He@parsons.com	<b>Email</b>	

### Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

<b>Site Name</b>	Outlet #1
<b>Recommended Stormceptor Model</b>	OSR 300
<b>Target TSS Removal (%)</b>	80.0
<b>TSS Removal (%) Provided</b>	86
<b>PSD</b>	F-60 Sand
<b>Rainfall Station</b>	LONDON A

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
OSR Model	% TSS Removal Provided
OSR 300	86
OSR 750	93
OSR 2000	95
OSR 4000	97
OSR 6000	98
OSR 9000	98
OSR 14000	99
StormceptorMAX	Custom

### Stormceptor

The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor’s patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur. Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

### Design Methodology

Stormceptor is sized using PCSWMM for Stormceptor, a continuous simulation model based on US EPA SWMM. The program calculates hydrology using local historical rainfall data and specified site parameters. With US EPA SWMM’s precision, every Stormceptor unit is designed to achieve a defined water quality objective. The TSS removal data presented follows US EPA guidelines to reduce the average annual TSS load. The Stormceptor’s unit process for TSS removal is settling. The settling model calculates TSS removal by analyzing:

- Site parameters
- Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
- Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
- TSS load
- Detention time of the system

### Hydrology Analysis

PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour). Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

### Rainfall Station

<b>State/Province</b>	Ontario	<b>Total Number of Rainfall Events</b>	5513
<b>Rainfall Station Name</b>	LONDON A	<b>Total Rainfall (mm)</b>	28681.4
<b>Station ID #</b>	4475	<b>Average Annual Rainfall (mm)</b>	667.0
<b>Coordinates</b>	43°02'00"N, 81°09'00"W	<b>Total Evaporation (mm)</b>	1736.4
<b>Elevation (ft)</b>	912	<b>Total Infiltration (mm)</b>	10006.7
<b>Years of Rainfall Data</b>	43	<b>Total Rainfall that is Runoff (mm)</b>	16938.3

### Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (ha)	1.57
Imperviousness %	65.0

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (L)	
Peak Conveyed Flow Rate (L/s)	
Water Quality Flow Rate (L/s)	

Up Stream Storage	
Storage (ha-m)	Discharge (cms)
0.000	0.000

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cms)	

Design Details	
Stormceptor Inlet Invert Elev (m)	
Stormceptor Outlet Invert Elev (m)	
Stormceptor Rim Elev (m)	
Normal Water Level Elevation (m)	
Pipe Diameter (mm)	
Pipe Material	
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

Particle Size Distribution (PSD)		
Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.		
F-60 Sand		
Particle Diameter (microns)	Distribution %	Specific Gravity
75.0	1.0	2.65
150.0	9.0	2.65
230.0	50.0	2.65
350.0	30.0	2.65
500.0	10.0	2.65

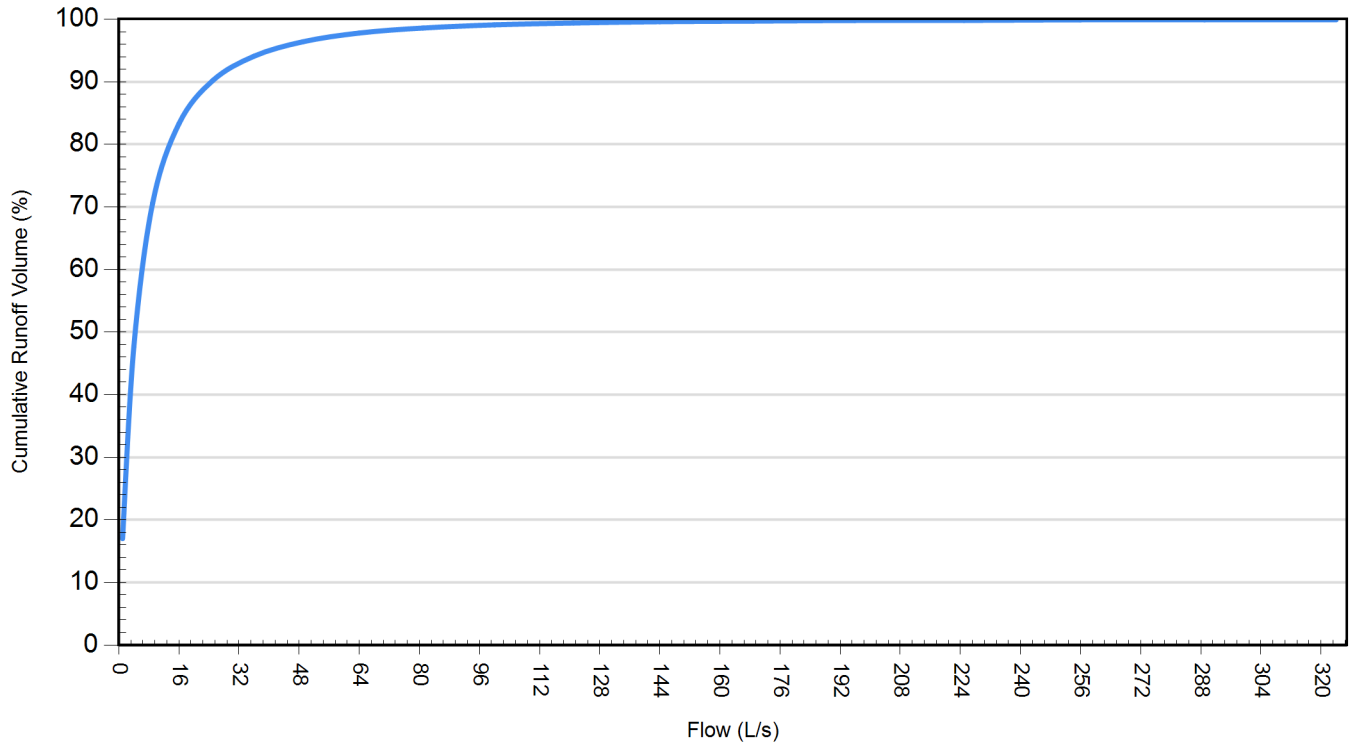
Site Name		Outlet #1	
<b>Site Details</b>			
<b>Drainage Area</b>		<b>Infiltration Parameters</b>	
Total Area (ha)	1.57	Horton's equation is used to estimate infiltration	
Imperviousness %	65.0	Max. Infiltration Rate (mm/hr)	61.98
<b>Surface Characteristics</b>		Min. Infiltration Rate (mm/hr)	10.16
Width (m)	251.00	Decay Rate (1/sec)	0.00055
Slope %	2	Regeneration Rate (1/sec)	0.01
Impervious Depression Storage (mm)	0.508	<b>Evaporation</b>	
Pervious Depression Storage (mm)	5.08	Daily Evaporation Rate (mm/day)	2.54
Impervious Manning's n	0.015	<b>Dry Weather Flow</b>	
Pervious Manning's n	0.25	Dry Weather Flow (lps)	0
<b>Maintenance Frequency</b>		<b>Winter Months</b>	
Maintenance Frequency (months) >	12	Winter Infiltration	0
<b>TSS Loading Parameters</b>			
TSS Loading Function			
<b>Buildup/Wash-off Parameters</b>		<b>TSS Availability Parameters</b>	
Target Event Mean Conc. (EMC) mg/L		Availability Constant A	
Exponential Buildup Power		Availability Factor B	
Exponential Washoff Exponent		Availability Exponent C	
		Min. Particle Size Affected by Availability (micron)	



Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (L/s)	Runoff Volume (m³)	Volume Over (m³)	Cumulative Runoff Volume (%)
1	45492	222305	17.0
4	127692	140122	47.7
9	189122	78745	70.6
16	222958	44853	83.3
25	241441	26383	90.2
36	251931	15880	94.1
49	258141	9673	96.4
64	261791	6020	97.8
81	264034	3778	98.6
100	265391	2421	99.1
121	266203	1608	99.4
144	266721	1090	99.6
169	266985	826	99.7
196	267171	640	99.8
225	267308	503	99.8
256	267416	395	99.9
289	267486	325	99.9
324	267538	273	99.9

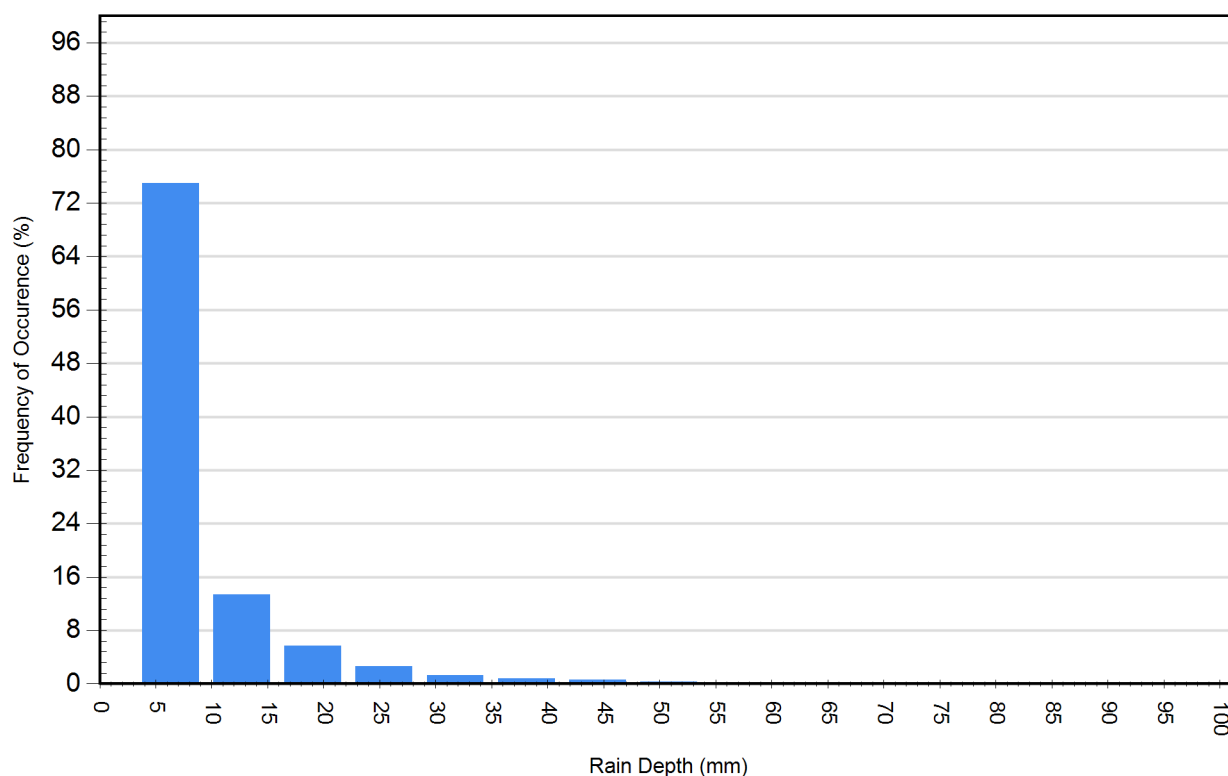
### Cumulative Runoff Volume by Runoff Rate

For area: 1.57(ha), imperviousness: 65.0%, rainfall station: LONDON A



Rainfall Event Analysis				
Rainfall Depth (mm)	No. of Events	Percentage of Total Events (%)	Total Volume (mm)	Percentage of Annual Volume (%)
6.35	4133	75.0	7031	24.5
12.70	739	13.4	6819	23.8
19.05	313	5.7	4859	16.9
25.40	146	2.6	3251	11.3
31.75	72	1.3	2047	7.1
38.10	42	0.8	1464	5.1
44.45	33	0.6	1353	4.7
50.80	18	0.3	850	3.0
57.15	9	0.2	488	1.7
63.50	5	0.1	303	1.1
69.85	1	0.0	65	0.2
76.20	1	0.0	70	0.2
82.55	1	0.0	83	0.3
88.90	0	0.0	0	0.0
95.25	0	0.0	0	0.0

Frequency of Occurrence by Rainfall Depths



For Stormceptor Specifications and Drawings Please Visit:  
<http://www.imbriumsystems.com/technical-specifications>

## Detailed Stormceptor Sizing Report – Outlet #2

Project Information & Location			
<b>Project Name</b>	Adelaide Street Widening	<b>Project Number</b>	476793
<b>City</b>	London	<b>State/ Province</b>	Ontario
<b>Country</b>	Canada	<b>Date</b>	5/21/2019
Designer Information		EOR Information (optional)	
<b>Name</b>	Nicanor Juan	<b>Name</b>	
<b>Company</b>	Parsons	<b>Company</b>	
<b>Phone #</b>	905-917-3290	<b>Phone #</b>	
<b>Email</b>	Mike.He@parsons.com	<b>Email</b>	

### Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

<b>Site Name</b>	Outlet #2
<b>Recommended Stormceptor Model</b>	OSR 300
<b>Target TSS Removal (%)</b>	80.0
<b>TSS Removal (%) Provided</b>	89
<b>PSD</b>	F-60 Sand
<b>Rainfall Station</b>	LONDON A

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
OSR Model	% TSS Removal Provided
OSR 300	89
OSR 750	94
OSR 2000	96
OSR 4000	98
OSR 6000	98
OSR 9000	99
OSR 14000	99
StormceptorMAX	Custom

### Stormceptor

The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor’s patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur. Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

### Design Methodology

Stormceptor is sized using PCSWMM for Stormceptor, a continuous simulation model based on US EPA SWMM. The program calculates hydrology using local historical rainfall data and specified site parameters. With US EPA SWMM’s precision, every Stormceptor unit is designed to achieve a defined water quality objective. The TSS removal data presented follows US EPA guidelines to reduce the average annual TSS load. The Stormceptor’s unit process for TSS removal is settling. The settling model calculates TSS removal by analyzing:

- Site parameters
- Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
- Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
- TSS load
- Detention time of the system

### Hydrology Analysis

PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour). Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

### Rainfall Station

<b>State/Province</b>	Ontario	<b>Total Number of Rainfall Events</b>	5513
<b>Rainfall Station Name</b>	LONDON A	<b>Total Rainfall (mm)</b>	28681.4
<b>Station ID #</b>	4475	<b>Average Annual Rainfall (mm)</b>	667.0
<b>Coordinates</b>	43°02'00"N, 81°09'00"W	<b>Total Evaporation (mm)</b>	1688.1
<b>Elevation (ft)</b>	912	<b>Total Infiltration (mm)</b>	10280.6
<b>Years of Rainfall Data</b>	43	<b>Total Rainfall that is Runoff (mm)</b>	16712.7

### Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (ha)	1.17
Imperviousness %	64.0

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (L)	
Peak Conveyed Flow Rate (L/s)	
Water Quality Flow Rate (L/s)	

Up Stream Storage	
Storage (ha-m)	Discharge (cms)
0.000	0.000

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cms)	

Design Details	
Stormceptor Inlet Invert Elev (m)	
Stormceptor Outlet Invert Elev (m)	
Stormceptor Rim Elev (m)	
Normal Water Level Elevation (m)	
Pipe Diameter (mm)	
Pipe Material	
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

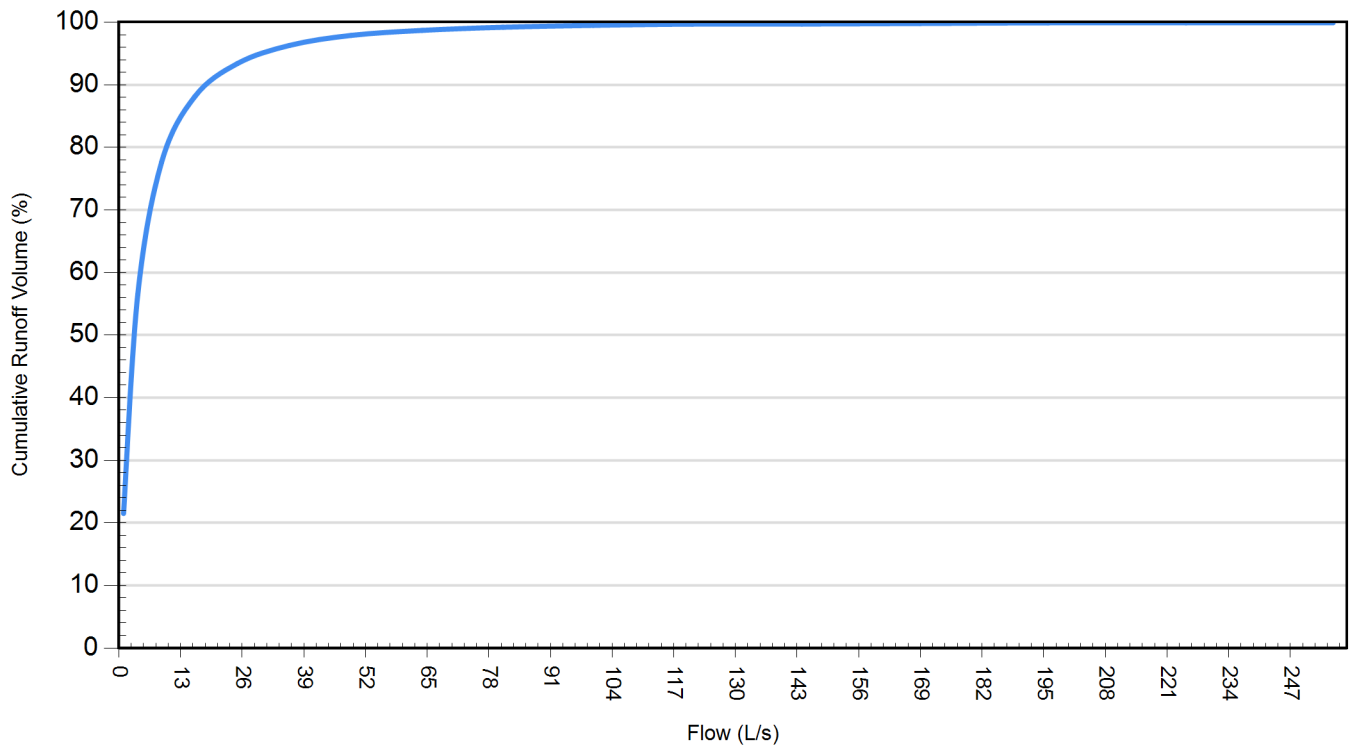
Particle Size Distribution (PSD)		
Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.		
F-60 Sand		
Particle Diameter (microns)	Distribution %	Specific Gravity
75.0	1.0	2.65
150.0	9.0	2.65
230.0	50.0	2.65
350.0	30.0	2.65
500.0	10.0	2.65

Site Name		Outlet #2	
<b>Site Details</b>			
<b>Drainage Area</b>		<b>Infiltration Parameters</b>	
Total Area (ha)	1.17	Horton's equation is used to estimate infiltration	
Imperviousness %	64.0	Max. Infiltration Rate (mm/hr)	61.98
<b>Surface Characteristics</b>		Min. Infiltration Rate (mm/hr)	10.16
Width (m)	216.00	Decay Rate (1/sec)	0.00055
Slope %	2	Regeneration Rate (1/sec)	0.01
Impervious Depression Storage (mm)	0.508	<b>Evaporation</b>	
Pervious Depression Storage (mm)	5.08	Daily Evaporation Rate (mm/day)	2.54
Impervious Manning's n	0.015	<b>Dry Weather Flow</b>	
Pervious Manning's n	0.25	Dry Weather Flow (lps)	0
<b>Maintenance Frequency</b>		<b>Winter Months</b>	
Maintenance Frequency (months) >	12	Winter Infiltration	0
<b>TSS Loading Parameters</b>			
TSS Loading Function			
<b>Buildup/Wash-off Parameters</b>		<b>TSS Availability Parameters</b>	
Target Event Mean Conc. (EMC) mg/L		Availability Constant A	
Exponential Buildup Power		Availability Factor B	
Exponential Washoff Exponent		Availability Exponent C	
		Min. Particle Size Affected by Availability (micron)	

Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (L/s)	Runoff Volume (m³)	Volume Over (m³)	Cumulative Runoff Volume (%)
1	42356	154563	21.5
4	111253	85663	56.5
9	152862	44086	77.6
16	173503	23412	88.1
25	183991	12931	93.4
36	189634	7282	96.3
49	192722	4196	97.9
64	194443	2473	98.7
81	195427	1489	99.2
100	195976	941	99.5
121	196256	661	99.7
144	196424	493	99.7
169	196544	373	99.8
196	196633	284	99.9
225	196685	231	99.9
256	196727	190	99.9

### Cumulative Runoff Volume by Runoff Rate

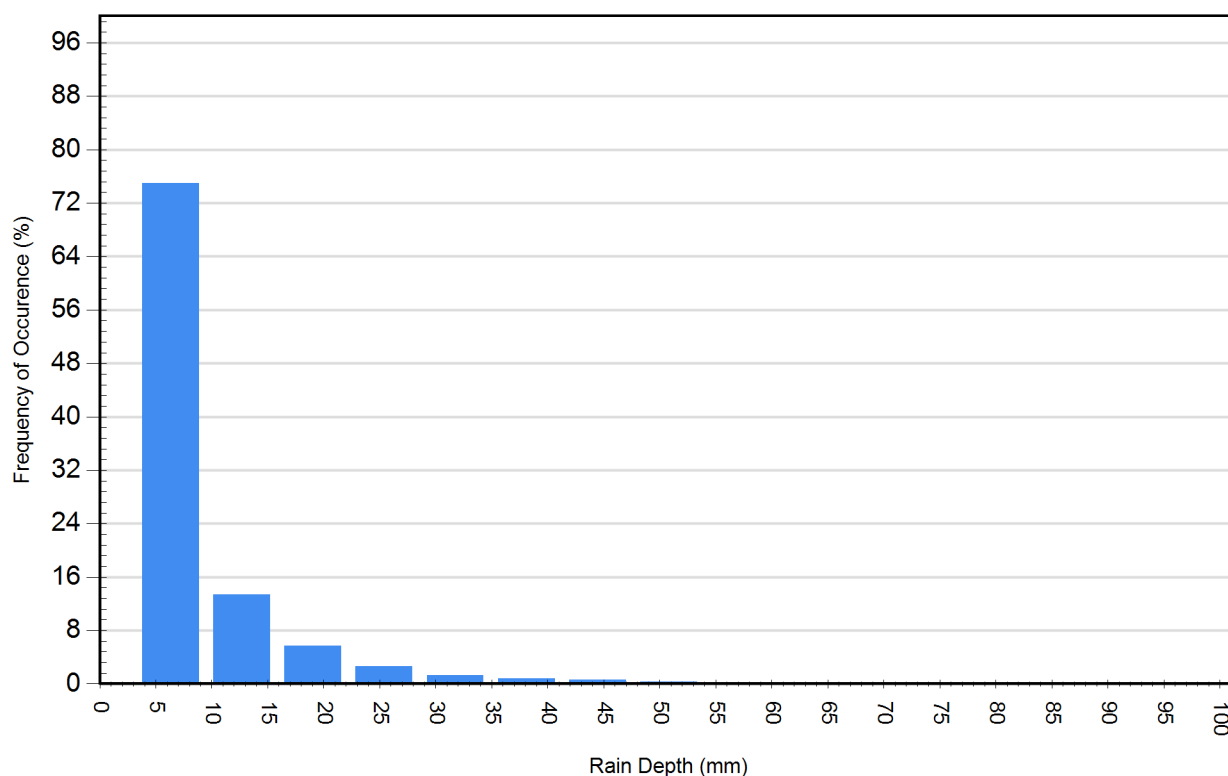
For area: 1.17(ha), imperviousness: 64.0%, rainfall station: LONDON A





Rainfall Event Analysis				
Rainfall Depth (mm)	No. of Events	Percentage of Total Events (%)	Total Volume (mm)	Percentage of Annual Volume (%)
6.35	4133	75.0	7031	24.5
12.70	739	13.4	6819	23.8
19.05	313	5.7	4859	16.9
25.40	146	2.6	3251	11.3
31.75	72	1.3	2047	7.1
38.10	42	0.8	1464	5.1
44.45	33	0.6	1353	4.7
50.80	18	0.3	850	3.0
57.15	9	0.2	488	1.7
63.50	5	0.1	303	1.1
69.85	1	0.0	65	0.2
76.20	1	0.0	70	0.2
82.55	1	0.0	83	0.3
88.90	0	0.0	0	0.0
95.25	0	0.0	0	0.0

Frequency of Occurrence by Rainfall Depths



For Stormceptor Specifications and Drawings Please Visit:  
<http://www.imbriumsystems.com/technical-specifications>

## Detailed Stormceptor Sizing Report – Outlet #3

Project Information & Location			
<b>Project Name</b>	Adelaide Street Widening	<b>Project Number</b>	476793
<b>City</b>	London	<b>State/ Province</b>	Ontario
<b>Country</b>	Canada	<b>Date</b>	5/21/2019
Designer Information		EOR Information (optional)	
<b>Name</b>	Nicanor Juan	<b>Name</b>	
<b>Company</b>	Parsons	<b>Company</b>	
<b>Phone #</b>	905-917-3290	<b>Phone #</b>	
<b>Email</b>	Mike.He@parsons.com	<b>Email</b>	

### Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

<b>Site Name</b>	Outlet #3
<b>Recommended Stormceptor Model</b>	OSR 300
<b>Target TSS Removal (%)</b>	80.0
<b>TSS Removal (%) Provided</b>	81
<b>PSD</b>	F-60 Sand
<b>Rainfall Station</b>	LONDON A

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
OSR Model	% TSS Removal Provided
OSR 300	81
OSR 750	89
OSR 2000	93
OSR 4000	95
OSR 6000	96
OSR 9000	97
OSR 14000	98
StormceptorMAX	Custom

### Stormceptor

The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor’s patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur. Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

### Design Methodology

Stormceptor is sized using PCSWMM for Stormceptor, a continuous simulation model based on US EPA SWMM. The program calculates hydrology using local historical rainfall data and specified site parameters. With US EPA SWMM’s precision, every Stormceptor unit is designed to achieve a defined water quality objective. The TSS removal data presented follows US EPA guidelines to reduce the average annual TSS load. The Stormceptor’s unit process for TSS removal is settling. The settling model calculates TSS removal by analyzing:

- Site parameters
- Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
- Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
- TSS load
- Detention time of the system

### Hydrology Analysis

PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour). Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

### Rainfall Station

<b>State/Province</b>	Ontario	<b>Total Number of Rainfall Events</b>	5513
<b>Rainfall Station Name</b>	LONDON A	<b>Total Rainfall (mm)</b>	28681.4
<b>Station ID #</b>	4475	<b>Average Annual Rainfall (mm)</b>	667.0
<b>Coordinates</b>	43°02'00"N, 81°09'00"W	<b>Total Evaporation (mm)</b>	1636.5
<b>Elevation (ft)</b>	912	<b>Total Infiltration (mm)</b>	11436.1
<b>Years of Rainfall Data</b>	43	<b>Total Rainfall that is Runoff (mm)</b>	15608.8

### Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (ha)	2.65
Imperviousness %	60.0

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (L)	
Peak Conveyed Flow Rate (L/s)	
Water Quality Flow Rate (L/s)	

Up Stream Storage	
Storage (ha-m)	Discharge (cms)
0.000	0.000

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cms)	

Design Details	
Stormceptor Inlet Invert Elev (m)	
Stormceptor Outlet Invert Elev (m)	
Stormceptor Rim Elev (m)	
Normal Water Level Elevation (m)	
Pipe Diameter (mm)	
Pipe Material	
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

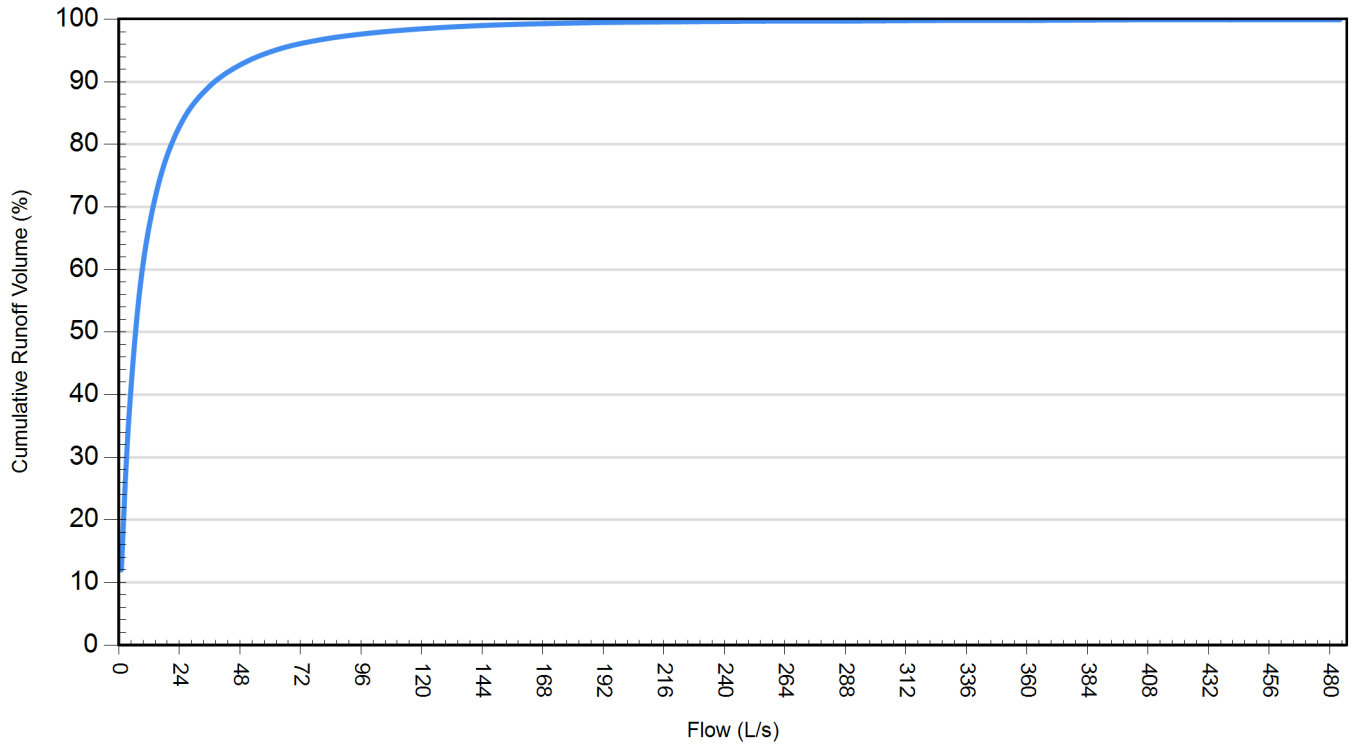
Particle Size Distribution (PSD)		
Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.		
F-60 Sand		
Particle Diameter (microns)	Distribution %	Specific Gravity
75.0	1.0	2.65
150.0	9.0	2.65
230.0	50.0	2.65
350.0	30.0	2.65
500.0	10.0	2.65

Site Name		Outlet #3	
<b>Site Details</b>			
<b>Drainage Area</b>		<b>Infiltration Parameters</b>	
Total Area (ha)	2.65	Horton's equation is used to estimate infiltration	
Imperviousness %	60.0	Max. Infiltration Rate (mm/hr)	61.98
<b>Surface Characteristics</b>		Min. Infiltration Rate (mm/hr)	10.16
Width (m)	326.00	Decay Rate (1/sec)	0.00055
Slope %	2	Regeneration Rate (1/sec)	0.01
Impervious Depression Storage (mm)	0.508	<b>Evaporation</b>	
Pervious Depression Storage (mm)	5.08	Daily Evaporation Rate (mm/day)	2.54
Impervious Manning's n	0.015	<b>Dry Weather Flow</b>	
Pervious Manning's n	0.25	Dry Weather Flow (lps)	0
<b>Maintenance Frequency</b>		<b>Winter Months</b>	
Maintenance Frequency (months) >	12	Winter Infiltration	0
<b>TSS Loading Parameters</b>			
TSS Loading Function			
<b>Buildup/Wash-off Parameters</b>		<b>TSS Availability Parameters</b>	
Target Event Mean Conc. (EMC) mg/L		Availability Constant A	
Exponential Buildup Power		Availability Factor B	
Exponential Washoff Exponent		Availability Exponent C	
		Min. Particle Size Affected by Availability (micron)	

Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (L/s)	Runoff Volume (m³)	Volume Over (m³)	Cumulative Runoff Volume (%)
1	50086	366356	12.0
4	151462	264943	36.4
9	245910	170601	59.1
16	308724	107656	74.1
25	347906	68461	83.6
36	371898	44495	89.3
49	386959	29407	92.9
64	396700	19670	95.3
81	403102	13265	96.8
100	407300	9071	97.8
121	410139	6229	98.5
144	412026	4343	99.0
169	413297	3071	99.3
196	414139	2229	99.5
225	414720	1648	99.6
256	415070	1298	99.7
289	415316	1051	99.7
324	415496	872	99.8
361	415647	721	99.8
400	415769	599	99.9
441	415855	513	99.9
484	415921	446	99.9

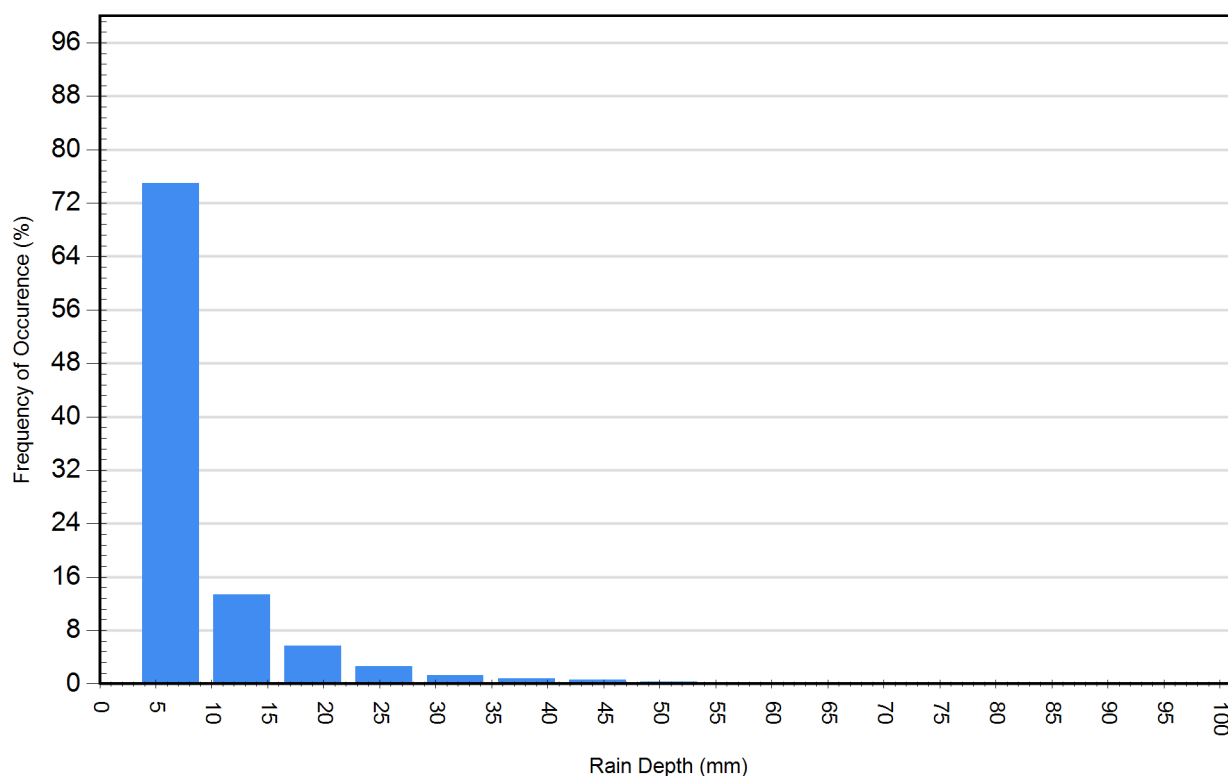
### Cumulative Runoff Volume by Runoff Rate

For area: 2.65(ha), imperviousness: 60.0%, rainfall station: LONDON A



Rainfall Event Analysis				
Rainfall Depth (mm)	No. of Events	Percentage of Total Events (%)	Total Volume (mm)	Percentage of Annual Volume (%)
6.35	4133	75.0	7031	24.5
12.70	739	13.4	6819	23.8
19.05	313	5.7	4859	16.9
25.40	146	2.6	3251	11.3
31.75	72	1.3	2047	7.1
38.10	42	0.8	1464	5.1
44.45	33	0.6	1353	4.7
50.80	18	0.3	850	3.0
57.15	9	0.2	488	1.7
63.50	5	0.1	303	1.1
69.85	1	0.0	65	0.2
76.20	1	0.0	70	0.2
82.55	1	0.0	83	0.3
88.90	0	0.0	0	0.0
95.25	0	0.0	0	0.0

Frequency of Occurrence by Rainfall Depths



For Stormceptor Specifications and Drawings Please Visit:  
<http://www.imbriumsystems.com/technical-specifications>



## Detailed Stormceptor Sizing Report – Outlet #4

Project Information & Location			
<b>Project Name</b>	Adelaide Street Widening	<b>Project Number</b>	476793
<b>City</b>	London	<b>State/ Province</b>	Ontario
<b>Country</b>	Canada	<b>Date</b>	5/21/2019
Designer Information		EOR Information (optional)	
<b>Name</b>	Nicanor Juan	<b>Name</b>	
<b>Company</b>	Parsons	<b>Company</b>	
<b>Phone #</b>	905-917-3290	<b>Phone #</b>	
<b>Email</b>	Mike.He@parsons.com	<b>Email</b>	

### Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

<b>Site Name</b>	Outlet #4
<b>Recommended Stormceptor Model</b>	OSR 750
<b>Target TSS Removal (%)</b>	80.0
<b>TSS Removal (%) Provided</b>	88
<b>PSD</b>	F-60 Sand
<b>Rainfall Station</b>	LONDON A

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
<b>OSR Model</b>	<b>% TSS Removal Provided</b>
OSR 300	77
OSR 750	88
OSR 2000	91
OSR 4000	94
OSR 6000	96
OSR 9000	97
OSR 14000	98
StormceptorMAX	Custom

### Stormceptor

The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor’s patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur. Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

### Design Methodology

Stormceptor is sized using PCSWMM for Stormceptor, a continuous simulation model based on US EPA SWMM. The program calculates hydrology using local historical rainfall data and specified site parameters. With US EPA SWMM’s precision, every Stormceptor unit is designed to achieve a defined water quality objective. The TSS removal data presented follows US EPA guidelines to reduce the average annual TSS load. The Stormceptor’s unit process for TSS removal is settling. The settling model calculates TSS removal by analyzing:

- Site parameters
- Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
- Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
- TSS load
- Detention time of the system

### Hydrology Analysis

PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour). Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

### Rainfall Station

<b>State/Province</b>	Ontario	<b>Total Number of Rainfall Events</b>	5513
<b>Rainfall Station Name</b>	LONDON A	<b>Total Rainfall (mm)</b>	28681.4
<b>Station ID #</b>	4475	<b>Average Annual Rainfall (mm)</b>	667.0
<b>Coordinates</b>	43°02'00"N, 81°09'00"W	<b>Total Evaporation (mm)</b>	1357.2
<b>Elevation (ft)</b>	912	<b>Total Infiltration (mm)</b>	14296.9
<b>Years of Rainfall Data</b>	43	<b>Total Rainfall that is Runoff (mm)</b>	13027.3

### Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (ha)	3.9
Imperviousness %	50.0

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (L)	
Peak Conveyed Flow Rate (L/s)	
Water Quality Flow Rate (L/s)	

Up Stream Storage	
Storage (ha-m)	Discharge (cms)
0.000	0.000

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cms)	

Design Details	
Stormceptor Inlet Invert Elev (m)	
Stormceptor Outlet Invert Elev (m)	
Stormceptor Rim Elev (m)	
Normal Water Level Elevation (m)	
Pipe Diameter (mm)	
Pipe Material	
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

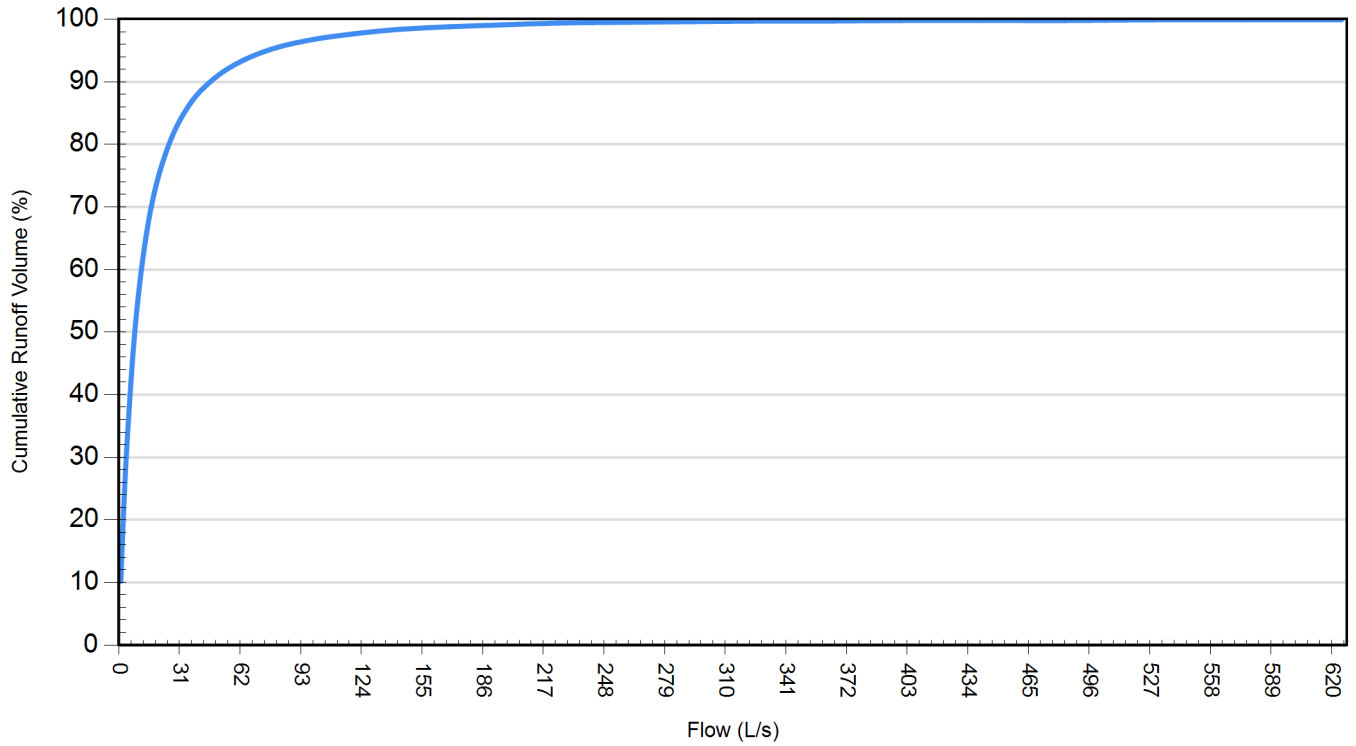
Particle Size Distribution (PSD)		
Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.		
F-60 Sand		
Particle Diameter (microns)	Distribution %	Specific Gravity
75.0	1.0	2.65
150.0	9.0	2.65
230.0	50.0	2.65
350.0	30.0	2.65
500.0	10.0	2.65

Site Name		Outlet #4	
<b>Site Details</b>			
<b>Drainage Area</b>		<b>Infiltration Parameters</b>	
Total Area (ha)	3.9	Horton's equation is used to estimate infiltration	
Imperviousness %	50.0	Max. Infiltration Rate (mm/hr)	61.98
<b>Surface Characteristics</b>		Min. Infiltration Rate (mm/hr)	10.16
Width (m)	395.00	Decay Rate (1/sec)	0.00055
Slope %	2	Regeneration Rate (1/sec)	0.01
Impervious Depression Storage (mm)	0.508	<b>Evaporation</b>	
Pervious Depression Storage (mm)	5.08	Daily Evaporation Rate (mm/day)	2.54
Impervious Manning's n	0.015	<b>Dry Weather Flow</b>	
Pervious Manning's n	0.25	Dry Weather Flow (lps)	0
<b>Maintenance Frequency</b>		<b>Winter Months</b>	
Maintenance Frequency (months) >	12	Winter Infiltration	0
<b>TSS Loading Parameters</b>			
TSS Loading Function			
<b>Buildup/Wash-off Parameters</b>		<b>TSS Availability Parameters</b>	
Target Event Mean Conc. (EMC) mg/L		Availability Constant A	
Exponential Buildup Power		Availability Factor B	
Exponential Washoff Exponent		Availability Exponent C	
		Min. Particle Size Affected by Availability (micron)	

Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (L/s)	Runoff Volume (m³)	Volume Over (m³)	Cumulative Runoff Volume (%)
1	51883	459146	10.2
4	159310	351722	31.2
9	270287	240904	52.9
16	353246	157757	69.1
25	406409	104549	79.5
36	440808	70228	86.3
49	462971	48003	90.6
64	477753	33238	93.5
81	487786	23196	95.5
100	494665	16328	96.8
121	499344	11641	97.7
144	502666	8322	98.4
169	504976	6010	98.8
196	506591	4396	99.1
225	507671	3315	99.4
256	508489	2498	99.5
289	509030	1956	99.6
324	509426	1561	99.7
361	509698	1288	99.7
400	509894	1092	99.8
441	510060	926	99.8
484	510198	788	99.8
529	510302	684	99.9
576	510378	608	99.9
625	510446	540	99.9

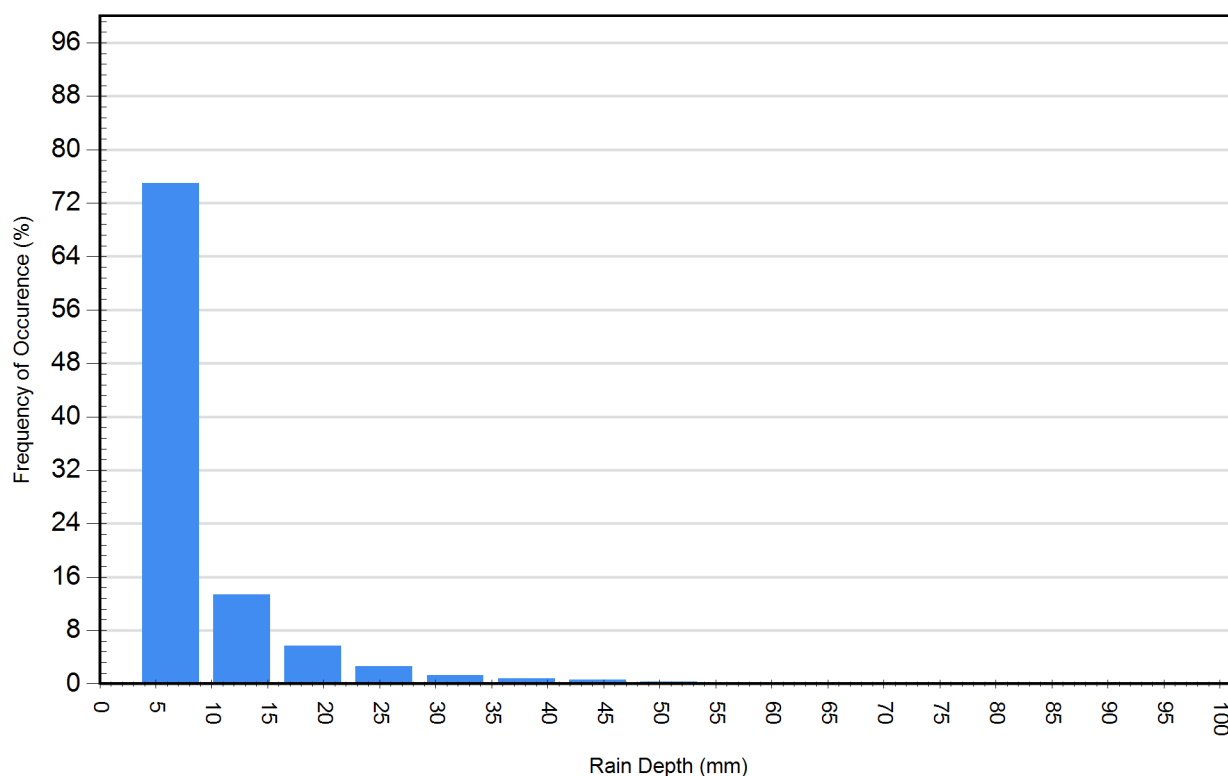
### Cumulative Runoff Volume by Runoff Rate

For area: 3.9(ha), imperviousness: 50.0%, rainfall station: LONDON A



Rainfall Event Analysis				
Rainfall Depth (mm)	No. of Events	Percentage of Total Events (%)	Total Volume (mm)	Percentage of Annual Volume (%)
6.35	4133	75.0	7031	24.5
12.70	739	13.4	6819	23.8
19.05	313	5.7	4859	16.9
25.40	146	2.6	3251	11.3
31.75	72	1.3	2047	7.1
38.10	42	0.8	1464	5.1
44.45	33	0.6	1353	4.7
50.80	18	0.3	850	3.0
57.15	9	0.2	488	1.7
63.50	5	0.1	303	1.1
69.85	1	0.0	65	0.2
76.20	1	0.0	70	0.2
82.55	1	0.0	83	0.3
88.90	0	0.0	0	0.0
95.25	0	0.0	0	0.0

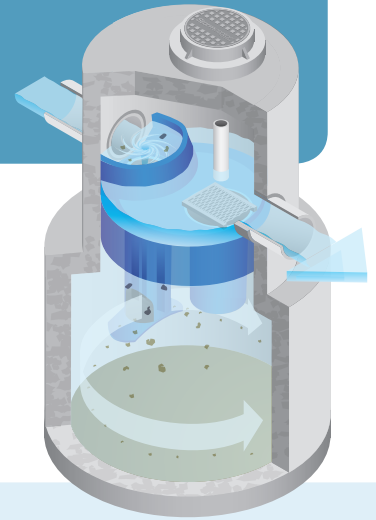
Frequency of Occurrence by Rainfall Depths



For Stormceptor Specifications and Drawings Please Visit:  
<http://www.imbriumsystems.com/technical-specifications>

## **Drawings & Specifications**





## STORMCEPTOR® OSR

The Stormceptor OSR is optimized to remove oil and sediment from stormwater runoff. Recommended applications include pretreatment and redevelopment/retrofit projects to meet your water quality objectives.

### Product Overview

- Patented design is optimized for fine sand-sized particle removal and increased hydraulic capacity
- Typically designed for removal of 50 micron particles and greater
- Easily and flexibly sized to meet your site's needs
- Hydrocarbon spill protection in wet and dry weather conditions

### Proven performance

- Independent third-party tested
- Independent verification by New Jersey Corporation for Advanced Technology (NJCAT)

### Flexible and versatile

- Easy installation – small footprint saves time and money with limited site disruption
- Minimal drop between inlet and outlet; 1-inch (25mm) or 3-inch (75mm)
- Can be used as a bend structure
- Low head loss from inlet to outlet makes it compatible with existing infrastructure
- Ideal for new developments, redevelopment and retrofit applications

### Essential part of a stormwater treatment train

- Seamless support with pretreatment of existing BMPs (wet/dry ponds, filtration and infiltration devices)
- Improves water quality, extends BMP maintenance life and minimizes costs

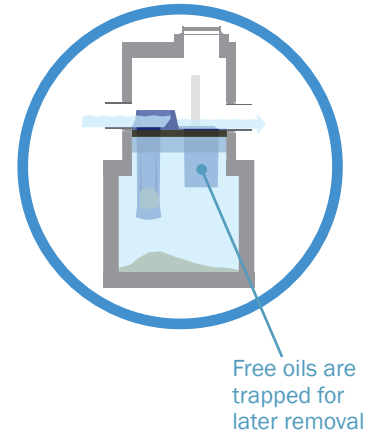
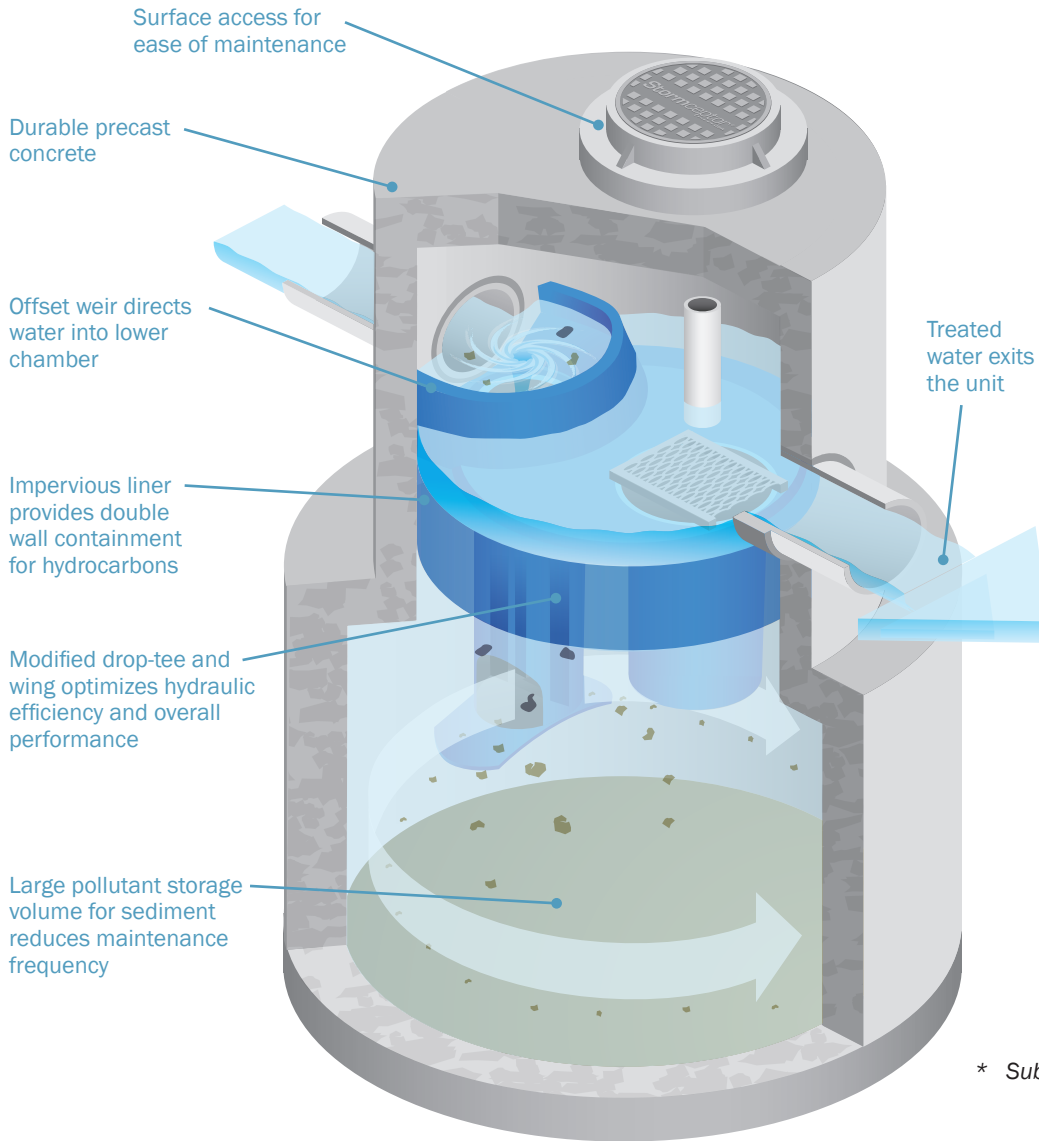
### Maintenance made easy

- Maintenance is convenient and trouble-free, with virtually no site disruption
- Easy unit entrance from surface access cover – no confined space entry needed



With over 40,000 units operating worldwide, Stormceptor performs and protects every day, in every storm.

# Oil & Sediment Removal

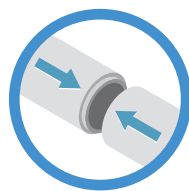


Stormceptor OSR Models		
Model	US	CAD
Inlet OSR	065	300
In-line OSR	140	750
	250	2000
	390	4000
	560	6000
Series OSR	780	9000
	1125	14000

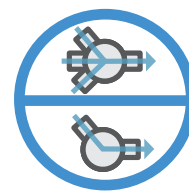
\* Submerged conditions can be accommodated  
\*\* Fiberglass units are available



**Easy to install**  
Small footprint saves time and money with limited disruption to your site.



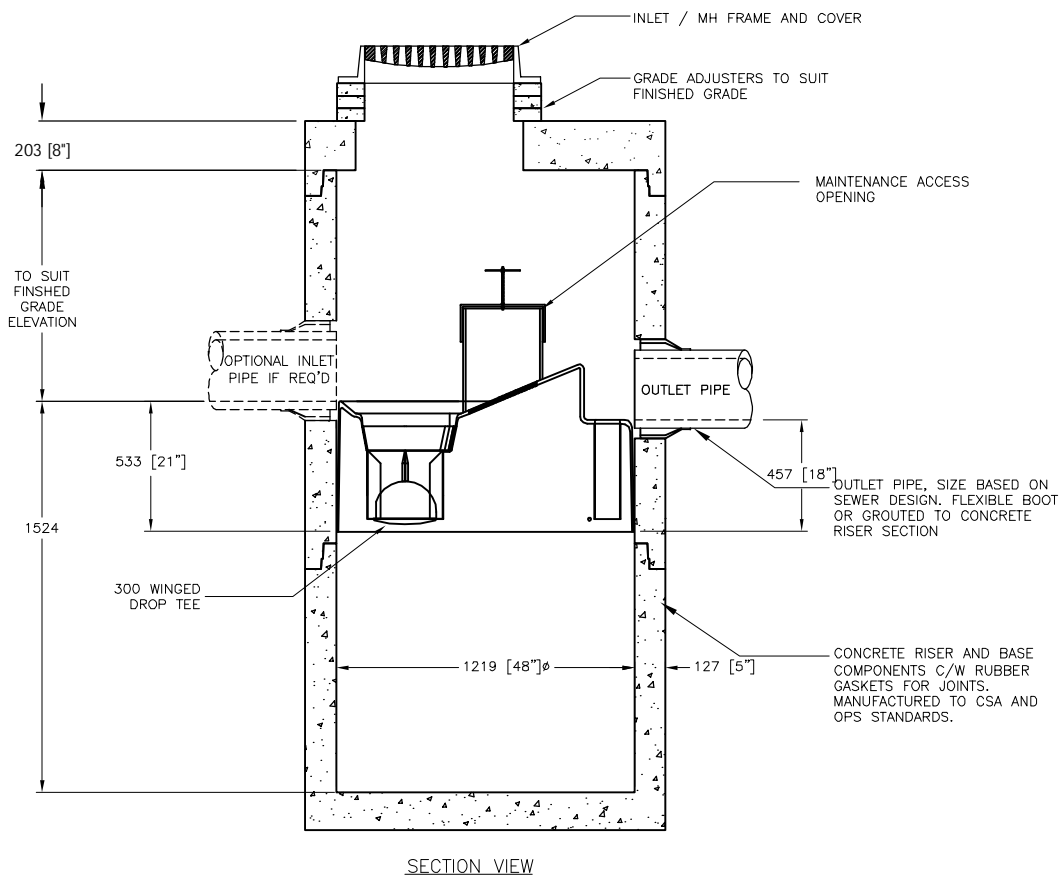
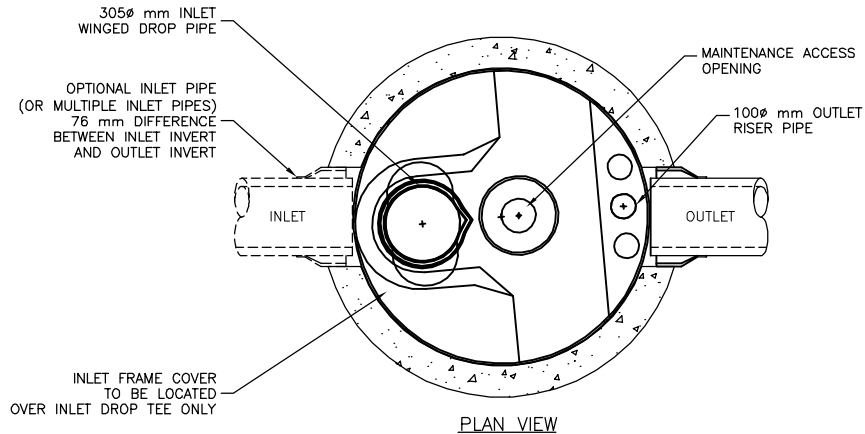
**Seamless**  
Minimal drop between inlet and outlet pipes makes Stormceptor ideal for retrofits and new development projects.



**Flexible**  
Inlet OSR can connect multiple inlet pipes.  
Can be used as a bend structure.


# DRAWING NOT TO BE USED FOR CONSTRUCTION

THE STORMCEPTOR SYSTEM IS PROTECTED BY ONE OR MORE OF THE FOLLOWING PATENTS:  
 United States Patent No. 5,753,115 • 5,849,181 • 6,068,765 • 6,371,690 • 7,582,216 • 7,666,303 | Australia Patent No. 693,164 • 707,133 • 729,096 • 779,401 • 289,647 • 2008,279,378 • 2008,288,900 |  
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 Korea Patent No. 10-1451593 • 0519212 | Malaysia Patent No. 118987 | New Zealand Patent No. 314,646 • 583,583 • 583,008 | South African Patent No. 2010/00683 • 2010/01796 |



## Stormceptor<sup>®</sup>

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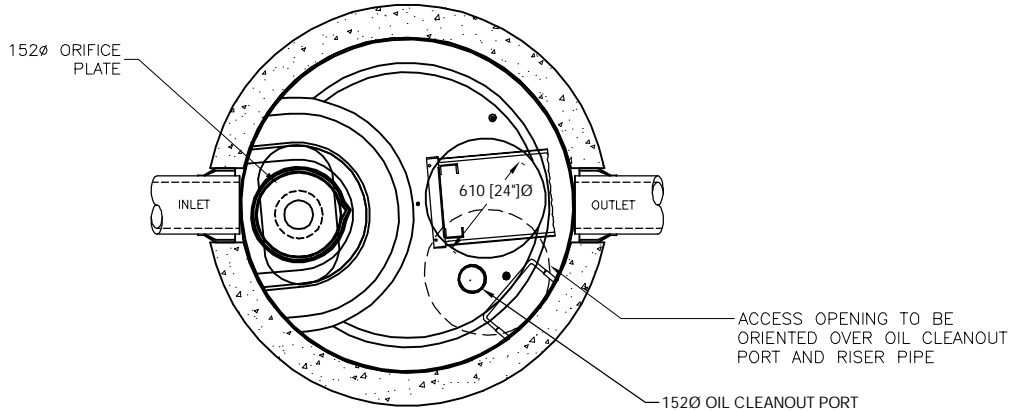
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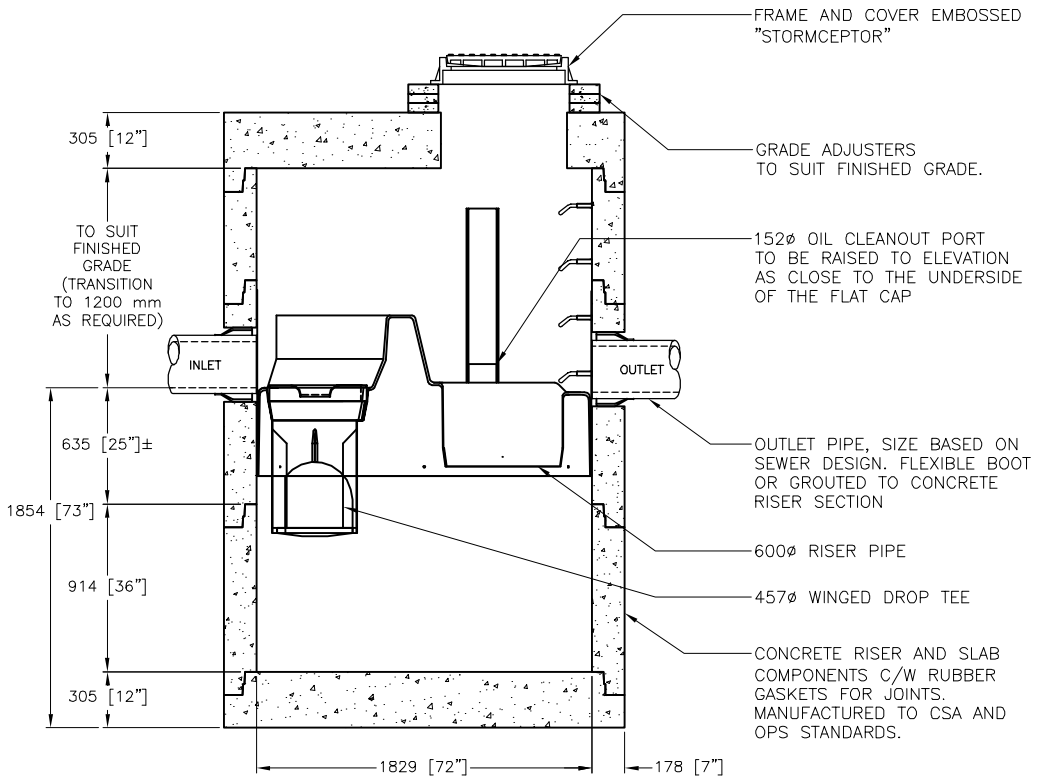
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PLAN VIEW




SECTION VIEW

## Stormceptor®

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